Evaluation of Model Parameters of HAND Model for Real-Time Flood Inundation Mapping: Iowa Case Study

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Abstract

Getting accurate flood extents in a timely manner is of great importance. In this paper, the performance of a real-time-flood-inundation-mapping framework based on a simplified model-Height Above the Nearest Drainage (HAND) was evaluated in a case study for Iowa. The impacts of three key model parameters (drainage threshold, water depth, and the resolution of data) on the prediction accuracy of the HAND model were studied. Furthermore, the effectiveness of several approaches-variable water depths, community-level analysis, and inner zone creation was explored, with which the performance of inundation predictions could be improved without involving additional data. The performance of the HAND model was evaluated by comparing simulated flood extents against statewide flood hazard maps for 100and 500-year flooding events. The results showed that compared to water depths and drainage threshold, the HAND model's accuracy was affected by the resolution of DEM the most. Also, the results proved HAND a useful tool to generate flood inundation maps. Compared to the reference floodplain maps, the Hit Rate reaches 0.80 (0.79 for the 500-year event), Proportion Correct reaches 0.93, and Kappa Value reaches 0.69 (0.70 for the 500-year event) for the basic case where water depths at the outlet point were used and no improvements were involved. The performance of the model was further improved when variable water depths, community-level approach, and inner zone selection were adopted. In the case study, the community-level approach and the inner zone creation helped the Hit Rate improve to 0.88 and the Mixed Index increase by 5.83 % (100-year flooding event) and 5.77 % (500-year flooding event). This study presented how the performance of inundation mapping based on HAND model could be further improved in a simple and practicable way.

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1. Introduction

Flood is one of the most devastating natural events that happen both in rural areas and urban regions (Hlodversdottir et al., 2015; Morris et al., 2016; Short Gianotti et al., 2018; Zhou et al., 2019) causing colossal life, and monetary loses every year (Arrighi et al., 2013; Sayama et al., 2015). Except for natural causes that lead to periodical flooding, climate change and urbanization are the most influential factors that cause extreme flooding events (Huong and Pathirana, 2011; Mahmoud and Gan, 2018; Yazdanfar and Sharma, 2015). There is a trend for flooding events to happen even more frequently in the future since the need for impermeable pavements increases as more people tend to live in cities, and more extreme rainfall caused by climate change will appear (Hammond et al., 2015).

Humans have a long history of fighting against floods (Bukhari, 2016; Cœur, 2017; Yan, 2019), and hydrologic models are playing an increasingly essential role in flooding predictions with the development of computing capability. Hydrodynamic models, empirical methods, and simplified hydrological models are widely accepted for hydrologic modeling and predictions. Among these methods, the simplified conceptual ones are receiving an increasing focus since they have significantly fewer data and computing needs, meanwhile, they are comparably robust and accurate (Teng et al., 2017). Unlike hydrodynamic models, simplified conceptual models do not focus on simulating the movement of water. Also, they do not divide study regions into small grids and apply equations of physics to these grids. Thus, the simplified models will not need boundary conditions, initial conditions, and the like to get started (Teng et al., 2017). Also, unlike empirical methods that depend heavily on data acquisition and processing techniques, and the similarities between historical scenarios and the current scenario (Teng et al., 2017), results from simplified conceptual models could be more independent and robust. Some representatives of simplified models are Rapid Flood Spreading Method (RFSM) (Lhomme et al., 2008), Teng-Vaze-Dutta (TVD) (Teng et al., 2015), and Height Above the Nearest Drainage (HAND) (Rennó et al., 2008).

The flood hazard map (FHM) that communicates flood information (inundation depths, extents) is a useful tool to reduce losses and support social developments (Kreibich et al., 2017, 2005; Mahmood, 2018). FHMs are vital to the society in the following aspects (Luke et al., 2018; Mahmood, 2018): providing essential information for long term flood risk management and response; helping in decision making (Carson et al., 2018) for both stakeholders and households; assisting in making effective land-use decisions, and being a useful tool for insurance professionals and lenders to calculate the premium of houses or businesses based on the risk level. Obtaining FHMs and other hydrological information in a real-time manner is of great importance. We have witnessed efforts being put into real-time flood forecasting using machine learning (Sit and Demir, 2019; Xiang et al., 2020) and community-centric applications. Loi et al. (2019) integrated hydrologic models and weather and stream gauges in a web-based environment. Yildirim and Demir (2019) presented a web-based framework for flood loss estimation using flood maps. Haynes et al. (2018) presented a real-time Mobile Augmented

Reality app linked to live sensor data for flood visualization. Sermet and Demir (2019) introduced information-centric ontologies. Sermet and Demir (2018) introduced an intelligent system that facilitates hydro-information collecting and analyzing, and communication through web-based information platforms and smart assistants.

Recent developments in web and mobile technologies allow high-end capabilities on clientside systems in optimized network representation (Demir and Szczepanek, 2017), distributed volunteer computing for hydrological modeling (Agliamzanov et al., 2020), watershed delineation applications (Sit et al. 2019a), stage measurements at ungauged sites (Sermet et al. 2019a), and crowdsourced data exploration (Sit et al. 2019b). Being less computationally expensive, robust, and accurate, simplified conceptual models like HAND are favorable for creating real-time FHMs and especially suitable for online applications.

HAND was first introduced by Rennó et al. (2008) as a normalized digital elevation model that reflects the local soil water conditions, used for terrain classification (Nobre et al., 2011; Rennó et al., 2008). With its ability to reflect drainage potential, HAND was quickly adopted as a simplified hydrologic model for flood inundation mapping. de Lollo et al. (2019) combined land use information and HAND to obtain quick and low-cost predictions of flood risks for Brazilian urban areas. Speckhann et al. (2018) presented a flood hazard mapping methodology by combining flow frequency analysis with HAND to generate the 2011 flood extent maps in the Itajai River basin, Brazil. Jafarzadegan and Merwade (2019) proposed a statistical approach that creates probabilistic floodplain maps using the HAND model. Their method proved to be able to reduce overestimation and underestimation. Godbout et al. (2019) proposed an approach to adjust the initial slope to improve the performance of the HAND model in reaches with extreme slopes. In addition to studies focusing on the methodology improvements, various comparisons between the HAND and other hydrological models exist in literature. For example, McGrath et al. (2018) presented a comparison among three non-physics-based simplified conceptual flood models. The results showed HAND the best predictor for inundation extents among those three models. Afshari et al. (2018) carried out a comparison among HAND, AutoRoute, HEC-RAS 2D in Cedar River watershed, and part of the Black Warrior River. The results showed the modeling outcomes from HAND were comparable to those from other hydrological models and reference maps.

Researchers have been focusing on improving the performance of flood predictions based on the HAND model since it was introduced. Most studies in literature tried to achieve better performance by involving other data sources or methodology modifications. However, the point of the simplified conceptual models is all about keeping the workflow simple and easy to compute and implement, and about being able to generate robust results with minimum data requirements. Thus, in this study, our purpose is to figure out how the HAND's performance is affected by model parameters, and how we could make the most of the existing data and workflow to yield a better prediction.

2. Material and Methods

2.1 Real-Time HAND Mapping System

We generated the flood inundation maps using a real-time implementation of the HAND model running entirely on client-side web systems (UIHILab, 2020). This system does not require any server-side GIS or database processing and allows users to set study scope and modify parameters when generating an inundation map on the web. The flood extent maps are shown through image files, while users can export the results to raster files for further analyses. The system integrates reference flood map layers from the Iowa Flood Information System (IFIS) that communicates flood forecasts, warnings, and other related information to communities (Demir et al., 2018; Demir and Krajewski, 2013; Krajewski et al., 2017; Weber et al., 2018). Figure 1 shows the parameter selection and definition interface of the system. Five different grid sizes for HAND model—1 m, 5 m, 10 m, 25 m, and 50 m, are available for users to choose from, while Drainage Area (drainage threshold) and HAND (water depth) values accept user-defined integer or float inputs.



Figure 1. The interface of the real-time map system developed by the UIHILab at the University of Iowa

Five counties in the southeast part of Iowa—Linn, Jones, Johnson, Cedar, and Muscatine, are selected as the study region for comparative analysis. Figure 2 shows the scope of the five counties.



Figure 2. The study scope in this case study and its position in the State of Iowa

2.2 Data Processing and Integration

Digital Elevation Model (DEM) data of the study region are integrated into Real-time Map System and can be accessed through web requests. We utilized the floodplain mapping products generated at the Iowa Flood Center (IFC) as reference maps. These maps are made available from the previous works at IFC, where the detailed floodplain maps are created with HEC-GeoRAS using LiDAR data of 1-m resolution (Gilles et al., 2012). Floodplain maps are generated in correspondence with flood events with 2-, 5-, 10-, 25-, 50-, 100-, 200-, and 500-year return period that cover the majority of the State of Iowa. We utilized two specific elements in reference maps—flood extents and water depths. In order to be consistent in comparison, we used water depths from reference flood maps instead of water levels from USGS or other sources as one of the inputs for HAND model in this case study. Missing data in both 100- and 500-year floodplain maps are marked with "no data" tag, meaning they would not play any role in the calculation.

2.3 Procedure of Calculating Draining Potential Using HAND

As introduced by Rennó et al. (2008), the HAND procedure starts from DEM data. A sequence of minor modifications is applied to remove depressions and create a hydrologically coherent DEM (Jones, 2002; Lindsay and Creed, 2005; Nobre et al., 2016, 2011; Rennó et al., 2008). Several modification methods are acceptable (Garbrecht and Martz, 2002; Jones, 2002; Lindsay and Creed, 2005; Martz and Garbrecht, 1998; Rieger, 1998) in this step. Next, flow

paths are obtained from the coherent DEM (Nobre et al., 2011) using flow direction calculating approaches, such as D_8 , D_∞ or the like (Holmgren, 1994; Mark, 1984; Quinn et al., 1991; Tarboton, 1997) and then drainage channels are determined through pixels with an accumulated value not less than the accumulated area threshold (Mark, 1984; Tarboton, 1997). Finally, the elevation of each DEM grid is normalized by subtracting the elevation of the nearest drainage point it drains to from its original elevation. The HAND value of each pixel obtained through the elevation normalization is the elevation difference between a pixel and its nearest drainage point. Detailed introduction of the HAND model and its steps can be found in the literature (Nobre et al., 2011; Rennó et al., 2008).

2.4 Flood Inundation Mapping Based on HAND Model

There are two approaches to obtain a flood extents map using HAND. The first approach is to carry out flood inundation mapping by combining HAND and National Hydrograph Dataset (NHD-HAND), introduced by Liu et al. (2016). This approach starts by generating HAND using DEM data. Then reach-averaged river geometry parameters are calculated, and the Manning's equation is applied to obtain a water stage height-streamflow rating curve. Finally, given a streamflow value from other hydraulic models, such as the National Water Model (NWM), or a gauge point, such as the USGS measuring point, the corresponding water depth can be obtained, and the inundation areas can be calculated. Zheng et al. (2018) compared the river geometry parameters and the rating curve obtained using this approach with those derived from HEC-RAS and reported the results to be reasonable. Keane et al. (2016) realized an online application of viewing flood maps generated by NHD-HAND based on the Tethys Platform.

The other approach is more straightforward and closer to the original definition of the HAND model. The HAND value of each grid represents the relative height of the point over its nearest drainage pixel in main streams. By comparing a given water depth with the HAND value of each grid, we can separate the inundated pixels from the non-inundated ones. According to the concept of the HAND value, if a pixel has a HAND value less than the normalized water depth, it is considered as inundated. Some case studies accepted this more straightforward approach to calculate flood extents (McGrath et al., 2018; Nobre et al., 2016). We choose the straightforward method in this study since the real-time map system supports it.

2.5 Comparative Analysis Design

As one of the simplified-conceptual models, HAND model does not require as many inputs as hydrodynamic ones, but a few inputs are still required. We have selected critical parameters of the model for comparison purposes. The *drainage threshold* is a critical parameter since it distinguishes mainstream grids from those non-mainstream ones based on the total number of upstream grids that converge at the current point. In the model generation process, only the pixel with the number of accumulated grids that reaches or goes above the predefined threshold will be considered as grids in the mainstream. Those mainstream grids are the base points to which the elevations of the other grids are normalized. Therefore, changing the value of the threshold equals to the modification of the river networks. In general, a small threshold value will allow more grids in the mainstream, while a bigger one narrows the river network. Nobre et al. (2016) and Rennó et al. (2008) presented the impacts of changing threshold values on the shape of the river network and modeling results. Another parameter that affects the accuracy of modeling significantly is the *water depth*. Because all grids with a HAND value less than the given water depth are underwater, changing water depth will have a direct impact on the number of points considered inundated or not. The last parameter we chose was the resolution of the DEM for HAND generation. The resolution reflects how approximate the results are when comparing reference flood maps and HAND based ones. If the resolution difference is significant between the two maps, the HAND-based map will be less capable of reflecting the details of the study region. For example, if we compare a 50-meter-resolution HAND-based flood map with a 1meter-resolution reference map, we need to set a single value for every 2,500 grids in reference (50 grids by height and 50 grids by width), even if the values of these 2,500 grids may be different from each other. By comparison, if we use a 5-meter HAND map, then only every 25 reference grids will share the same value, and we will preserve more variation. However, solutions for engineering problems always try to keep a balance between costs and benefits. The accuracy is obtained at the cost of computing time and storage expenses since vast amounts of grids require more significant memory and take a far longer time to process. Also, the largest region with 1-meter-resolution grids that the system can calculate is set to 5 km by 5 km due to browser memory limitations, making it less suitable for large scale analysis at 1m resolution. Therefore, we focused on 5, 10, 25, 25-m-resolution data to compare.

Our comparison analysis consists of three major components. First, we picked two small regions within the study area to demonstrate how three key parameters affect the accuracy of HAND flood extent maps. We only changed one parameter in each parameter set and kept the other two unchanged. Figure 3(a) shows the two regions we picked. Next, we created a 50-meter-resolution region that covers the five counties and determined the best combination of water depth and the threshold for this region (hereafter called regional analysis). Then, we divided the big region into four 25-meter-resolution subregions with a size of 50 km by 50 km and figured out the best parameter set for each subregion (hereafter called community-level analysis). Figure 3(b) presents the division of the 5-county region. Finally, we merged the best-matching HAND map of each subregion and compared the results with the one obtained by regional analysis. Areas that reach off the study scope will be ignored.



Figure 3. (a) Two regions selected to demonstrate the impacts of drainage threshold, water depth and DEM grid resolution (b) The division of the whole region into four 50 km by 50 km subregions with 25meter-resolution grids

Also, allowing users to select study regions in real-time map system may cause areas to sit among multiple watersheds. Borders cutting across watersheds will create false accumulation values for grids near the border. As explained above, all upper stream grids that drain to the current point should be counted as its accumulating values. However, for positions near boundaries, the upper accumulating grids that go outside the border will be ignored and will lower the accumulation values for grids near borders. Therefore, the shape of the river network may change because of this. Creating an inner zone will help address the border problem since we ignore grids affected by border issues and thus can obtain a more consistent result.

2.6 Model Comparison Metrics

We applied quantitative measures, together with the non-quantitative ones to evaluate the performance of HAND-based flood maps by comparing them with the reference floodplain maps. For a pixel in a flood map, there are just two possible statuses, inundated or not inundated, in both model predictions and real situations. Therefore, we developed a two by two Error Matrix (Congalton and Green, 2019; Wilks, 2011) or called Confusion Matrix (Provost, 1998) to

represent the status of a pixel. As shown in Figure 4, we use *Positive* or *Negative* to describe whether a pixel is inundated in HAND model or not, and *True* and *False* to indicate if the prediction is correct taking the reference as the real value.

		Real V	/alues				
		Positive	Negative				
Predicted Values	Positive	TP	FP				
	Negative	FN	TN				

Figure 4. The matrix to evaluate the consistency of model results and references, where TP means inundated grids both in references and model results, FP means inundated grids in model results but not in reference maps, FN means inundated grids in reference maps but not in model results, and TN means dry grids in both references and model results

Researchers have suggested various measures to assess the ability of a model to give good predictions (Congalton and Green, 2019; Wilks, 2011). However, most of the indexes are partial and incomplete representations of the performance, and therefore should be applied together with each other (Wilks, 2011). Here, we adopted the Proportion Correct (PC), Bias Ratio (B), Hit Rate (H), Kappa index (K), and Fitness-statistic (F) to evaluate the performance of HAND model. Readers may find a detailed description of these indexes in the relevant literature (Landis and Koch, 1977; Wilks, 2011).

The Proportion Correct (PC) stands for the most direct and intuitive *correct ratio*, and it does not discriminate True-Positive points from True-Negative ones. The best possible PC value is 1. PC is calculated as in Equation 1:

$$PC = \frac{TP + TN}{TP + FN + FP + TN} \tag{1}$$

The Bias (B) is the ratio of the number of inundated pixels in the HAND flood map to the number of inundated pixels in the reference maps. A Bias value higher than 1 means the HAND model tends to predict more than the event occurs, or in other words, a tendency of overestimating. B is calculated as in Equation 2:

$$B = \frac{TP + FP}{TP + FN} \tag{2}$$

The Hit Rate (H) demonstrates the ability of the HAND model to make correct predictions when the event occurs. The best possible value of H is 1. H is calculated as in Equation 3:

$$H = \frac{TP}{TP + FN} \tag{3}$$

Kappa value (K) is a ratio that measures the actual agreement between two models to the chance of agreement (Afshari et al., 2018; Landis and Koch, 1977). Afshari et al. (2018) indicate that a good K value is possible if the number of correctly-predicted dry pixels is far more than that of the correctly-predicted inundated ones, even the model is failing in detecting inundated grids correctly. The best possible value of the Kappa value is 1 and is calculated as in Equation 4:

$$K = \frac{N(TP+TN) - ((TP+FP) \times (TP+FN) + (FP+TN) \times (FN+TN))}{N^2 - ((TP+FP) \times (TP+FN) + (FP+TN) \times (FN+TN))}$$
(4)

Fitness-statistic (F) is often taken as a supplement to K to address the shortcoming of K value mentioned above. F value stresses more on the consistency of inundated pixels in both flood maps. F is calculated as in Equation 5:

$$F = \frac{TP}{TP + FP + FN} \tag{5}$$

PC, H, K, and F are commonly used to evaluate the model performance. However, literature is limited when it comes to interpreting the results when these four indexes are not entirely consistent with each other. In our cases, there are situations when PC goes up, H goes down, or when H increases, both K and F decrease. Thus, we utilized a dimensionless mixed index to address that problem. As introduced above, we should keep both K and F since they focus on non-inundated grids and inundated ones. We also accepted H, as Hit Rate is a widely used index in evaluating model performance, and it focuses more on how many inundated pixels are distinguished by the HAND mode. Equation 6 shows how the Mixed Index (hereafter called *MI*) is calculated:

$$MI = H + K + F \tag{6}$$

3. Results and Discussion

3.1 Flood Extents with Different Parameter Settings in Two Small Regions

Table 1 summarizes the performance of flood extent estimation in Region #1 and #2 and shows how changes in HAND model parameters (resolution, threshold, and water depth) affect the accuracy of flood extents maps in 500-year flood event. The best performance in each region is bolded and underlined in Table 1. In this section, the water depths used in the calculation are picked around the outlet position. As explained in the previous section, the water depths we used

are products of HEC-GeoRAS modeling. Thus a few different values may be available around the outlet cross-section. The results suggest that the HAND model's resolution has the most significant impacts compared to the other two parameters. As analyzed in the previous section, the resolution reflects how many details in the reference maps are captured when compared. Also, this indicates the importance of the quality of references, since an incorrect or a rough reference map will harm the matching ratio. Figure 5 shows the best matching scenarios in Region #1 and #2 of the 500-year flood event.

Study Region	DEM Resolution (m)	Water Depth (ft.)	Drainage Threshold (km ²)	PC	Н	K	F	MI
	5	17	4.0	0.95	0.96	0.83	0.75	2.55
	10	17	4.0	0.94	0.97	0.80	0.72	2.50
	25	17	4.0	0.93	0.94	0.77	0.68	2.40
	50 5 5	17	4.0	0.92	0.92	0.74	0.65	2.32
		17	3.0	0.95	0.96	0.84	0.76	2.57
Region #1		17	3.5	0.95	0.97	0.84	0.76	2.58
	5	17	3.7	0.95	0.96	0.84	0.76	2.57
	5	17	4.5	0.95	0.95	0.83	0.76	2.55
	5	16	3.5	<u>0.96</u>	<u>0.95</u>	<u>0.86</u>	<u>0.79</u>	<u>2.60</u>
	5	18	3.5	0.94	0.98	0.82	0.74	2.55
	5	19	3.5	0.94	0.98	0.82	0.74	2.55
Region	5	16	4.0	0.94	0.96	0.66	0.53	2.16
#2	10	16	4.0	0.93	0.96	0.64	0.51	2.12

Table 1 The performance summary of the HAND-based flood extents predictions in 500-year flood event using different parameters in region #1 and #2. Where PC stands for Proportion Correct, H stands for Hit Rate, K stands for Kappa value, F stands for Fitness-statistic, and MI stands for Mixed Index.

	25	16	4.0	0.91	0.96	0.57	0.44	1.98
:	50	16	4.0	0.91	0.95	0.56	0.43	1.95
	5	16	3.0	0.93	0.97	0.64	0.51	2.13
	5	16	3.5	0.94	0.96	0.66	0.53	2.16
	5	16	3.7	0.94	0.96	0.66	0.53	2.16
	5	16	4.5	0.95	0.94	0.69	0.55	2.19
	5	16	4.7	0.95	0.92	0.69	0.56	2.18
	5	16	5.0	0.95	0.92	0.69	0.56	2.18
	5	18	5.0	0.92	0.97	0.61	0.48	2.06
	5	17	5.0	0.93	0.94	0.64	0.50	2.09
	5	15	5.0	0.95	0.92	0.69	0.56	2.18
	5	14	5.0	0.95	0.91	0.70	0.57	2.18
	5	13	5.0	<u>0.97</u>	<u>0.86</u>	<u>0.76</u>	<u>0.63</u>	<u>2.25</u>

As shown in Table 1, all indexes except for PC are better in region # 1 than those in region # 2. We believe this is mainly because of the topological difference between the two regions. As shown in Figure 3(a), region #1 locates in the lower stream of the main river channel, while region #2 lies in a relatively upper stream and is consisted mostly of small or ending branches. However, the two images in Figure 5 still show some similarities—the predictions in the main channel in each image were acceptable, and overestimations were more common among small branches.



Figure 5. Best matching scenario for two regions in the 500-year flood event (a) Region #1, (b) Region #2

3.2 Regional vs. Community-Level Analysis

In this section, instead of testing possible water depths near the outlet position, we used three typical water depths—depth at the outlet (D_0) , weighted average depth (D_W) among all inundated grids in reference floodplain maps, and the average water depth (D_A) of the D_0 and the D_W . We adopted the depth at outlet position because of its clear physical meaning—it is the point where all branches in the upstream converge, so water depth picked here has the same meaning as those from hydrologic stations even though there is no real station. However, using only D_0 might create too many overestimations since the outlet point lies downstream, and the value is picked from the middle of the river's cross-section. Therefore, weighted average water depth is selected as our next testing choice because it reflects the average water depth on top of each inundated grid. We calculated the weighted average depth using Equation 7:

$$D_W = \frac{\sum_n Area_i \times Depth_i}{\sum_n Area_i} \tag{7}$$

Where n is the total number of inundated grids within the study scope, and *i* represents each individual pixel. The shortcoming of D_W is that it distributes water equally among all inundated grids. Thus, water depths of grids in main river channels tend to be lowered by the vast number of water-affected pixels that are not in the main channel or near the riverbank. Therefore, by arithmetically averaging D_O and the D_W , we obtained an eclectic water depth (D_A) that can reflect both the characteristics of all inundated grids and the outlet point.

After defining these three typical water depths, we first calculated the five-county region using the regional approach. Table 2 shows the flood extents comparison using D_0 , D_W , and D_A in the 100- and 500-year flood event, where the best performance in each region is bolded and underlined. TP, FP, FN, TN values are shown using the percentage of the numbers of the

classified grids within the study scope of the total 2,960,740 and 2,945,656 points, respectively. The number of grids is different because the missing data regions in the 100-year reference are slightly different from those in the 500-year reference map.

				U	~~~						
Flood event (Return Period)	Wate (r Depth ft.)	TP (%)	FP (%)	FN (%)	TN (%)	PC	Н	K	F	MI
100-year	D ₀	9.00	<u>9.01</u>	<u>4.52</u>	<u>2.20</u>	<u>84.27</u>	<u>0.93</u>	<u>0.80</u>	<u>0.69</u>	<u>0.57</u>	<u>2.06</u>
	D_W	6.81	8.12	3.39	3.08	85.41	0.94	0.73	0.68	0.56	1.97
	D_A	7.91	9.01	4.52	2.20	84.27	0.93	0.80	0.69	0.57	2.06
500-year	D _O	10.00	9.53	4.16	2.52	83.79	0.93	0.79	0.70	0.59	2.08
	D_W	7.85	9.35	3.77	2.70	84.18	0.94	0.78	0.71	0.59	2.08
	D _A	8.92	<u>9.35</u>	<u>3.77</u>	<u>2.70</u>	<u>84.18</u>	<u>0.94</u>	<u>0.78</u>	<u>0.71</u>	<u>0.59</u>	<u>2.08</u>

Table 2 Flood modeling results using different water depths by the regional approach. Where D_0 stands for water depth at the outlet, D_W stands for the weighted average depth, and D_A is the average of D_0 and D_W .

The first and last record of the 100-year flood in Table 2 is the same because the region lacks the value of 8 feet in the HAND matrix, which is the same reason why the last two records of 500-year flood events in the table are the same.

In the community-level analysis, we tested the performance using different parameter sets, and the best parameter set was selected for each subregion in Figure 3(b). Figure 6 shows the variation of the MI in each subregion in the 100- and the 500-year flood events when HAND model parameters vary. The blue line represents the 100-year flood event, and the red line represents the 500-year one. Figure 6 indicates that there is no parameter set that can guarantee the best modeling results in all cases. The fact that the optimal set varies from different subregions suggests that the best model parameters may be affected by the intrinsic characters of that region, such as averaged slope, river network density, etc. However, a few similar results are shared among subregions. First, the results generated using outlet water depth are similar to those using average water depth. Second, results using outlet water depth and average water depth are better than that using the weighted average depth in both 100- and 500-year flood events in all subregions 3, the performances of three water depths are

quite similar in the 100-year flood case, while the weighted average depth outperformed the other two in the 500-year flood event.



Figure 6. Flood extents evaluation under different parameter settings in four subregions (a) subregion 1; (b) subregion 2; (c) subregion 3; (d) subregion 4

Just as Nobre et al. (2016) increased the drainage threshold value from 0.405 km² to 4.05 km² and greatly improved the inundation results, our results demonstrated the stable performance of the threshold value of 4.0 km², with half of the parameter sets having the 4.0 km² threshold. In our case, all threshold values of the best parameter sets fall between 3.0 km² and 4.5 km². But still, the best threshold value varies slightly among subregions. Figures 7 and 8 show the inundation maps using the best parameter set for all subregions in the 100- and 500-year flood events. The outer black boxes show the true scope of each subregion. The details of the best performances in each subregion are listed in Table 3.



Figure 7. Best predictions for each subregion in the 100-year flood event (a) subregion 1; (b) subregion 2; (c) subregion 3; (d) subregion 4



Figure 8. Best predictions for each subregion in the 500-year flood event (a) subregion 1; (b) subregion 2; (c) subregion 3; (d) subregion 4

T 1 1	Parame		тр	FD	FN	TN							
Flood Event	Water Depth (ft.)	Threshold (km ²)	Region	(%)	(%)	(%)	(%)	PC	В	Η	K	F	MI
100-year	<i>D</i> _A =10.9	4.0	1	6.9	5.0	1.0	87.1	0.94	1.51	0.87	0.66	0.53	2.06
	<i>D</i> _A =8.9	3.5	2	6.6	4.3	1.3	87.8	0.94	1.38	0.84	0.68	0.55	2.07
	<i>D</i> _W =9.9	3.0	3	12.1	4.7	2.5	80.7	0.92	1.15	0.82	0.72	0.62	2.16
	<i>D</i> ₀ =12.0	4.5	4	11.3	7.0	1.9	79.8	0.91	1.38	0.85	0.66	0.56	2.07
500-year	<i>D</i> _A =11.4	4.0	1	7.6	4.3	1.5	86.6	0.94	1.30	0.84	0.69	0.57	2.10
	<i>D</i> _A =11.8	4.0	2	7.6	5.2	1.0	86.2	0.94	1.48	0.88	0.68	0.55	2.11
	<i>D</i> _W =11.6	3.0	3	12.4	5.7	2.2	79.7	0.92	1.24	0.85	0.71	0.61	2.17
	<i>D</i> _A =10.1	4.0	4	12.5	6.4	2.1	79.0	0.91	1.29	0.85	0.69	0.59	2.13

Table 3 The best predictions in each subregion in the 100-year and 500-year flood events with parameters

Table 3 shows that HAND flood extents maps successfully predicted most inundated grids in reference floodplain maps, with H reaching at least 82.0 %, PC above 91.0 %, and Kappa value more significant than 0.6 in all cases. According to Landis and Koch (1977), Kappa value higher than 0.6 indicates a substantial agreement with the reference maps. B higher than one means HAND predictions in all four subregions tend to overestimate more than underestimate. This has also been demonstrated by Figures 7 and 8, as green areas (overestimated areas) look more prominent than red areas (underestimated areas). We noticed that compared to small branches, major channels are better predicted as fewer mismatchings appearing around large channels than smaller ones.

3.3 Final Comparison

As the last part of the analysis, we mosaicked the best matching scenario of the four subregions together and compare the mosaicked result with the one from the regional approach. Furthermore, we created an inner zone to address the border issue. Figure 9 illustrates the best matching predictions after merging and the creation of the inner zone in the 500-year flood events. The values of the overlapping area among subregions were from the best matching

subregion. Improvements in modeling accuracy because of the mosaicking process and the internal zone creation are listed in Table 4.

Flood Event	Calculation Approach	TP (%)	FP (%)	FN (%)	TN (%)	Total Grids	PC	В	Н	K	F	MI
100- year	region as a whole	9.01	4.52	2.20	84.27	2,960,740	0.93	1.21	0.80	0.69	0.57	2.06
	mosaic	9.58	5.41	1.68	83.33	11,756,660	0.93	1.33	0.85	0.69	0.57	2.11
	inner zone of the mosaicked region	9.09	5.04	1.28	84.59	7,012,049	0.94	1.36	0.88	0.71	0.59	2.18
500- year	region as a whole	9.35	3.77	2.70	84.18	2,945,656	0.94	1.09	0.78	0.71	0.59	2.08
	mosaic	10.42	5.48	1.72	82.38	11,700,281	0.92	1.31	0.86	0.70	0.59	2.15
	inner zone of the mosaicked region	9.87	5.10	1.36	83.67	6,970,707	0.94	1.33	0.88	0.72	0.60	2.20

Table 4 Comparison of modeling results using different approaches

As Table 4 shows, the community-level approach helps improve H. However, other indexes did not improve or failed to remain unchanged as H and MI increased. Compared to the mosaicked region, the inner zone improved PC, H, K, and F. The approach of community-level calculation helped MI increased by 2.4 % (the 100-year flood event) and 3.4 % (the 500-year flood event) compared to the regional calculating approach. The inner zone increased MI by 3.3 % (100-year flood) and 2.3 % (500-year flood), compared to the results using the community-level approach. However, we believe that different from calculating big regions at a community level, the inner zone approach should be carefully applied. Only when the model border cuts across watersheds, should it be considered. If the boundaries of the study region and watershed overlap, then no inner zone should be used.



Figure 9. Predictions in 500-year event after mosaicking the best results in four subregions and the inner area creation (a) mosaicked flood extents in 4 subregions; (b) the internal zone of (a)

4. Conclusions

In this study, we evaluated how flood extent predictions by a simplified-conceptual model— HAND (Height Above the Nearest Drainage) change under different parameter settings and how they could be used to improve the model results without involving additional data sources. We chose Linn, Jones, Johnson, Cedar and Muscatine counties in the State of Iowa as the study areas, and 100-year and 500-year floodplain maps as references. We utilized a real-time flood map system developed at the University of Iowa for the flood maps generation based on the HAND model. The results demonstrated the capacity of the HAND model to give acceptable flood extent predictions, even using the coarsest DEM data with no additional improving methods involved. In the basic case, the Hit Rate reaches 0.80 (0.79 for the 500-year event), Proportion Correct reaches 0.93, and Kappa Value reaches 0.69 (0.70 for the 500-year event). Furthermore, we studied the impacts of drainage threshold, water depth, and grid resolution on the HAND matrix generating in two regions with different topology characters. The results show that the DEM grid resolution used has the most significant influence as it reflects how approximate the comparison is between the model results and the reference. The lower the resolution of DEM is, the more details in the reference maps will be lost. Also, we compared the inundation results from the regional and community-level calculations in the five-county region.

We also created an inner-zone approach that could help solve the boundary issue occurring when the border of the study area and the border of watershed do not overlap. The results indicate that the community-level calculation help improve the H compared to the regional calculation approach. The approach of creating buffer zones around the border helps make a more comprehensive improvement for PC, H, K, and F values. In our case study, the final Mixed Index (MI) improved by 5.83 % and 5.77 % in the 100- and 500-year flood events respectively, compared to the original values. However, despite the effect of the inner-zone approach, we suggested that it should not be applied unless the study scope is cutting across watersheds. This study indicates that the flood extent predictions based on the HAND model can be further improved if we model in smaller regions and choose suitable parameter set for each area or accept targeted strategies such as creating inner zones. Findings in this study may benefit largescale modeling using the HAND method or other simplified conceptual models since the approach we introduced does not require additional data sources.

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