

1 CFAAR without a plan: a design-development framework for unplanned river
2 restoration opportunities

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7 **Abstract**

8 The practice of river restoration is a growing field that seeks rejuvenation of river
9 functionality. Restoration opportunities can range from prescriptive actions falling out of
10 detailed planning efforts, to completely unplanned opportunities driven by extenuating
11 circumstances. Unplanned opportunities have received relatively little attention within the
12 literature, and as a result there is a lack of formal guidance on how to step through unplanned
13 restoration opportunities in a holistic manner. To improve our collective abilities to address
14 unplanned opportunities we have developed *CFAAR*, a design-development framework
15 constructed and tested for applicable circumstances. *CFAAR* is short for Context, Feasibility,
16 Alternatives, Analysis and Refinement. It is a simple framework in concept, which sequentially
17 builds a restoration vision founded on a concrete understanding of the project site *context*, from
18 physical to regulatory, coupled with a clear and honest appraisal of restoration *feasibility*,
19 acknowledging that restoration can simply describe an enhanced or improved river condition.
20 *CFAAR* offers a viable solution for unplanned actions and it can be utilized by trained and
21 untrained scientists, engineers, or resource managers alike, who are supported by an appropriate
22 team of restoration professionals. In developing and applying *CFAAR* it is clear that the
23 framework works well with projects that are scheduled on compressed timelines, circumstances
24 which heighten the risk of making mistakes and taking shortcuts. Under such circumstances,
25 *CFAAR* compels restorationists to produce rational and transparent design concepts, effectively
26 communicated via the *CFAAR* construct.

27 Keywords: river restoration; unplanned restoration; design framework; geomorphology

28 **1. Introduction**

29 Functional river ecosystems provide humans clean drinking water, a rich food supply, flood
30 protection, a source of income, and recreation (World Water Assessment Programme, 2006).
31 Unfortunately there are fewer and fewer rivers in the world that are functional (Wohl, 2010).
32 Decades of dam building, over allocation of flow, and other myriad of stressors has resulted in
33 the degradation of ecosystem services, aquatic and terrestrial habitat, and biodiversity (World
34 Water Assessment Programme 2006; Palmer and Filoso, 2009). Predicted change to climate and
35 associated phenomenon add to the seriousness of the observed trends (Fig. 1), and heightens the
36 need for enhancement of rivers worldwide. River enhancement falls within the realm of *river*
37 *restoration*, a somewhat controversial yet growing practice (Bernhardt et al., 2005).

38 River restoration – defined here as deliberate acts to measurably improve existing river
39 conditions – benefits from a substantial and diverse literature base (e.g. Doll et al. 2003, Shields
40 et al. 2003, FISWRG 2004, Saldi-Caromile et al. 2004, Palmer et al. 2005, Darby and Sear 2008,
41 Beechie et al. 2010, Skidmore et al. 2010). A common theme within the literature provides that
42 ideal(ly) restoration is supported by science-based planning that identifies specific actions. The
43 wisdom of this theme is clear, and it is, in general, accepted as standard practice within the
44 restoration community. In application however, science-based planning efforts do not account
45 for all restoration actions. There exists a whole class of river restoration that is not envisioned or
46 planned, but crops up due to unpredictable circumstances. This class of restoration opportunities
47 can be thought of as non-ideal cases, as they commonly lack scientific basis with regards to their
48 identification as a viable restorative or enhancement action. Despite this shortcoming, it is
49 proposed that these opportunities can systematically and positively contribute to revitalization of
50 river systems through use of a science-based design-development process, created specifically
51 for the circumstances which characterize non-ideal opportunities. Restoration practitioners
52 presently lack a science-based design framework for non-ideal cases, heightening its need.

53 Lack of a suitable design-development framework sets non-ideal opportunities at a
54 disadvantage to their planned counterparts. This inequity is critical given that we find ourselves
55 in an era defined by financial uncertainty, which raises the importance of unplanned
56 opportunities. To respond effectively when non-ideal cases materialize, river restoration practice
57 needs a simple yet effective framework that can successfully adapt the practice to unplanned

58 opportunities. Beyond a basis in science, the framework needs to also serve as a useful
59 communication and outreach vehicle (Jacobson and Berkely, 2011). Effective communication
60 and outreach will foster efficiency, consensus building, and help to formalize an associated
61 standard of practice for non-ideal circumstances.

62 We introduce *CFAAR*, short for Context, Feasibility, Alternatives, Analysis and Refinement,
63 a design-development framework suitable for the circumstances of unplanned restorative
64 opportunities, and usefully comparable to the defining framework of Adaptive Management
65 (Smith, 2011). Experience suggests that diligent use of *CFAAR* as a science-based design
66 framework will help us to realize the collective restorative successes that we seek and need. A
67 brief review of the *CFAAR* framework follows and focuses on describing each component to a
68 level sufficient for utilization.

69 **2. CFAAR: Context, Feasibility, Alternatives, Analysis and Refinement (Fig. 2)**

- 70 a. *Context* – spatial, temporal and cultural considerations. It is essential that restorative plans
71 explicitly consider the contextual setting of each project (Montgomery 2004, Beechie et al.
72 2010, Skidmore et al. 2010; Jacobson and Berkeley, 2011), because context defines the scale
73 at which river corridor processes, and impacts to those processes, are broadly manifest.
74 Without context, it is not possible to explain the present river corridor’s character, let alone
75 begin to conceptualize design alternatives to affect the desired change. Consideration and
76 characterization of context is challenging because it comprises the physical, ecological,
77 cultural, and regulatory dimensions (Jacobson and Berkley, 2011). This necessitates that
78 design teams include scientists and practitioners from a broad spectrum of disciplines
79 (Montgomery 2004, Palmer 2008). Furthermore, design teams need to truly function as
80 teams, drawing on the strengths of the various team members to develop the best concepts
81 and plans possible. Clear deliberation of context at the start of the design process for
82 unplanned opportunities defines the core precept of *CFAAR*. This supports scientists and
83 practitioners to knowledgeably assess restorative feasibility, and ultimately enhances the
84 probability of project success.
- 85 b. *Feasibility* – a distilled manifestation of goals, objectives and design criteria. The
86 development of project feasibility is appropriately guided by establishing a concise and
87 guiding restorative image (Palmer et al. 2005). Understanding context, focus of the

88 restorative image is sharpened by weighing three attributes of contemporary river systems:
89 perceived risk of restorative actions, land availability, and the hydrologic and sediment
90 supply integrity of the basin (see *Feasibility* diagram of Fig. 2). The usefulness of these
91 attributes stems from their representation of psychological (perceived risk), practical (land
92 availability), and ecosystem (hydrologic and sediment supply integrity) circumstances, which
93 together complements the “self-healing” suitability paradigm developed by Kondolf (2011),
94 and the conceptual restoration model of Jacobson and Berkley (2011). Collectively, these
95 circumstances will largely govern the trajectory of feasible restorative actions, which can be
96 summarized as design approaches to: (a) control corridor process, (b) rejuvenate corridor
97 process, and (c) achieve both control and rejuvenation of corridor process. For example, if
98 working on a reach of river where the available land is low and the perceived risk is high, it
99 is likely that it will be necessary to control corridor processes over the project lifetime, which
100 is likely to extend 25 to 50 years. When project feasibility has been identified, it is detailed
101 into goals, objectives, and design criteria. Goals are understood as the general or specific end
102 states to achieve, comparable to hypotheses under Adaptive Management (Smith, 2011),
103 objectives as the defined and measurable system attributes by which achievement of goals
104 are judged, contributing to the learning process so key to Adaptive Management (Jacobson
105 and Berkley, 2011; Smith, 2011), and design criteria the articulated performance metrics
106 against which design analyses are compared. Notably, objectives are the cornerstone of the
107 Adaptive Management approach to restoration (Smith, 2011). Likewise, objectives are one
108 of two key outcomes to feasibility visioning within CFAAR, and therefore serve to guide
109 development of suitable design alternatives. By front loading CFAAR with careful
110 assessment of context and feasibility prior to development of specific design objectives, it is
111 envisioned that the selected project design will be more likely to successfully drive the
112 system to the desired and scientifically-appropriate outcome.

113 c. *Alternatives* – development of viable design concepts. Design alternatives are
114 conceptualized once project context and feasibility are clearly established. In order to drive
115 creativity, two alternatives at a minimum should be developed for each project. Design
116 alternatives should address the physical and ecological circumstances, and potential of a site
117 (Palmer et al. 2005), as established by studies, back-grounding and consensus building
118 completed to characterize context and feasibility. For projects that seek to rejuvenate

119 corridor processes, alternatives are first mocked up by sketching bird's-eye and section view
120 layouts for the project reach, considering the target vegetative communities, range of channel
121 bed slopes, and characteristic stream and corridor width which the constructed project will
122 need to express. For projects which seek to control process, bird's-eye and section view
123 layouts should focus, in part, on the general details of proposed structures, with careful
124 thought given to the interface between proposed structures and infrastructure, or private
125 property which control-centric projects generally aim to protect. The common goal at this
126 point is to advance and illustrate alternatives in enough detail to understand design intent and
127 facilitate analysis. Achieving this goal also aids outreach to project stakeholders and
128 regulatory agencies in order to seek their constructive input at this critical stage in the design-
129 development process.

130 d. *Analysis* – functional and technical evaluation of alternatives. Proposed alternatives should
131 be fully evaluated technically (Wilcock 2004, Beechie et al. 2008, Darby and Sear 2008),
132 including an assessment of potential impacts framed by applicable environmental regulations
133 such as the National Environmental Policy Act (1969), and many others. Analysis generally
134 involves hydraulic modeling, sediment transport and bed stability modeling, evaluation of
135 fish passage and habitat conditions, and risk-based evaluations related to climate change and
136 land use projections. Hydraulic modeling is used to evaluate and compare the predicted
137 hydraulic performance of alternatives against the goals of the project, typically for a suite of
138 flood events. Pertinent hydraulic performance elements include the predicted spatial patterns
139 of flooding, and the spatial distribution and magnitude of hydraulic characteristics such as
140 velocity. Sediment transport analysis relates to evaluation of the spatial patterns and
141 magnitudes of streambed and floodplain erosion, as well as deposition, and how these
142 patterns compare to the proposed stream morphology (Wilcock 2004, Beechie et al. 2008).

143 Fish passage evaluation is based on comparing hydraulic modeling results to established fish
144 passage criteria (NOAA Fisheries 2001, CDFG 2009). Projects that seek to improve in-
145 stream fish habitat are commonly evaluated with Habitat Suitability Index (HSI) curves
146 (USFWS 1996). The use of HSI curves to evaluate design alternatives helps to maximize the
147 potential benefit of constructed habitat to target fish species. Accordingly, HSI curves have
148 been developed for a large number of freshwater fish species. Their use, along with the use of

149 fish passage criteria expressly motivates design professionals to seek new levels of design
150 ingenuity, and fosters a design culture that provides for methodical and careful evaluation of
151 alternatives outside the typical stability-based paradigm (Shields et al. 2003).

152 e. *Refinement – final adjustment of design elements to address deficiencies revealed through*
153 *analysis and stakeholder review.* Refinement represents the last critical step in the CFAAR
154 process, and it is intended to provide all parties involved with an opportunity to express clear
155 commitment to the project. The process of securing stakeholder commitment is facilitated
156 through preparation of a design basis report. Design basis reports (DBRs) have a long
157 tradition of application in architecture and engineering, and their use in ecological restoration
158 design is a natural extension of this useful tool. CFAAR can be used as the organizing
159 framework for DBRs because it provides the structure necessary to prepare a report that
160 documents the design process in a clear and comprehensive fashion. It also facilitates more
161 meaningful participation by project stakeholders who may lack the particular expertise
162 related to ecological restoration because the components of CFAAR provide the cues
163 necessary to ask appropriate and critical questions. All wrapped up, this should help to foster
164 stakeholder confidence, minimize dramatic design overhauls, reduce restoration design costs
165 and facilitate an enhanced probability of restorative success.

166 Once the scope of design refinement has been established through review of the DBR, the
167 normal design progression provides for completion of the design package, suitable for
168 regulatory permitting, and advertisement for implementation. It is important to note that
169 unplanned restorative opportunities should not generally be viewed as experiments, as the
170 Adaptive Management system demands, because regulatory agencies will typically require
171 explicit project outcomes in order to successfully mitigate for impacts of associated actions.
172 This underscores the practical logic framing the CFAAR progression, accentuated by design
173 refinements that are informed by results of supporting technical or other analyses, and
174 feedback by stakeholders and regulatory agencies. Experience suggests that practitioners
175 should budget the refinement step carefully.

176 3. **CFAAR Case Study Application**

177 A brief case study application of *CFAAR* is provided. It represents a summary of the design
178 development process for the Upper Penitencia Creek mitigation project (UPC Project) (Fig. 3)
179 (Chartrand, 2011). Compensatory mitigation of Upper Penitencia Creek was associated with an
180 approximately 2.3 billion dollar (U.S.) commuter rail expansion project spearheaded by The
181 Santa Clara Valley Transportation Authority (VTA), a transportation agency servicing the South
182 San Francisco Bay region. Mitigation was necessary in order to balance construction related
183 impacts to freshwater resources, which in California are protected by numerous State and Federal
184 (U.S.) regulations. The UPC Project was chosen as an example *CFAAR* application because it is
185 characterized by competing interests, the watershed harbors an important regional steelhead
186 population (*O. mykiss*) and it lacks a comprehensive planning document, identifying science-
187 based and coordinated restoration actions. More importantly though, design of the UPC Project
188 occurred under a very compressed timeline, without prior foresight, and as such it exemplifies an
189 unplanned restorative opportunity.

190 Available technical studies useful to development of mitigation design alternatives included a
191 (1) steelhead limiting factors analysis (Stillwater Sciences, 2006), (2) historical ecology
192 assessment of the lower watershed (Grossinger et al., 2006), (3) geomorphic assessment (Jordan
193 et al., 2009), (4) three seasons of streamflow and sediment transport measurements (unpublished
194 data), and (5) geotechnical analysis. During preparation of UPC Project design documents, the
195 U.S. Army Corps of Engineers (ACOE), in conjunction with the local flood control agency, the
196 Santa Clara Valley Water District, was in the process of completing a new flood control study
197 for UPC. The flood control project was developing along a trajectory to support a functional
198 stream corridor environment. This was a benefit to the UPC Project design process as mitigation
199 concepts were more aligned with the Flood Risk Management (Menke and Nijland, 2008), or
200 Living River concepts of flood control, building from traditional flood control approaches.

201 a. *Context* – Surficial geologic maps (Helley and others, 1994) show that the UPC Project is
202 located at the distal end of Upper Penitencia Creek’s youngest alluvial fan, at the transition
203 with floodplain or bank levee deposits from Coyote Creek. An historical map circa 1800
204 suggests that Upper Penitencia Creek at the project site did not in fact exist at that time (Fig.
205 3), and was instead an intermittent, distributary stream system. The distributary streams
206 supported a freshwater marsh complex of thick willow groves, known locally as sausals

207 (Grossinger et al., 2006). The sausals were fed by seasonal surface flows generated during
208 large, winter precipitation events, as well as by shallow ground-water flow, discharged
209 during the spring and early summer months at the head of the alluvial fan.

210 In the early 1900's the sausals were destroyed and in their place a single thread channel was
211 dug to drain Upper Penitencia Creek to Coyote Creek in order to support land conversion for
212 agricultural purposes. Agriculture was abandoned beginning in the 1960s as the "Silicon
213 Valley" technology center emerged, and with it substantial population growth and
214 urbanization, leading way to neighborhoods and technology campuses. These land use shifts
215 have brought significant change to the stream's hydrology, driving conversion of the
216 lowermost reaches of the stream from an ephemeral, to a nearly perennial stream, despite a
217 29% reduction in the lowland watershed footprint (Jordan et al., 2009). Urbanization has
218 also led to a shift in the flood flow hydrology, increasing the magnitude of flood flows and
219 creating a new highly recurrent flood. Hydrological modeling shows that flows < 0.5
220 bankfull discharge occur more frequently under urbanized conditions (Jordan et al., 2009).

221 At the UPC Project site, the watershed drains approximately 62 km², 54 of which occur
222 within the predominantly un-urbanized headwaters canyon of the Diablo Range. Upstream
223 of the UPC Project, the contributing drainage area produces an approximate bankfull flow of
224 7.6 cubic meters per second (cms) (Jordan et al., 2009), which corresponds to roughly a 1.5-
225 year recurrence interval flood. The 5-year flood is estimated as 45 cms. At the same
226 location Jordan et al. (2009) also report a bankfull slope of about 0.68%, and an average
227 bankfull width and depth of 8 m and 0.6 m, respectively.

228 The UPC Project streambed is composed of cobbles, gravels and sand. The sediment supply
229 regime of the upper watershed is relatively intact, and sediment delivery from the upper
230 watershed to the lower, urbanized portion of the watershed is for the most part unimpeded.
231 As a result, the watershed sediment transport regime can be considered reasonably
232 functional. At the UPC Project, the stream was confined by steep banks composed of natural
233 substrate, and a variety of man-made materials. The lower one and a half kilometers or so of
234 the channel lacks floodplain, and is characterized by weakly developed riffle, pool and bar
235 morphology. The most apt description of the UPC Project reach is that akin to a drainage
236 canal.

237 The Upper Penitencia Creek watershed is a primary producer of steelhead within the Coyote
238 Creek basin (Stillwater Sciences, 2006), boasting one of the few steelhead runs in the South
239 Bay of San Francisco (Buchanan et al, 1999) and offering what is considered to be the best
240 steelhead habitat within the region (SCBWMI, 2003). The watershed also supports several
241 other native fish species including Pacific Lamprey (*L. tridentate*), California Roach (*L.*
242 *symmetricus*) and others (Stillwater Sciences, 2006).

243 Synthesis of UPC Project *Context* frames several tangible design development
244 considerations. The historical alluvial fan setting coupled with the reasonably intact
245 sediment supply regime suggests that enhancement design alternatives should account for
246 periods of high sediment loading and deposition. Promotion of transient sediment storage
247 will need to be balanced by flood conveyance standards given the urban nature of the UPC
248 Project. Enhancement design alternatives should also address the altered watershed
249 hydrologic regime, as accelerated rates of erosion are common within streams of altered
250 hydrology.

251 b. *Feasibility* – The highly urbanized nature of the UPC Project suggests that the perceived risk
252 of the project is *high*, as any significant failure of the project could have serious
253 consequences. The perceived risk in this particular case is balanced by the generally *high*
254 land available to the UPC Project. We also assume that the integrity of the hydrologic
255 regime is relatively *low*, whereas the integrity of the sediment supply regime is *high*. These
256 three site- and watershed-specific feasibility characteristics suggest that the UPC Project can
257 likely achieve some level of process rejuvenation at a local scale, while also needing to
258 control process where the perceived risk is most high, for example at a sharp meander bend
259 bordered by a roadway. Long-term liabilities associated with efforts to control process must
260 be communicated honestly to project owners, namely that control measures in particular have
261 a finite lifetime, much like any engineering project, and as such long-term plans need to
262 account for eventual repair, reconstruction, etc. of pertinent project components.

263 Determination of project *Feasibility* enabled development of rational goals, objectives and
264 design criteria for the UPC Project. The primary goal of the UPC Project was to physically
265 and ecologically enhance the existing and affected stream channel. To address this goal, four
266 design objectives were envisioned:

- 267 1. Provide a constructed channel environment that will ultimately support enhanced
268 geomorphic and ecologic functions of the creek corridor;
- 269 2. Provide a constructed channel environment that will provide for long-term stability and
270 minimization of risk associated with large-scale channel migration, bank or slope failure,
271 or damage to local infrastructure;
- 272 3. Provide a constructed channel environment that will maintain and improve fish passage
273 conditions; and
- 274 4. Provide a constructed channel environment that will improve flood conveyance
275 performance of the creek corridor consistent with preliminary planning of the new flood
276 control project (Spinks et al., 2010).

277 The primary goal, and objectives were articulated into several design-specific criteria,
278 pertinent to these being development of a mitigation approach that provides for (a) floodplain
279 inundation during floods greater than bankfull; (b) opportunities to effectively store pulses of
280 bedload sediment yet minimize risk of channel avulsion; and (c) high-flow refugia habitat for
281 steelhead according to depth and velocity habitat suitability metrics.

282 c. *Alternatives* – From *Context* and *Feasibility* two mitigation design alternatives were
283 developed, however here we present only the preferred one (Fig. 4).

284 The UPC Project alternative would re-locate the existing channel to the southwest, where it
285 would generally occupy the middle portion of a stream corridor widened by some 50 meters.
286 A riffle and pool channel architecture was proposed, coupled with a high-flow secondary or
287 distributary channel consistent with distal alluvial fan environments. The distinguishing
288 principal behind the primary/secondary channel approach is to provide defined flow paths to
289 modulate sedimentation cycles, and minimize the risk of channel migration through
290 floodplain areas. The proposed primary channel size was designed to convey the 1-year
291 flood, above which the secondary channel is engaged. Combined, the channels will convey
292 the approximate 2-year flood. Fringing the 2 active channels is a floodplain of varied width
293 that would be inundated by floods > 2-year event. The UPC Project alternative would
294 provide an estimated 1.05 acres of new floodplain, numerous large wood structures to
295 provide bank protection and aquatic habitat, a backwater pool/wetland feature to provide
296 high-flow refugia, and numerous bio-engineered measures, including a log crib wall, to

297 protect the outside of meander bends, confluence zones, steep channel bank slopes and points
298 of outfall discharge. Hydraulic roughness associated with planted vegetation (at full growth)
299 was designed to be consistent with flood conveyance goals (Spinks and others, 2010).

300 d. *Analysis* – The UPC Project alternative was analyzed through several different means: (a)
301 geomorphic analysis, (b) 1-, and 2-dimensional hydraulic and sediment transport modeling
302 with the ACOE HEC-RAS (Army Corps of Engineers’ Hydrologic Engineering Center River
303 Analysis System) and CCHE2D (National Center for Computational Hydroscience and
304 Engineering) platforms, respectively, (c) a proprietary log crib wall force balance model, and
305 (d) a proprietary large wood elements force balance and structural model. The hydraulic and
306 sediment transport modeling were conducted on two different platforms to bookend the likely
307 range of hydraulic and sediment transport conditions, and assess comparability of results
308 utilizing two different computational routines and associated simplifying assumptions. The
309 HEC-RAS sediment transport model was utilized to establish general spatial patterns of
310 sediment erosion and deposition, and to fine tune design geometries for the floodplains and
311 the high-flow secondary channels. The CCHE2D sediment transport model supplemented
312 the HEC-RAS modeling and was utilized to provide detailed information on spatial patterns
313 of sediment erosion and deposition.

314 To address the altered, urbanized flow regime of Upper Penitencia Creek, a two-staged
315 channel was proposed: a low-flow channel to convey the more frequent, low-magnitude
316 floods inset within a larger active, secondary channel complex to convey the bankfull flood.
317 This channel form was chosen in order to provide a constructed environment which more
318 closely simulates channel pattern conditions of active alluvial fan surfaces. Site-specific data
319 from Upper Penitencia Creek (Jordan, 2009) suggests that a constructed channel at the
320 project site may evolve to a multi-threaded channel state (Fig 5). In this context it is
321 hypothesized that the UPC Project design concept would provide for channel form stability
322 over the foreseeable future given an absence of major shifts in land-use or climate. The
323 multi-channel form approach also provides defined pathways for channel course shifting at
324 susceptible locations which is assumed to reduce the avulsion risk.

325 Hydraulic analysis using CCHE2D was performed for bankfull, and 5-year flood conditions
326 (Fig. 6). Notable results related specifically to UPC Project objectives and design criteria

327 include the following observations. Bankfull is predicted to fully engage the high-flow
328 secondary channels, and spill onto the adjacent floodplains at multiple locations. This
329 suggests that the UPC Project alternative will flood in a manner consistent with design
330 criteria, and bolster enhancement of channel-floodplain connectivity. Spatial patterns of
331 flooding for the 5-year flood are consistent with the potential benefits observed for bankfull
332 condition. Two-dimensional modeling also suggests that the 5-year flood will provide more
333 complex hydraulics within the low-flow channel and at junctions between the low-flow
334 channel and the high-flow secondary channels. We hypothesize that more complex
335 hydraulics will help to create and maintain aquatic habitat in these locations as velocity
336 gradients, etc. provide multiple types and scales of habitats for aquatic organisms.

337 HEC-RAS sediment transport analysis was carried out for two different hydrologic scenarios:
338 a 3-day duration bankfull flood and a 12-day duration stacked flood hydrograph defined by
339 the 1-year flood, followed by the 5-year, the bankfull and ending with the 1-year flood. We
340 assembled the stacked flood event to evaluate the magnitude to which sediment transport
341 pattern predictions are skewed towards the 5-year flood. The CCHE2D sediment transport
342 model considered only one flow scenario, that being a bankfull hydrograph of 24-hour
343 duration.

344 The HEC-RAS sediment transport results (Fig. 6) suggest that (a) the reach from sections
345 900 to 675 is likely to be depositional in the post-construction period-even if the predicted
346 volumes are thought to be conservative; (b) the remainder of the project reach will likely
347 function with relatively minor amounts of erosion and deposition, with the potential, however
348 for enhanced pool scour along the upstream 90 degree meander bend, which is desired in
349 order to maintain pool habitat for migrating steelhead; (c) minor amounts of sediment
350 deposition could occur during frequent annual flows; and (d) larger floods will accentuate
351 predicted patterns of sediment transport, deposition and erosion perhaps necessitating some
352 level of corridor sediment maintenance at frequencies governed by the occurrence of larger
353 floods. The need for sediment maintenance is dependent in this case on performance of the
354 multi-channel design in relation to upstream sediment supply.

355 In relation to the HEC-RAS results, CCHE2D predicts a similar pattern of deposition and
356 erosion along the project reach for the bankfull flood, with a clear focus of predicted

357 sediment deposition within the upstream meander. The later result is consistent with the
358 HEC-RAS results for both hydrologic scenarios, and thus increases confidence in the
359 prediction. Importantly however, the refined spatial view provided by CCHE2D suggests
360 that deposition through the upstream meander is focused along the outer bank of the channel
361 rather than across the active channel width, and characterized by the suspended load fraction,
362 which may drive development of a natural levee (Fig. 6). Possible levee development is
363 associated with the specified high roughness for the meander, as well as non-erodible bank
364 conditions, given proposal of fortifying log structures and wrapped fill lifts through the
365 meander. Predicted zones of erosion are isolated to the upstream-most project segment
366 where flows and sediment leave the incised, urban corridor and enter the UPC Project site.
367 This segment is characterized by a gradual transition to the created floodplain environment,
368 over which distance the sediment transport capacity outpaces the modeled sediment supply.
369 The channel banks through this segment were also modeled as non-erodible due to the use of
370 fortifying log structures and wrapped fill lifts. The predicted degree of bed erosion was
371 deemed not critical as it occurs within a pool, and HEC-RAS suggests markedly less net
372 erosion for broader hydrologic conditions (i.e. more than one hydrograph).

373 e. *Refinements* – The results presented in Figs. 4 and 6 reflect a design geometry selected as
374 final based on numerous hydraulic and sediment transport model iterations completed in
375 order to achieve predicted post-project conditions which are consistent with the mitigation
376 project objectives and design criteria. In particular, preferred alternative channel and
377 floodplain geometries were adjusted to minimize the predicted volume of sediment
378 deposition within the upstream meander, while maintaining hydraulic performance goals
379 such as full engagement of the secondary channel segments at the morphologic bankfull, and
380 channel-floodplain connectivity above this flood condition. Of the approximate 8-month
381 design development timeline, roughly 2 months were spent analyzing refined design
382 geometries until a minimum of predicted depositional volumes was achieved through the
383 upstream meander, balanced by secondary channel engagement condition at the morphologic
384 bankfull. Pertinently for the present discussion, implementation of the first design geometry
385 iteration for the preferred alternative was predicted to possibly drive channel avulsion to the
386 south of the upstream meander under the morphologic bankfull. This scenario underscores

387 the usefulness of the *CFAAR* framework and specifically honest use of the refinements when
388 results are not consistent with the enhancement vision.

389 **Concluding Remarks**

390 *CFAAR* offers the river restoration community an opportunity-oriented design-
391 development framework, which regulatory agencies can adopt as a standard to guide project
392 visioning, communication, assessment and permitting under non-ideal circumstances. *CFAAR*
393 provides the framework needed to realize new efficiencies and transparency in the stakeholder
394 review, and regulatory permitting process for unplanned opportunities, and usefully
395 complements the self-healing paradigm (Kondolf, 2011) and the Adaptive Management
396 approach to river restoration (Jacobson and Berkley, 2011; Smith, 2011). This in turn may
397 promote wider participation in river restoration by landowners and project proponents who have
398 thus far chosen not to participate in such endeavors. Broader involvement in river restoration
399 may provide the critical mass necessary to overcome the continued degradation of freshwater
400 ecosystems. Perhaps most importantly though, use of *CFAAR* will further the call to
401 conceptualize design approaches which respect the river system, not just a particular location
402 (Kondolf 2006, Beechie et al. 2008, Beechie et al. 2010), and suitably improve the chance of
403 restoring physical and ecological functionality to river systems.

404

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412 **References**

- 413 Beechie T, Pess G, Roni P. 2008. Setting river restoration priorities – a review of approaches and
414 a general protocol for identifying and prioritizing actions. *North American Journal of*
415 *Fisheries Management* 28: 891-905.
- 416 Beechie T, Sear D, Olden J, Pess G, Buffington J, Moir H, Roni P, Pollock M. 2010. Process-
417 based principles for restoring river ecosystems. *BioScience* 60(3): 209-222.
- 418 Bernhardt ES, Palmer MA, Allan JD, Alexander G, Barnas K, Brooks S, Carr J, Clayton S,
419 Dahm C, Follstad-Shah J, Galat D, Gloss S, Goodwin P, Hart D, Hassett B, Jenkinson R,
420 Katz S, Kondolf GM, Lake PS, Lave R, Meyer JL, O'Donnell TK, Pagano L, Powell B,
421 Sudduth E. 2005. Synthesizing U.S. river restoration efforts. *Science* 308: 636-637.
- 422 Buchan LAJ, Leidy RA, Hayden MK. 1999. Aquatic resource characterization of
423 Western Mt. Hamilton Stream fisheries. Report prepared by Eisenberg, Olivieri & Associates
424 in association with United States Environmental Protection Agency for The Nature
425 Conservancy, Sunnyvale, California.
- 426 CDFG. 2009. Part VII: Fish passage design and implementation. In *California Salmonid Stream*
427 *Habitat Restoration Manual*.
- 428 Chartrand S.M. 2011. Feasibility Study and Design Basis Memorandum for Mitigation Project -
429 Upper Penitencia Creek at Berryessa Campus. Report prepared by Balance Hydrologics for
430 Kimley-Horn and Associates and the Silicon Valley Rapid Transit Program.
- 431 Darby S, Sear D, eds. 2008. *River Restoration – Managing the Uncertainty in Restoring Physical*
432 *Habitat*. United Kingdom: John Wiley & Sons Ltd.
- 433 Doll BA, Grabow GL, Hall KR, Halley J, Harman WA, Jennings GD, Wise DE. 2003. *Stream*
434 *Restoration: A Natural Channel Design Handbook*. NC Stream Restoration Institute, NC
435 State University.
- 436 FISWRG. 2004. *Stream Corridor Restoration - Principles, Processes, Practices*. Federal
437 Interagency Stream Restoration Working Group (FISRWG). GPO Item No. 0120-A; SuDocs
438 No. A 57.6/2:EN 3/PT.653. ISBN-0-934213-59-3.
- 439 Grossinger R, Askevold R, Striplen C, Brewster E, Pearce S, Larned K, KcKee L, Collins J.
440 2006. Coyote Creek watershed historical ecology study: Historical condition, landscape
441 change, and restoration potential in the eastern Santa Clara Valley, California: A San
442 Francisco Estuary Institute (SFEI) Publication #426 prepared for the Santa Clara Valley
443 Water District.

- 444 Jacobson RB, Berkley J. 2011. Conceptualizing and communicating ecological restoration. In
445 Simon, A. et al (eds) Stream Restoration in Dynamic Fluvial Systems: Scientific Approaches,
446 Analyses, and Tools. American Geophysical Union, Washington DC.
- 447 Jelks HL, Walsh SJ, Burkhead NM, Contreras-Balderas S, Diaz-Pardo E, Hendrickson DA,
448 Lyons J, Mandrak NE, McCormick F, Nelson JS, Platania SP, Porter BA, Renaud CR,
449 Schmitter-Soto JJ, Taylor EB, Warren ML. 2008. Conservation status of imperiled North
450 American freshwater and diadromous fishes. *Fisheries* 33(8): 372-407.
- 451 Jordan BA, Annable WK, Watson CC. 2009. An urban geomorphic assessment of the Berryessa
452 and Upper Penitencia Creek watersheds in San Jose, California. Report prepared for the
453 Santa Clara Valley Water District (WLA project #1667).
- 454 Leopold LB, Wolman MG. 1957. River channel patterns: Braided, meandering and straight, U.S.
455 Geol. Survey Prof. Paper 282-B, 51p.
- 456 Kondolf GM. 2006. River restoration and meanders. *Ecology and Society* 11(2): 42.
- 457 _____. 2011. Settings goals in river restoration: When and where can the river “heal itself”? In
458 Simon, A. et al (eds) Stream Restoration in Dynamic Fluvial Systems: Scientific Approaches,
459 Analyses, and Tools. American Geophysical Union, Washington DC.
- 460 Menke U, Nijland H. 2008. Flood risk management and river restoration: introduction.
461 Proceedings of the 4th International River Restoration Conference: Chapter 5, Session 4.
- 462 Montgomery DR. 2004. Geology, geomorphology, and the restoration ecology of salmon. *GSA*
463 Today 14(11): 4-12.
- 464 NOAA Fisheries. 2001. Guidelines for salmonid passage at stream crossings. Prepared by
465 NOAA Fisheries Southwest Division.
- 466 Palmer MA. 2008. Reforming Watershed Restoration – Science in Need of Application and
467 Applications in Need of Science. *Estuaries and Coasts*: 32(1): 1-17.
- 468 Palmer MA, Bernhardt E, Allan J, Lake P, Alexander G, Brooks S, Carr J, Clayton S, Dahm C,
469 Shah J, Galat D, Loss S, Goodwin P, Hart D, Hassett B, Jenkinson R, Kondolf GM, Lave R,
470 Meyer J, O’Donnell T, Pagano L, Sudduth E. 2005. Standards for ecologically successful
471 river restoration. *Journal of Applied Ecology* 42: 208-215.
- 472 Palmer MA, Filoso S. 2009. Restoration of ecosystem services for environmental markets.
473 *Science* 325:575-576 (2009).
- 474 SCBWMI, 2003. Watershed Assessment Report. Watershed Management Plan, Volume Two.
475 Report prepared by the Santa Clara Basin Watershed Management Initiative, Palo Alto,
476 California.

477 Saldi-Caromile K, Bates K, Skidmore P, Barenti J, Pineo D. 2004. Stream habitat restoration
478 guidelines: final draft. Co-published by the Washington Department of Fish and Wildlife and
479 Ecology and the U.S. Fish and Wildlife Service. Olympia, WA.

480 Shields FD, Copeland R, Klingeman P, Doyle M, Simon A. 2003. Design for stream restoration.
481 *Journal of Hydraulic Engineering* 129(8): 575-584.

482 Skidmore PB, Thorne CR, Cluer B, Pess GR, Castro J, Beechie TJ, Shea CC. 2010. Science base
483 and tools for evaluating stream engineering, management and restoration proposals. U.S.
484 Dept. Commerce, NOAA Tech. Memo. NMFS-NWFSC.

485 Smith CB. 2011. Adaptive management on the central Platte River – Science, engineering and
486 decision analysis to assist in the recovery of four species. *Journal of Environmental*
487 *Management* 92(5): 1414-1419.

488 Spinks C, Nash WA, Basma H. 2010. Preliminary hydraulic design report—Upper Penitencia
489 Creek at Berryessa Campus. Draft report prepared by Kimley-Horn and Associates for the
490 Silicon Valley Rapid Transit Program.

491 Stillwater Sciences. 2006. Upper Penitencia Creek limiting factors analysis: Final technical
492 report. Consulting report prepared for the Santa Clara Valley Urban Runoff Pollution
493 Prevention Program.

494 USFWS. 1996. Manual 870 FW 1. Published by the Division of Policy and Directive
495 Management.

496 Wilcock PR. 2004. Sediment transport in the restoration of gravel-bed rivers. Proceedings of the
497 ASCE River Restoration and Urban Streams Symposium: 433-444.

498 Wohl EE. 2010. *A World of Rivers: Environmental Change on Ten of the World’s Great Rivers*.
499 University of Chicago Press.

500 World Water Assessment Programme. 2006. *The United Nations World Water Development*
501 *Report 2: Water – A Shared Responsibility*. Paris: UNESCO, and New York: Berghahn
502 Books.

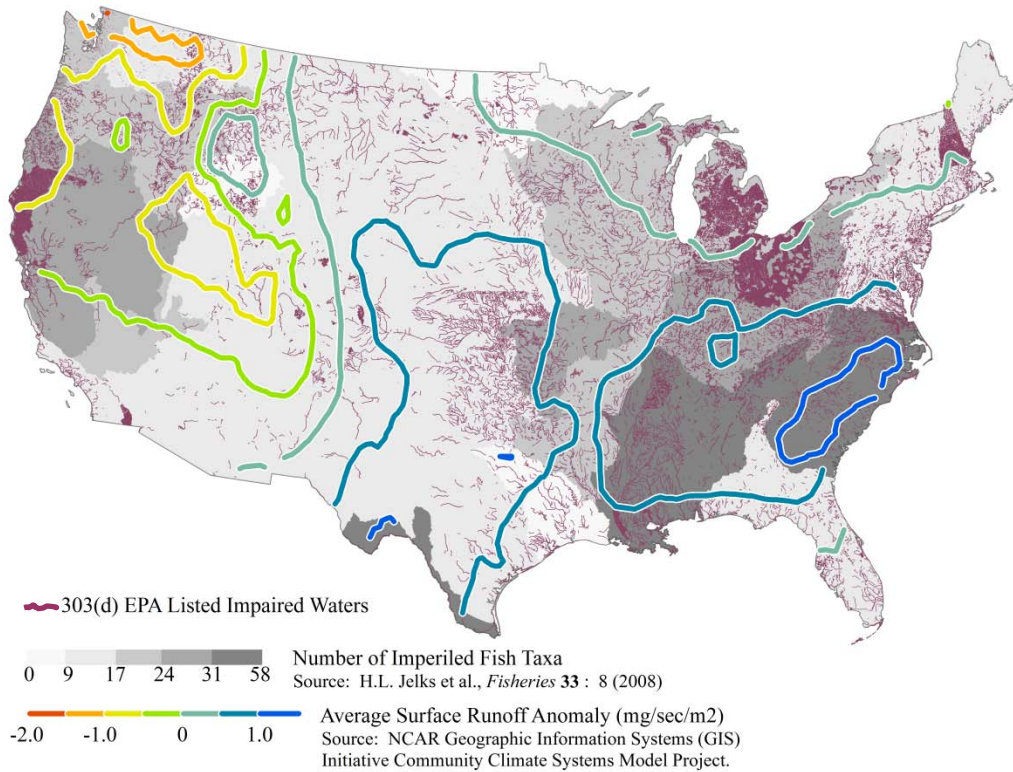
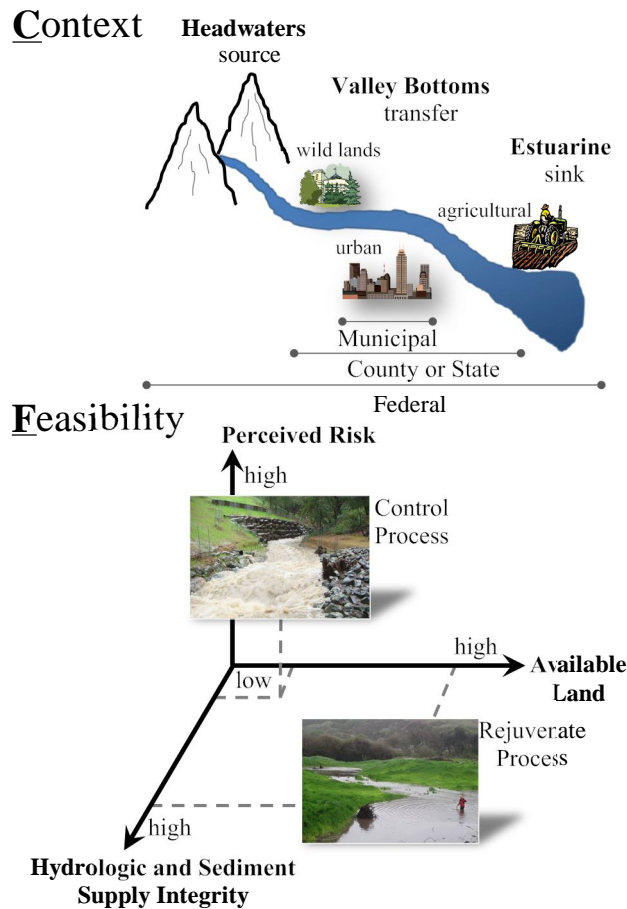


Fig. 1. State of freshwater ecosystems and an uncertain future.

The combination of freshwater impairment due to a variety of pollutants and sediment and the narrowing of freshwater fish diversity highlights that freshwater ecosystems are in poor condition. Predicted changes in runoff volume due to climate change add uncertainty to how observed trends in freshwater ecosystems may unfold, and heightens the need for properly executed ecological restoration. Runoff anomaly represents the average annual difference of the 1979-1999 to 2079-2099 periods under the Intergovernmental Panel of Climate Change’s “SRES B1” emissions scenario. Annual averages were computed from monthly averages for each 20 year period. Imperiled fish taxa data source: Jelks et al. 2008. Climate change data source: NCAR Geographic Information Systems Initiative – Community Climate Systems Model Project.



Alternatives

- Bank & bed stabilization
- Habitat creation & fish passage improvement
- Floodplain creation & reconnection
- Riparian forest re-establishment
- Corridor reconstruction & relocation

Analysis

- Hydraulic & sediment transport modeling
- Design element stability modeling
- Fish passage & habitat analysis
- Ground water/surface water exchange modeling

Refinement

- Refine designs based on results of analysis

Fig. 2. The CFAAR framework

CFAAR is a sequential framework, providing a clear progression for design development from basis of understanding and intent, to final design completion. In this way, restoration design development can move forward in a coordinated and predictable fashion.



Map source: Microsoft, 2010

Map source: Grossinger et al., 2006

Fig. 3. Upper Penitencia Creek Mitigation project site at Berryessa Road, San Jose, CA, USA.

A and B. The modern day Upper Penitencia Creek at the UPC Project site consists of an artificial channel that extends to Coyote Creek. On its course to Coyote Creek UPC runs through a commercial/residential portion of North San Jose and is confined by existing infrastructure and property lines. B. The inset photo of the UPC Project reach illustrates deteriorated channel conditions. C. An historical ecology map prepared by Grossinger et al. (2006) suggests that UPC did not drain directly to Coyote Creek prior to land settlement, but instead drained to a large wetland complex.

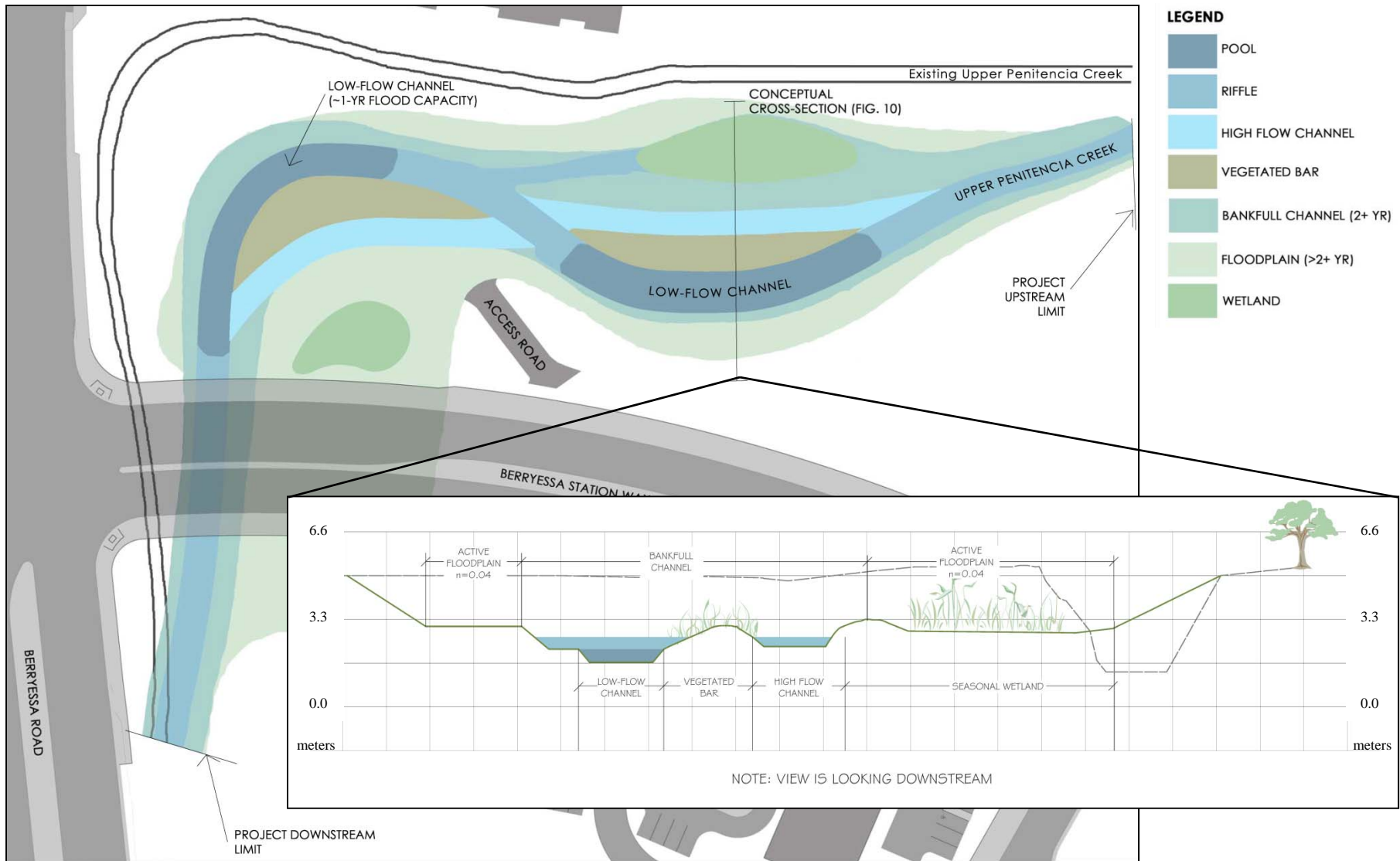


Fig. 4. Design Concepts for the Upper Penitencia Creek Mitigation project site at Berryessa Road, San Jose, CA, USA. The plan view map illustrates the preferred design concept for the UPC Project and the inset image shows the concept in cross-section at the indicated section location. The UPC Project preferred concept is characterized by a multi-channel concept reflective of the alluvial fan setting of the project location. Please note that the vertical and horizontal axes of the inset cross-section are equivalent.

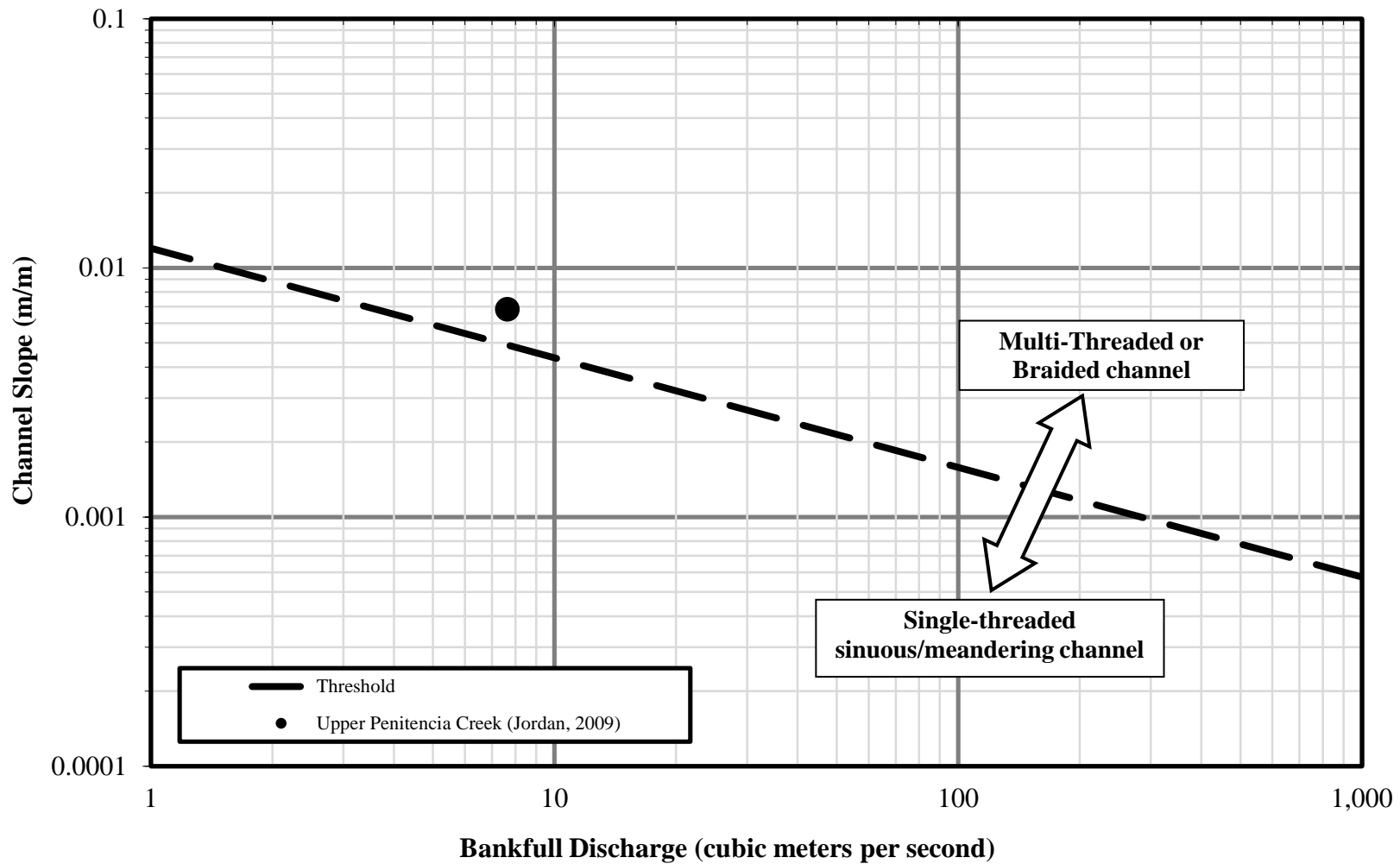


Fig. 5. Geomorphic metrics in relation to channel planform thresholds

Existing slope and bankfull discharge for the UPC Project compared to a threshold for geomorphic channel planforms in alluvial channels (Leopold and Wolman, 1957). The preferred alternative adopts a multi-channel concept, consistent with the single-thread/multi-channel threshold.

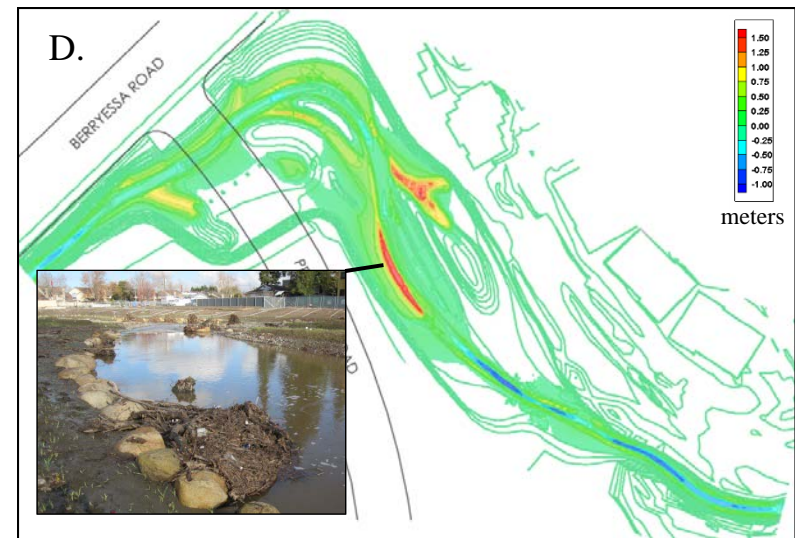
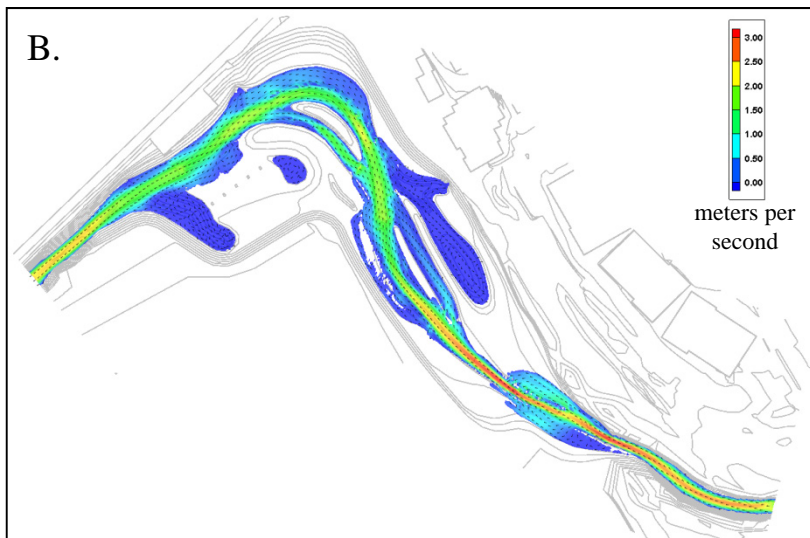
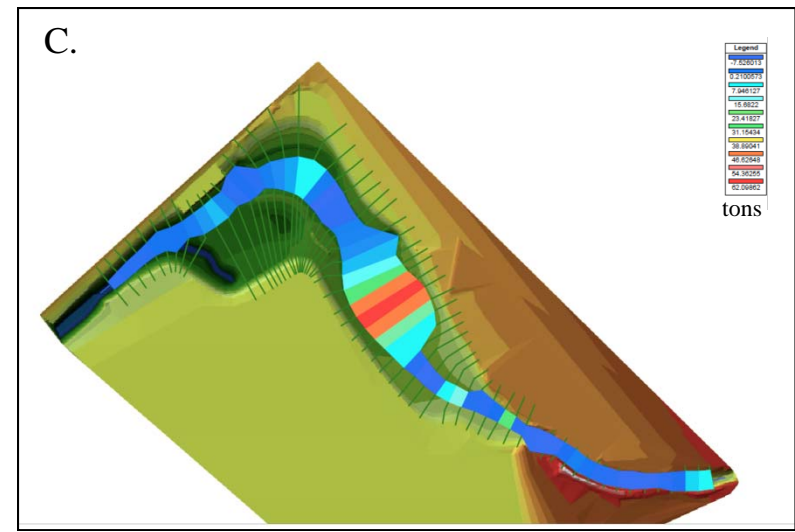
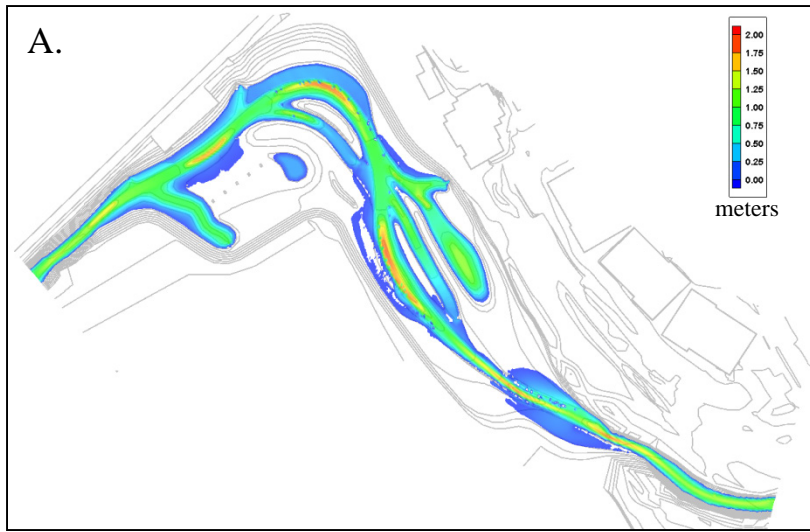


Fig. 6. Hydraulic and Sediment Transport Results, Upper Penitencia Creek at Berryessa Road, San Jose, CA.

A. and B. CCHE2D predicted water depths and flow velocities for the bankfull flood, respectively. C. HEC-RAS predicted sediment transport pattern of erosion and deposition for the bankfull flood. D. CCHE2D predicted sediment transport pattern of erosion and deposition for the bankfull flood; inset photograph shows fine sediment deposition 2 months after construction resulting from a bankfull flood. Please note that the numerical scales in A – D are not equivalent, and that cross-section lines in C vary from ~ 3 to 15 meters apart with cross-section lines ~ 8 meters apart within the zone of deposition.