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ABSTRACT

Many cities and settlements are organized around alluvial rivers, which are self-formed channels composed of gravel, sand and mud. Much of the time alluvial river channels are oversized, in that they could accommodate greater water flow; yet during extreme storms they are woefully undersized, and potentially catastrophic flooding can occur. Considering widely varying hydroclimates, sediment supply, geologic constraints and varying vegetation, it is not altogether obvious that rivers should achieve an average channel geometry that is pattern stable – let alone predictable from theory. Yet, natural rivers follow remarkably consistent hydraulic-geometry scaling relations, that are reproduced in laboratory experiments. Starting with the constraint that channel formation requires that fluid stress exceeds the threshold for sediment entrainment, we review the explanatory power of threshold channel models. Moreover we explore how deviations from threshold channel theory relate to higher-order dynamics of fluid and sediment transport — essentially perturbations to the threshold state — clarifying misconceptions regarding model applicability. Finally, we demonstrate the utility of the threshold channel framework for understanding channel patterns and responses to variations in external forcing such as hydroclimate and land use. Accurate field determination of the entrainment threshold itself is a notorious problem, and emerges as a central challenge in further development and application of threshold channel theory.

Key points:

- The size and shape of a river channel both control and adjust to the flow of water and sediment within it, with consequences for flooding and ecological habitat.
- The question of what determines river-channel size and shape galvanized a quantitative revolution in geomorphology and its applications through “hydraulic geometry scaling”, yet there is no generally accepted model.
- We consider the most prevalent models, and lay out the case that rivers are adjusted to the threshold of erosion for the most resistant material lining the channel.
- We demonstrate how the threshold-limited channel model represents a distillation of complex processes and variables, and provide a framework for its application in predicting mean channel size.
- The threshold-limited model may be beneficial for predicting how changes in hydroclimate and land use drive adjustments in the mean geometry of river channels.

Website summary: Alluvial rivers consist of channels formed by erosion and deposition of sediment; they are the continents’ arteries of water, nutrients and commerce. This Review examines how the threshold for sediment entrainment controls the size, shape and dynamics of alluvial rivers.

Introduction

1 The flow of water and sediment across terrestrial landscapes is concentrated in, and organized by, rivers. In this Review we
2 examine the geometry of alluvial rivers, channels for which the bed and banks are composed of **sediment** transported by the
3 river itself. As one traverses from steep mountain streams to the mouths of the world's great rivers, alluvial channel parameters
4 span a staggering range of scales: slopes (S) decrease from 10^{-1} to 10^{-6} ; widths (W) increase from decimeter (10^{-1} m) to
5 kilometer scale (10^3 m); channel-filling water discharge increases by over nine orders of magnitude ($10^{-4} - 10^5$ m³/s); and
6 bed and bank sediments decrease from boulder (10^0 m) to clay (10^{-6} m). Alluvial river formation can involve comparably
7 large space and time scales: from the **entrainment** of a single sediment grain by a turbulent burst or particle collision^{1,2}, to the
8 evolution of continental-scale drainage networks and basin filling in response to climatic and tectonic forcing over millions of
9 years^{3,4}.

10 It is difficult to overstate the importance of alluvial rivers to the evolution of the Earth's surface and its inhabitants. On
11 geologic timescales, alluvial rivers propagate and filter signals of climate and base level (e.g. sea level) change between
12 sediment sources and sinks⁵⁻⁸. Alluvial rivers' stratigraphic signatures record the birth and demise of ancient mountain ranges,
13 seas and basins^{4,9-11}. Alluvial rivers are bioreactors that modulate nutrient and carbon cycles¹²⁻¹⁴, and networks for the flow
14 of biomass. They have historically been a cornerstone of societal development because they provide reliable water sources,
15 fertile soil to surrounding floodplains, shipping routes, hydropower, recreation, and habitat for aquatic species¹⁵⁻¹⁹. Living
16 along rivers, however, comes at a cost. Catastrophic flooding destroys lives, crops and infrastructure^{18,20}. Land-use changes
17 associated with urbanization and agriculture – including the storage of water in reservoirs for energy production, flood control,
18 and irrigation purposes – have drastically altered the delivery of nutrients, water and sediment to alluvial rivers²¹⁻²⁵. The
19 resulting erosion, flooding, and water-quality impairment have propelled a vicious cycle of damming, armoring and restraining
20 rivers, which requires ever-increasing investments in mitigation and restoration projects^{26,27}.

21 The challenges above lead to two central questions that helped to galvanize a quantitative revolution in **fluvial geomor-**
22 **phology** more than a half century ago²⁸: what determines the size of a river; and how is this size characterized? Two key
23 principles developed to answer these questions: 'hydraulic geometry scaling'²⁹ and 'geomorphic work'³⁰ established the basis
24 for relating **bankfull** channel geometry (width, depth and slope) and planform pattern³¹ to a 'characteristic' discharge³². The
25 commonly observed power-law relations^{29,33-38}, compiled from measurements around the world, have been taken to suggest
26 that alluvial river size is determined primarily by hydraulic conveyance (Fig. 1). Debates have ensued, however, regarding both
27 the **universality** of the scaling exponents and their meaning; vegetation, cohesive banks, hydroclimate, flow resistance and
28 other regional variations have been reported to influence hydraulic geometry scaling relations³⁹⁻⁴³. This variation has been
29 reduced, and physical insight gained, by recasting the observations in **dimensionless** form^{34,44}. Yet the empirical relations
30 alone have limited predictive power, and do not reveal the organizing principle(s) of alluvial rivers. They are suitably robust,
31 however, to have tempted the development of several simplified and generalized models.

32 Early research linked fluid mechanics with alluvial channel geometry⁴⁵⁻⁴⁷ through the development of flow resistance
33 relations in threshold canals, designed to convey water while never exceeding the entrainment threshold. Building on canal
34 theory^{46,48,49}, a family of models has been advanced in which sediment transport is formally treated as a mathematical
35 perturbation to the threshold state⁵⁰⁻⁵². Though different in detail, these models indicate that alluvial rivers at bankfull organize

36 their geometry such that fluid shear stresses at the channel center only slightly exceed the entrainment threshold. These “near-
37 threshold models” are physically rational, and appear to explain the first-order trends in hydraulic geometry of alluvial rivers —
38 providing an explanation for how alluvial rivers can transport sediment without destabilizing their banks^{44,52-54}. This does not,
39 however, indicate they are generally accepted. Researchers have presented evidence for a wide range of fluid stress states in
40 alluvial rivers that appear to contradict the near-threshold condition⁵⁵⁻⁵⁷. Evidence of apparent deviation from near-threshold
41 conditions has been attributed to factors not considered within the model: sediment supply, bed grain-size properties, vegetation,
42 cohesive banks, and the influence of extreme events⁵⁸⁻⁶⁵. Yet, others have observed that such discrepancies may arise from
43 mischaracterization of the threshold condition and the near-threshold model itself^{6,52,66}. Alternative models for hydraulic
44 geometry have proliferated, based on: optimization of sediment transport⁶⁷⁻⁷¹; feedback between flow resistance and channel
45 form^{72,73}; and geotechnical stability of river banks⁷⁴.

46 For the practitioner of river science or management, it is not obvious how to reconcile disparate claims regarding the veracity
47 of these models. For the researcher, it may be unclear what kinds of data are needed to discriminate among competing models.
48 Moreover, with the advent of increasingly sophisticated **morphodynamic** models that are capable of simulating flow and
49 sediment transport in three dimensions (3D)^{75,76}, one may rightly question the need for a simplified model of average channel
50 geometry⁷⁷. We contend, however, that a physically-based and simple model for alluvial channel geometry is an indispensable
51 tool for understanding and managing rivers. Such a model can give insight on the most important controlling variables, provide
52 a benchmark for numerical simulations, serve as an initial design criterion for channel restoration, provide first-order river
53 geometry predictions in the absence of high-resolution topography or in headwater streams where topographic data resolution
54 limits currently exist, predict channel response to changes in land use and hydroclimate, and be easily implemented in landscape
55 evolution and conservation models.

56 A viable theory for alluvial channel geometry must: (i) explain, to leading order, the shape of channel cross sections; (ii)
57 explain how the channel maintains this state under natural (highly stochastic) forcing; and (iii) directly link channel shape to the
58 mechanics of sediment transport. While a multitude of models are capable of predicting channel cross-section geometry, only
59 the near-threshold model has been demonstrated to achieve all three criteria^{6,51,52}. This Review examines the physical basis
60 and surprising explanatory power of the near-threshold model, and attempts to clarify misconceptions regarding its formulation
61 and application to natural rivers. We first describe the nature and extent of hydraulic geometry scaling. We then introduce
62 the family of near-threshold models, and compare them to other alternative frameworks. We demonstrate the application of
63 the near-threshold model to dynamic rivers, and outline current challenges in validation and prediction. Finally, we examine
64 alluvial river responses to changes in hydroclimate and land use through the near-threshold lens. In staking out the scope of this
65 Review, we point the reader toward previous studies that we view as complementary and informative, on the following topics:
66 sediment transport⁷⁸; channel morphology and morphodynamics^{77,79}; a review of hydraulic geometry³⁷; and river restoration
67 and management practices^{27,80,81}. Within this Review we do not consider bedrock rivers, which require a phase transition —
68 breaking or dissolving rock — for channel formation and evolution^{82,83}. Strong similarities exist between alluvial and bedrock
69 river hydraulic geometry⁸⁴⁻⁸⁷, indicating that this Review may be of interest for those studying the role of rivers in setting the
70 pace and style of mountain erosion⁸⁸.

71 **The basis for hydraulic geometry scaling**

72 **Orders of channel behavior**

73 We first propose a conceptual framework to organize the patterns and dynamics of alluvial rivers within a hierarchy of models,
74 in terms of their order of complexity. We relate this hierarchy of channel behaviors to the order of approximation of the fluid
75 and sediment transport equations. Models developed for one order often, by necessity, neglect processes and behaviors at
76 other orders (Fig. 2). A zeroth-order model for alluvial rivers addresses the questions of existence and stability; under what
77 conditions does a river form and what end state does this produce? Linear stability theory can be used to predict the onset and
78 initial scale of channel formation^{89–91}. Because higher-order interactions between perturbations are neglected, these models
79 cannot describe the nonlinear feedbacks that ultimately stabilize channels under the imposed boundary conditions of sediment
80 and water fluxes. We consider a first-order description of alluvial rivers to be the average geometry (width, depth, slope) and
81 grain size; thus, this corresponds to the first moment in statistical distributions of these variables. We posit that a first-order
82 (and essentially 1D) model for flow and sediment transport can predict first-order features, while placing no constraint on the
83 nature of variation around the mean⁵². Second-order patterns in alluvial rivers are commonly driven by secondary flows⁹²
84 where deviations from straight-channel configurations and spatial oscillations within channel geometry cause streamlines to
85 follow curvilinear paths generating secondary currents; 2D vertical or horizontal variations in fluid stress contribute to and are
86 influenced by bed morphology (dunes and bars), which may preferentially sort sediment⁹³. We hypothesize that the second
87 moment in the distributions of width, depth, slope and grain size becomes relevant for these patterns. Models developed to
88 describe second-order patterns and flows, such as meander growth models^{94–98}, typically fix first-order patterns such as mean
89 channel geometry. Finally, we suggest that a third-order description (Fig. 2) of alluvial rivers corresponds to a 3D treatment of
90 the flow and sediment transport fields. At present such a treatment is not analytically tractable, and thus the relevant models are
91 full 3D numerical simulations^{99,100}.

92 **Bankfull Hydraulic Geometry**

93 “Bankfull Hydraulic Geometry”²⁹ is the first-order description of alluvial channels that we examine here. This describes the
94 average width (W_{bf}), depth (H_{bf}) and surface slope (S) of the flow associated with a discharge (Q_{bf}) that fills the channel to the
95 top of its banks (Fig. 1c; Box 1). One reason for this choice is that it provides a relative reference point for comparison of cross
96 sections among different rivers, or for the same river at different locations downstream²⁹ (Fig. 1a-d). Another reason is that
97 bankfull flows typically activate channel dynamics through significant sediment transport (see below; Box 1), and therefore
98 they are relevant for setting the size and shape of the river^{30,34,36}. Traditional “Hydraulic Geometry Scaling” refers to the
99 observed power-law relations between bankfull geometry and discharge: $W_{bf} = a_W Q_{bf}^{b_W}$, $H_{bf} = a_H Q_{bf}^{b_H}$ and $S = a_S Q_{bf}^{b_S}$, where
100 the coefficients a and exponents b are determined from empirical fits to data. Decades of data compilations from surveyed
101 river cross sections^{29,34,38} have firmly established the existence of power-law trends (Fig. 1e), but also found variations in the
102 reported exponents across different physiographic provinces⁴³. This indicates that the set of variables considered provides an
103 incomplete description, and that a physically-informed non dimensionalization of the problem may reduce scatter.

104 Channel formation requires entrainment of bed and bank material by a gravity-driven flow, suggesting that the following
105 additional parameters should be considered at a minimum: median grain size of mobile bed material, D_{50} ; relative submerged
106 density, $R = \rho_s/\rho - 1$ where ρ_s is sediment density and ρ is fluid density; and acceleration due to gravity, g . The dimensionless

107 discharge is $Q_* = Q_{bf}/\sqrt{RgD_{50}^5}$, and the dimensionless hydraulic geometry scaling relations become^{34,44,53}:

$$108 \quad W_{bf}/D_{50} = \alpha_W Q_*^{\beta_W}, \quad H_{bf}/D_{50} = \alpha_H Q_*^{\beta_H}, \quad S = \alpha_S Q_*^{\beta_S}. \quad (1)$$

109 The dimensionless relations (Eq. 1) indeed collapse a significant portion of the scatter (Fig. 3), and are therefore utilized here.

110 Numerous compilations of data have been reported that may be used to fit and generally validate Eq. (1)^{55,56,66}. A
111 consideration of the nature of these data is in order. Data compilations of hydraulic geometry typically report, for each site,
112 average values of bankfull depth and width from a few cross-sectional surveys, a longitudinal profile of the river bed, and
113 a statistically random sampling of the river-bed grain-size distribution¹⁰¹. Variation and ambiguities exist in methods for
114 determining bankfull conditions as topographic indices are not always prevalent or multiple indicators exist^{102,103}; the data used
115 here generally represent river reaches where morphological breaks in channel cross section indicate the bankfull conditions. An
116 under utilized source in such compilations is the vast network of stream gages maintained by the United States Geological
117 Survey (USGS), in which ‘stage’ and discharge are determined for a wide range of flows in surveyed channel cross sections.
118 Stream gages are intentionally placed at locations where channel geometry is relatively ‘simple’ and stable^{104–106}, typically
119 precluding **braided rivers** and meander bends; such preclusion may introduce some bias into measured geometries. However,
120 the bias towards stable cross sections indicates that these data may be perfectly suited for determining the first-order channel
121 behavior. To date, USGS gages represent the largest quality controlled publicly-available database for examining hydraulic
122 geometry and hydroclimate^{107–109}. Accordingly, observations from North America, and particularly the USA, are typically
123 over represented in ‘global’ alluvial river hydraulic geometry compilations⁶⁶.

124 Here we follow recent work and utilize a compiled database of 1,662 cross sections¹¹⁰ from throughout various river
125 networks built primarily on USGS gages and independent studies of river processes, while incorporating to a lesser extent
126 natural rivers from outside the USA and a subset of laboratory experiments^{52,53}. Slope and grain size are not reported within
127 the river transect measurements, and must be determined from complementary reports and/or independent studies that may
128 utilize differing methodologies. Accordingly, we expect some irreducible degree of scatter from indeterminate methodological
129 error and natural heterogeneity. Despite these shortcomings, the observed trends for dimensional and dimensionless width
130 and depth are nonetheless remarkably clean across the entire range of Q_* in the database (Fig. 1e and 3a). Given the small
131 variation in R for natural rivers (and constant g), these data indicate that the width and depth of rivers are strongly determined by
132 hydraulic conveyance. Slope, however, is different: its correlation with Q_* is much more scattered and, moreover, gravel- and
133 sand-bedded rivers separate into two distinct clouds (Fig. 3c). These data require an additional variable, beyond discharge and
134 grain size, to account for observed slope. It has been proposed that sediment supply, Q_s , is the missing factor^{5,34,111}; however,
135 this parameter is rarely reliably reported as it remains difficult to measure. Others have suggested that the timescale for slope
136 adjustment is very long (compared to width and depth) due to the large volume of sediment that must be reworked⁷⁷, and thus
137 that the scatter reflects a lack of stationarity in slope. Any further advance in interpreting the hydraulic scaling relations (Eq. 1)
138 requires a model.

139 The importance of threshold: A minimal model for river channel geometry

140 Sediment transport as a perturbation to the threshold state

141 Alluvial river channels are a consequence of the feedback between flow and form: the form of a river determines the flow
142 within it under an imposed discharge, but the time-integrated effects of flow — and the associated sediment transport — sculpt
143 channel form. The formulation of an elementary model for hydraulic geometry rests on three key principles. First is separation
144 of scales: fluid and sediment transport adjust to channel form rapidly, while channel form adjusts slowly to transport. This
145 allows for quasi-steady and quasi-uniform flow assumptions for estimating the fluid boundary stress (τ , see Box 2), that greatly
146 simplify the problem¹¹². Second is the presumption of **stationarity**: river channels achieve a stable geometry in a statistically
147 averaged sense, and this geometry satisfies the stationary solution of mass conservation — i.e., no net erosion or deposition.
148 Many studies refer to this state as “dynamic equilibrium”¹¹³. Third is the constraint of threshold: a river must entrain sediment
149 locally to form a channel, and a channel will stop evolving if it reaches the threshold entrainment stress τ_c everywhere along its
150 boundary⁵³. The latter state, associated with no sediment transport, is the well known optimal solution for canal design^{33,53,114}.

151 In the limit of no sediment supply, experiments with laminar and turbulent flow demonstrate that channels evolve to a
152 threshold condition with a cross section in the shape of a cosine^{46,48,114} — where fluid and gravitational stresses everywhere
153 on the bed are balanced by friction. This reduces the solution for the stable threshold channel to a hydraulic problem: with
154 expressions for fluid-mass conservation and flow resistance, the shape and slope of the channel can be predicted if one imposes
155 values for: discharge, sediment properties (D_{50} , R) and flow resistance (C_f) (Box 2). In the absence of bed forms, flow
156 resistance arises primarily from grain-scale roughness and hence C_f may be estimated from D_{50} ^{53,114}. Alluvial rivers are not
157 canals of course; they regularly transport their bed sediment, and therefore experience fluid stresses in excess of threshold. Yet
158 many alluvial rivers maintain stable banks (on average), which would appear to require fluid stresses at or below threshold.
159 Gary Parker referred to this problem as the ‘stable channel paradox’⁵⁰, and presciently stated “[such] paradoxes are often
160 resolved in terms of **singular perturbation analysis**”. This suggests that sediment transport may be treated, conceptually and
161 mathematically, as a perturbation to the threshold state; and that the corresponding average stress condition is $\langle \tau \rangle = (1 + \varepsilon) \tau_c$,
162 where $\varepsilon \ll 1$. We refer generically to the model class based on a perturbation approach as the “ $1 + \varepsilon$ model”. Indeed, trend
163 lines in hydraulic geometry scaling of alluvial rivers follow predictions of the threshold channel theory (Fig. 3a), but with an
164 offset that indicates a formative fluid stress that is above threshold⁴⁴.

165 Parker’s⁵⁰ original $1 + \varepsilon$ model built directly on the hydraulic stable canal theory, and assumed ideal conditions including:
166 a straight channel, constant imposed discharge and C_f , and uniform grain size along the bed and banks. It was formulated for
167 gravel rivers, in which sediment moves purely by **bed load**. Parker proposed that lateral diffusion of momentum, from the
168 channel center toward the margins due to turbulence, is the perturbation that solves the stable channel paradox. The solution
169 describes a channel with stable banks ($\tau \leq \tau_c$), and active sediment transport in the channel center ($\tau > \tau_c$). This model predicts
170 a width-averaged formative shear stress $\langle \tau \rangle \approx 1.2\tau_c$, i.e., $\varepsilon \approx 0.2$. It is important to note that the value for ε depends on specific
171 model choices, such as the turbulent closure scheme and flow resistance relation. All reasonable choices, however, would
172 produce a near-threshold channel.

173 In Parker’s formulation, channel geometry is fundamentally determined by hydraulics. An alternative model was recently
174 proposed by researchers from the Institut de Physique du Globe de Paris (IPGP), in which channel shape, under laminar flow

175 conditions, is adjusted to achieve a balance between lateral diffusion of bed-load flux toward the channel margins, and inward
176 sediment motion due to gravity⁵¹. In this formulation, raising the imposed sediment discharge drives increases in channel
177 aspect ratio (W/H) and slope, away from the threshold state associated with no sediment flux (Fig. 3e). Experiments show,
178 however, that a channel will not tolerate a large increase in sediment discharge; instead, a single channel destabilizes into
179 multiple near-threshold threads akin to a braided river (Fig. 3d)^{53, 115–118}. In this manner, the threshold state is like the critical
180 angle of a sandpile¹¹⁹: alluvial rivers can adjust their slope and channel geometry when driven by an imposed sediment load,
181 but they always remain close (i.e., $1 + \epsilon$) to the threshold state.

182 The two $1 + \epsilon$ models above propose different perturbations to the threshold state to explain alluvial rivers. However, both
183 models are founded on the observation that an alluvial river adjusts its cross section to a state in which bed-load transport is
184 accommodated in the active channel center where fluid stress slightly exceeds threshold, and diminishes laterally to zero on the
185 banks. We believe that these models complement, rather than contradict, each other; Parker’s approach neglects lateral bed-load
186 flux arising from a concentration gradient in sediment transport, while the IPGP model⁵¹ neglects lateral fluid-momentum flux
187 arising from a gradient in stress. A more complete solution, which includes both effects, will likely have greater power for
188 predicting alluvial channel shape⁵¹. Nevertheless, the near-threshold constraint that $\langle \tau \rangle = (1 + \epsilon) \tau_c$ is sufficient to close the set
189 of governing equations for a first-order model of channel geometry. As we shall see, this model has surprising explanatory
190 power when applied judiciously.

191 **Modifications and generalizations of near-threshold models**

192 One person’s boundary condition is another person’s model. The near-threshold models above typically impose the following
193 variables as fixed boundary conditions: grain size, sediment discharge, threshold-fluid stress and flow resistance (among
194 others). In natural rivers, however, all of these parameters – where measured – can and do adjust to achieve a channel geometry
195 consistent with the near-threshold state. Here we briefly summarize relevant studies that explicitly examine these adjustments,
196 allowing generalization of the $1 + \epsilon$ model.

197 The fluid entrainment threshold is typically described by the dimensionless Shields stress, representing the ratio of fluid
198 forces over the submerged weight of a particle: $\tau_c^* = \tau_c / ((\rho_s - \rho)gD_{50})$. For loose and non-cohesive grains, τ_c^* is primarily a
199 function of near-bed turbulence and its mean value may be estimated from the Shields curve^{120, 121}. Variation in flow resistance
200 can result in apparent changes in τ_c^* , if an appropriate form drag correction is not applied when estimating the boundary
201 stress^{122, 123}. More vexing are the factors influencing the resistance to grain motion — not accounted for in the Shields curve —
202 that can significantly alter τ_c^* in ways that are difficult to predict. Among these are: bed slope effects^{124–127}; bed compaction
203 and sediment structures/morphology^{128, 129}, particle shape and size distributions^{130, 131}, and cohesion^{132–137}. Challenges in
204 determining τ_c^* , and their contributions to uncertainty in alluvial channel geometry, are described below. Here we summarize
205 a recent approach, however, that shows how the $1 + \epsilon$ model can be generalized to heterogeneous natural rivers — if the
206 entrainment threshold can be determined properly. It is common to observe a significant difference in τ_c between the bed and
207 banks for natural alluvial channels, where the bed is usually composed of sand or gravel and the banks are comprised of cohesive
208 materials (**mud**). While entrainment thresholds for mud vary widely as functions of clay and organic content, temperature,
209 and chemistry^{138–142}, in general gravel ($D_{50} > 1$ cm) has a larger τ_c , and sand ($D_{50} < 1$ mm) has a smaller τ_c , than naturally
210 consolidated mud. Dunne and Jerolmack⁵² proposed an extension of Parker’s model that they called the “threshold-limited

211 channel” model: it posits that alluvial rivers adjust their geometry to the threshold fluid entrainment stress of the most resistant
212 material lining the channel, i.e., $\langle \tau \rangle = (1 + \varepsilon) \tau_{c \max}$. In practice, gravel-bed rivers are adjusted to τ_c of the gravel bed, while
213 sand-bed rivers are adjusted to τ_c of the muddy banks (when present). This empirically validated model explains how sand-bed
214 rivers maintain stable banks, even though boundary shear stresses are far in excess of τ_c for bed material (Fig. 4).

215 The importance of flow resistance, in terms of channel hydraulics and sediment transport, has long been recognized^{45,47}.
216 The boundary stress available to transport sediment is only a fraction of the total fluid stress; the rest, termed **form drag**, is
217 dissipated by turbulence arising from channel roughness at all scales — from grain, to bed form, to bank curvature^{143,144}.
218 Francalanci et al.⁷² proposed a model in which the overall flow resistance of the channel is determined by the coupled solution
219 of the flow in the bank region with the channel center, which results in channel adjustment to the entrainment threshold of the
220 bank material. They showed how transverse undulations in the river bank can modulate the boundary shear stress, and that
221 accounting for this effect improves predictions of hydraulic geometry, allowing a remarkable collapse of the dimensionless data
222 concerning both gravel and sand-bed rivers with cohesive banks. This approach may be considered to be an elaboration of the
223 $1 + \varepsilon$ model.

224 **Alternatives to the $1 + \varepsilon$ model**

225 A distinctly different near-threshold model was recently proposed, wherein river-bank height is limited by the threshold for
226 gravitational collapse⁷⁴. In this scenario, the angle of repose of bank materials — rather than the fluid threshold τ_c — sets the
227 condition for channel adjustment. This model does not attempt to explain the fluid stress or sediment transport states within the
228 channel. Nevertheless it predicts changes in channel geometry as a function of bank cohesion that are similar to expectations
229 from the fluid stress models.

230 A broader and more pervasive class of models, based on "extremal hypotheses", has been proffered as the primary alternative
231 to near-threshold models for explaining hydraulic geometry scaling. There is some physical basis for proposing an extremal
232 hypothesis as a **closure scheme**: often in problems that can be cast in terms of conservation of energy, there is a unique system
233 configuration that minimizes energy or maximizes entropy^{145,146}. In classical physics problems, this configuration may be
234 formally derived from a well-posed mechanical or thermodynamic constraint¹⁴⁷. For rivers, the entrainment threshold is one
235 such mechanical constraint; yet, models that invoke extremal hypotheses do not formally apply this constraint. Researchers have
236 posited that rivers adjust their channel geometry to maximize flow resistance^{67,68}, maximize entropy⁶⁹, or maximize sediment
237 transport^{148,149}. There is, however, no physical basis for predicting this 'optimal' river configuration; one can only assert that
238 the observed state of a river is optimal. Recent developments in the mathematical theory of ramified optimal transport, which
239 seeks solutions that minimize transportation cost¹⁵⁰, may eventually yield a more formal treatment for routing of water and
240 sediment by rivers¹⁵¹ — and, consequently, their associated hydraulic geometry.

241 **Application of the near-threshold model to dynamic rivers**

242 Recent work has shown how the $1 + \varepsilon$ model — which describes an idealized channel with static banks — can also describe the
243 expected (average) channel geometry of dynamic natural alluvial rivers⁵². Correct application of the near-threshold model
244 requires (i) accurate **parameterization** of variables that serve as model inputs, and (ii) appropriate averaging over higher-order
245 behaviors (and their associated statistical moments). At least some of the apparent discrepancies reported between $1 + \varepsilon$ model

246 predictions, and observed hydraulic geometry, appear to be due to error arising from these two issues⁶⁶.

247 **The importance of parameterization**

248 We first consider gravel-bed rivers; based on the threshold-limited channel model, we assume that bank composition may be
249 neglected to first order⁵⁴. The bankfull Shields stress (τ_{*bf}) values in the global database cluster around τ_c predicted using
250 the Shields curve; the scatter around this trend, however, is more than an order of magnitude (Fig. 4a). These data would
251 appear, at first blush, to suggest that some gravel-bed rivers sustain bankfull shear stresses of almost $10\tau_c$ — conditions for
252 which bed material could be suspended — while others fall below the entrainment threshold at bankfull. Hydraulic geometry
253 scaling is correctly predicted by the $1 + \epsilon$ model, but with similarly large scatter around the trend (Fig. 3ab). There is mounting
254 evidence^{6,66} that these discrepancies arise primarily from mis-estimates of τ_c . Determining the threshold entrainment stress
255 is a notorious problem¹⁵²; there is not even a single agreed upon definition of threshold^{152,153}. While it is now well known
256 that the Shields curve is inadequate for many field applications^{125,127,154}, alternative formulations are empirical and have their
257 own issues. For example, widely used empirical relations between τ_c^* and channel slope can produce systematic errors, when
258 compared to *in-situ* estimates of τ_c^* determined from bed-load flux measurements⁶⁶. Using measured (rather than modeled)
259 threshold values, it was found that $\langle \tau_{bf} \rangle = 1.19\tau_c$ — remarkably close to the Parker model solution of $\langle \tau_{bf} \rangle = 1.2\tau_c$. Moreover,
260 scatter was reduced to the range $\tau_c \leq \langle \tau_{bf} \rangle < 2\tau_c$ — indicating that bed material should move exclusively as bed load, in
261 accordance with observations^{66,155}. Unfortunately, measuring τ_c is laborious and error prone. As a consequence, only a small
262 fraction (< 8 %) of gravel-bed rivers in the global database have estimates of τ_c . Nevertheless, this example shows how some of
263 the apparent deviation from the $1 + \epsilon$ model is not due to any shortcoming of the model itself, but rather a consequence of
264 improper parameterization of input variables.

265 Based on the threshold-limited channel model, Dunne and Jerolmack⁵² stated that “the cross-sectional geometry of [sand-
266 bed] rivers is set by the threshold stress of cohesive bank-toe material, which forms the structural anchor of the riverbank”. In
267 this view, the large deviations of sand-bed rivers from threshold — up to $100\tau_c$ of the sandy bed material — do not invalidate
268 the $1 + \epsilon$ model, but instead demonstrate the necessity of characterizing bank materials. *In-situ* measurements of τ_c for cohesive
269 bank-toe materials are, unfortunately, exceedingly rare. Empirical relations between τ_c and silt/clay content can provide only
270 order-of-magnitude estimates, and still require determination of bank-toe material composition^{54,156}. In the few examples
271 where the appropriate τ_c could be measured or estimated, however, observed $\langle \tau_{bf} \rangle$ and hydraulic geometry scaling of sand-bed
272 rivers are in remarkably good agreement with predictions of the $1 + \epsilon$ model⁵² (Fig. 4ab).

273 A related problem is the adequate determination of flow resistance and form drag. Due to the complexities of boundary
274 layer turbulence, form drag must be estimated from empirical relations. Data compilations indicate that flow resistance at
275 bankfull varies by one order of magnitude across a wide range of alluvial rivers⁵⁴. Although this variation is smaller than other
276 factors (Q , D_{50} , S , etc.), assuming a fixed value for C_f introduces significant scatter around the first-order trends in channel
277 geometry^{52,53}. Francalanci et al.⁷² determined that form drag, arising from river-bank macro-roughness elements (bumps)
278 and grains, dissipates 60-70% of the available fluid stress. As a consequence, rivers with stable cohesive banks and mobile
279 beds are narrower and deeper than one would expect if form drag were neglected. Francalanci et al.⁷² determined empirical
280 form-drag corrections, that reduced scatter in hydraulic geometry scaling relations. Similar to measuring threshold itself, *in-situ*
281 determinations of form drag for each river would likely improve the agreement of observations with the $1 + \epsilon$ model. Resolving

282 the sensitive dependence of turbulent momentum dissipation on complex boundaries is of fundamental importance for river
283 hydraulics — but is also clearly beyond the scope of a first-order model for hydraulic geometry.

284 One question that arises in the application of the $1 + \epsilon$ model is whether channel slope is an input parameter or a model
285 output. Both the Parker⁵⁰ and IPGP⁵¹ models for gravel-bed rivers derive stationary solutions for channel slope, width and
286 depth as functions of water and sediment discharge. However, solutions for width and depth can be rearranged to depend only
287 on hydraulic factors — and not sediment discharge — if channel slope is imposed as an input parameter (Box 2). Hydraulic
288 geometry data show that width and depth are well predicted by hydraulic conveyance, while the large scatter in slope (Fig. 3c)
289 suggests an additional unmeasured factor — presumably sediment discharge — is required. Another possible factor is time,
290 which of course is neglected in stationary solutions. Sediment transport models, that couple channel geometry to long-profile
291 evolution via sediment mass conservation, predict that the timescale of slope adjustment may be on the order of millenia —
292 much larger than the decadal timescales of width and depth adjustment^{5,77,111,157,158}. This separation of scales suggests that
293 slopes of many natural rivers are not stationary; i.e., they may still be adjusting to modern water and sediment loads. This
294 change may be slow enough, however, to be considered quasi-steady in terms of hydraulic geometry; i.e., width and depth
295 may adjust in lockstep with changes in slope. Practically, this means that on engineering timescales slope should be treated as
296 an input parameter to the $1 + \epsilon$ model⁵²; it is certainly easier to measure than sediment discharge. On geologic timescales,
297 however, alluvial rivers set their own slope through regrading of valleys and meandering.

298 **The importance of averaging**

299 Given a constant imposed water discharge above the entrainment threshold, a channel will develop a (statistically) stationary
300 geometry that just contains this flow^{51,114,159}. Natural rivers, however, experience a wide range of discharges; most are well
301 below bankfull, while occasional floods can be well above⁶. This raises a fundamental question: is bankfull discharge merely a
302 useful reference point for hydraulic geometry comparisons, or is it a channel-forming flow condition with physical significance?
303 The seminal work of Wolman and Miller³⁰ provided an elegant conceptual framework for answering this question. They
304 reasoned that channels are adjusted to the flow of 'maximum geomorphic work': the stress whose product of frequency of
305 occurrence, and intensity of sediment transport, moves the most sediment in the long-time limit. Large floods have high
306 transport intensity but low frequency, while frequent low flows that do not exceed threshold do no work in moving material; it
307 is intermediate stresses, with low transport intensity and moderate frequency, that do the most work in shaping the channel.
308 Empirically, it has been demonstrated that Q_{bf} also corresponds to the stress of maximum geomorphic work; in other words,
309 the bankfull flow indeed appears to generally be the channel-forming discharge^{6,160–162}. How is this achieved? To answer
310 this question one must understand how water discharge is converted into boundary shear stress (Box 1). Discharge may be
311 considered as a forcing condition on the river, determined by hydroclimate and drainage area. The frequency-magnitude
312 distributions of river discharge, determined from long term gaging stations, show immense variation across climatic gradients,
313 whereas the choice of bed-load transport equation produces less error in the estimate of the effective discharge¹⁶³. In temperate
314 rivers, discharge distributions are typically thin tailed, and Q_{bf} has a recurrence interval of 1-2 years; while in arid regions
315 discharge distributions can be heavy-tailed, and the recurrence interval of Q_{bf} may be considerably longer¹⁶⁴. Since flows
316 below the entrainment threshold do not modify channel geometry, one must consider only the distributions of fluid stresses
317 exceeding critical for the most resistant material (i.e., $\tau > \tau_c$). These distributions show a remarkably different behavior from

318 discharge; they follow a thin-tailed distribution often well-described by an exponential function whose average value coincides
319 with the bankfull discharge^{6,52,165}. This occurs because the boundary stress that results from an imposed water discharge is
320 determined by channel shape and flow resistance; i.e., Q is imposed by watershed hydrology, but τ is an intrinsic property of the
321 channel. For flow within the channel ($Q < Q_{bf}$) we expect that flow depth, and hence τ , increases nearly linearly with Q . Once
322 Q exceeds Q_{bf} , however, flow spreads across the floodplain and τ increases much more slowly with Q (Box 1). This results in
323 a rapid decline in the frequency of high stresses as flows exceed bankfull. The threshold constraint on channel organization is
324 central here: river banks destabilize, and widen the channel, with increases in boundary stress above threshold — producing a
325 negative feedback that keeps the channel in a near-threshold state¹⁵⁹. The transformation of widely varying discharges into
326 a common thin-tailed distribution of excess shear stresses has been termed the ‘critical filter’⁶. It is a logical consequence
327 of the organization of alluvial rivers to a near-threshold state, and justifies the use of a single bankfull discharge value in the
328 application of the $1 + \varepsilon$ model for hydraulic geometry.

329 The above should not be interpreted to mean that rivers do not respond to flows larger or smaller than bankfull, or experience
330 temporal variations in erosion and deposition⁶⁵. But in the context of hydraulic geometry (Fig. 1e), such variability represents
331 fluctuations about some suitably-averaged, stationary mean state. These dynamics can correspond to large individual floods⁶⁴,
332 seasonal or cyclical variations in flow and sediment supply¹⁰⁹, meander cutoffs, collapse of slump blocks into the channel, and
333 myriad others. To maintain a stable mean geometry, deviations from this state must be compensated by others; and indeed
334 there is emerging field documentation of such compensatory behavior. Sediment transport associated with smaller, frequent
335 floods can act to smooth over perturbations to channel geometry created by large, rare floods^{63,166}. The banks of a meandering
336 river have been observed to migrate independently from each other at the flood to annual scale, but erosion on one bank is
337 counterbalanced by deposition on the other such that river width is constant at decadal timescales¹⁶⁷. These observations help
338 to calibrate our expectation of the temporal averaging required for application of the $1 + \varepsilon$ model. We posit that a reasonable
339 averaging time must include several bankfull flow events, a notion that is supported by recent modeling results¹⁵⁷. For temperate
340 rivers this averaging timescale is on the order of a decade, but could be much longer within arid environments or comparatively
341 shorter in flood-rich rivers^{6,165}.

342 We now consider averaging over spatial variability in channel morphology. Dunes, bars and meander bends create systematic
343 variations in channel width, depth, slope and grain size — variations absent within a first-order hydraulic geometry model.
344 The length scales of these features should inform the spatial scales required for averaging^{52,168}. Despite these sources of
345 variability, a first-order model of channel geometry can still provide useful information. For example, measured channel widths
346 of a meandering river exhibited a wide statistical distribution, but the modal value was well predicted by the $1 + \varepsilon$ model⁵².
347 In the case of braided rivers, a laboratory experiment demonstrated that the average geometry of a thread conformed to the
348 near-threshold model, despite the braided threads’ high mobility and tendency to ceaselessly remold the channels¹¹⁷. Similarly,
349 field observations of a braided river found that the individual threads were, on average, each near-threshold channels¹¹⁸. These
350 examples illustrate the concept that the $1 + \varepsilon$ model can describe the average geometry of alluvial rivers, but says nothing about
351 higher-order dynamics and their contributions to variations about the mean.

352 It is well known that increasing the entrainment threshold of bank materials — whether by vegetation or cohesion — can
353 result in relatively narrower and deeper channels¹⁶⁹, and affect a transition from braided to single-thread (meandering or
354 straight) morphology^{170–172}. This transition is predominantly controlled by channel aspect ratio^{51,53}, through its influence on

355 lateral flow instability^{116,173}. The threshold-limited channel model explains how and why average channel geometry changes
356 with bank material strength. The predicted geometry from the $1 + \epsilon$ model can be evaluated using a classical hydrodynamic
357 stability criterion¹¹⁶, to predict whether one or multiple threads are expected. This approach has been shown to successfully
358 describe the planform pattern of natural rivers^{53,54,118}, and therefore may be useful for channel restoration schemes or predicting
359 potential channel responses to changes in boundary conditions. The near-threshold model could also help to better constrain
360 paleo-hydraulic conditions and channel pattern changes observed in past river deposits, on Earth^{174–176} and other planets such
361 as Mars^{177–180}.

362 **Outstanding problems and opportunities**

363 **Hydroclimate change and timescales of river adjustment**

364 A fundamental question arises when considering the applicability of stationary models for hydraulic geometry: how, and
365 how fast, do channels adjust their shape to changes in hydrology? This question is germane to determination of the requisite
366 averaging timescale, as discussed above; it is also central to predicting channel response to climate and land-use changes.
367 Each river may have its own adjustment timescales and patterns, determined by site-specific characteristics such as catchment
368 morphology, geology and tectonics, hydroclimate, vegetation, land use and engineering conditions^{25,41,109,158,181}. Due to these
369 complexities, the question of channel adjustment must first be addressed empirically. In recent decades, the same USGS gage
370 data discussed above has begun to be utilized to examine changes in cross-sectional geometry and hydraulic conveyance (fig.
371 5a-c)^{107,182,183}. Although not intended for this purpose, long-term gage records — extending decades and, in some cases, over
372 a century — are the best available data for relating channel size to hydroclimate^{184,185}.

373 A general observation is that, to first order, the hydraulic geometry of alluvial rivers is more-or-less adjusted to modern
374 hydroclimate regimes. This can be inferred from the tight scaling relations between bankfull channel width, depth and
375 discharge (Fig. 1e), and the general success of the stationary $1 + \epsilon$ model — which involves averaging timescales on the order
376 of a decade or longer⁶. This result implies that statistically significant changes in hydroclimate — i.e., the frequency and
377 magnitude of discharge events — may be expected to result in detectable changes in hydraulic geometry (Fig. 5 a-d). Indeed,
378 multi-decadal trends in river channel form are widespread¹⁰⁸. An analysis of almost a thousand, minimally-disturbed USGS
379 gauges across the USA found that 2/3 of the stations displayed significant temporal trends in riverbed elevation (Fig. 5), with
380 disproportionately higher rates of change in drier regions¹⁰⁷. Results imply that at least some of these trends may be attributable
381 to anthropogenic-driven climate change, although no formal attribution analysis has yet been performed.

382 The sensitivity of alluvial river geometry to climate change is only just beginning to be explored. Hydroclimatic variability
383 is typically expressed through the presence of “flood rich” and “flood poor” periods¹⁸⁶. Large-scale modes of variability, such
384 as the El Nino Southern Oscillation or the Atlantic Multidecadal Oscillation, alter precipitation and temperature patterns both
385 regionally and globally, over different timescales^{187–189}. Of particular importance for flooding is the change in precipitation
386 and discharge forcing scenarios and the resulting channel response¹⁰⁹ (Fig. 5 e-f). Anthropogenic-driven climate change is
387 currently driving increases in the most intense precipitation events in many regions^{190,191}. Analysis of long-term USGS gage
388 data suggests that alluvial rivers may “breathe” with climate cycles; i.e., increase and decrease their conveyance capacity
389 with flood rich and flood poor periods (fig. 5c), respectively¹⁰⁹. Understanding the nature and pervasiveness of these changes
390 is a burgeoning frontier research area for understanding the links between river geometry and hydroclimate. While some

391 channel adjustment may result principally from changes in the frequency and intensity of sediment transport¹⁹² and therefore
392 be amenable to a mechanics based solution, other factors may also contribute. For example, interannual vegetation growth
393 — along channel banks, on bars, and within the upstream catchment — could contribute significantly both in changing the
394 entrainment threshold, and also by potentially introducing lags and hysteresis in channel response^{193,194}.

395 The considerations above offer some reassurance regarding the utility of the first-order $1 + \epsilon$ model for interpreting observed
396 changes in mean hydraulic geometry as a consequence of climate change. They also, however, underscore a major shortcoming:
397 the assumptions of stationary water and weather regimes are broken¹⁹⁵ and many channels may be adjusting to a changing
398 climate, climatic oscillations, significant shifts in land use, and/or water management (Fig. 5). The close agreement between
399 channel width and discharge (Fig. 1e) indicates that the $1 + \epsilon$ model can be used to predict channel size following adjustment,
400 however the rates and modes of adjustment cannot be predicted from a stationary model. To move forward with an empirical
401 approach, the next logical step is to consider the information contained in the higher-order moments of channel geometry
402 data. In other words, one possibility may be to consider river morphology and hydroclimate as probability density functions
403 that evolve together over time. What would such an approach look like? The cross-sectional river width, for example, can
404 be described as a probability density function¹⁶⁸ that is reflective of such factors as formative discharge and sediment input,
405 variations in threshold along the investigated reach, and additional mechanisms such as slump-block protection. In turn, river
406 discharge can be described as a probability distribution that changes on annual or decadal timescales due to natural climate
407 oscillations, human-induced climate change, water management, or land-use changes. Examination of the joint probability
408 distributions of channel geometry and hydroclimate through time would open the door to statistical modeling of the influence
409 of climate on alluvial rivers.

410 **Land use change and multiple stable states**

411 The $1 + \epsilon$ model provides a framework for anticipating how alluvial channels may adjust to changes in hydrology or land use,
412 through their influence on factors such as the distributions of discharge and sediment flux. While channel adjustment may be
413 complex and involve time lags, we have implicitly assumed that there is a unique, stationary average channel geometry for
414 a fixed set of boundary conditions. We now consider the possibility, however, that there may exist multiple stable states of
415 channel geometry under the same imposed boundary conditions, as a result of landscape history. The pioneering work of Walter
416 and Merritts²⁵ revealed the deep and lasting influence of legacy sediments, from centuries old land-use changes, on modern
417 channel geometry and morphology. Most relevant here is their discovery that many small rivers in Northeastern USA used to be
418 shallow, marshy, multi-channel systems before land clearing and mill dam construction (1600- early 1900s²⁵) induced the rapid
419 filling of these valleys with fine sediments. Over centuries, however, much of these watersheds have reforested while abandoned
420 dams have breached. Modern channels formed by slicing through these fine sediments, and the underlying pre-European
421 colonization marsh muds, until they tapped a substrate of Pleistocene colluvium — cobbles with relatively high entrainment
422 stresses that line the valley bottoms (Fig. 6). Although the strong perturbations to hydrology and sediment supply associated
423 with mill production have been removed, the rivers have not returned to their original form. Rather, they are relatively deep and
424 narrow, meandering channels^{25,196} that appear to be pattern stable (Fig. 6a-c). We posit that both channel states, pre-European
425 colonization and modern, can be understood with the threshold-limited channel model. In this view, the shallow channels of the
426 pre-European colonization (Holocene) era were adjusted to the entrainment threshold of the (vegetated) wetland muds and

427 sands that lined their banks and beds²⁵. Following dam failure and breaching that caused transient and focused erosion through
428 loose legacy-sediment fill¹⁹⁷ and Holocene marsh sediments, these channels adjusted to a new threshold-limiting material
429 — the exhumed Pleistocene cobbles (Fig. 6d). Application of the threshold-limited model provides close predictions of the
430 modern channel width (Fig. 6d). This case study reveals how the history of a landscape, embedded in sedimentary deposits,
431 becomes a substrate that can exert a first-order control on channel geometry through the entrainment threshold. This idea
432 has practical consequences: currently, major restoration projects are underway in some of these Northeastern rivers, based
433 on the premise that channel geometry may be returned to pre-disturbance conditions by altering the substrate. More broadly,
434 dam removal projects are rapidly growing in number around the world, with the similar goal of returning rivers to a natural
435 state^{198–201}. The ultimate success of these projects will also be a measure of the success of the threshold-limited channel model.

436 **Understanding and predicting threshold**

437 If the $1 + \epsilon$ model is at least a sturdy vessel for encapsulating our current understanding of first-order channel patterns, it is
438 anchored to a shifting bottom: the entrainment threshold. For all intents and purposes, τ_c of *in-situ* river sediments — whether
439 gravel beds, or cohesive river banks — cannot be predicted from existing models to better than a factor of ten^{52, 125, 152}. Given
440 our premise that alluvial rivers organize to near-threshold conditions, this suggests that the primary challenge in predicting
441 channel geometry lies in proper determination of threshold itself. Extensive research on gravel-bed rivers has demonstrated the
442 importance of myriad factors that make prediction so difficult: variability in grain protrusion and exposure^{129, 202–204}; granular
443 structure effects including interlocking, armoring and compaction^{128, 153, 205, 206}; spatial segregation or patchiness in grain
444 size^{93, 207}; and the sensitivity of the near-bed turbulent stress distribution to bed topography^{144, 208–210}. For cohesive bank-toe
445 materials the situation is at least as challenging, as τ_c is sensitive to: variations in clay and organic content^{133, 169}; the degree of
446 compaction²¹¹; wetting and drying cycles^{212, 213}; and even water chemistry through its control on particle-surface charge^{141, 142}.
447 The final challenge is that many of the aforementioned factors influencing threshold are spatially and temporally variable.

448 As a result of these challenges, we advocate that researchers and practitioners collect site-specific, *in-situ* measurements
449 of τ_c for the most resistant material. There are currently so few measurements of cohesive bank-toe material that no general
450 trends can be reported⁵²; but our hope is that recent methodological improvements²¹⁴ will allow for broader data collection.
451 For gravel-bed rivers, a variety of techniques have been employed and reviewed elsewhere²¹⁵; but impact plates^{216–218} and
452 seismometers^{219–222} are emerging as tools for high-resolution temporal monitoring of bed-load transport and, by extension, the
453 entrainment threshold. These tools have demonstrated that τ_c is moving target; its value appears to depend on the history of
454 flows experienced by the river²²³. Observations of hysteresis in bed-load transport rate through individual floods have been
455 used to infer that τ_c changes due to reorganization of the river bed^{223–225}. Perhaps more intriguing is that τ_c appears to vary
456 from flood to flood²²⁶, and to evolve through time even under sub-threshold conditions^{129, 223}. Recent laboratory experiments
457 have begun to reveal the grain-scale origins of the temporal evolution of τ_c . Low-intensity bed-load transport and sub-threshold
458 creeping of grains both act to strain harden the bed and increase τ_c , through compaction and reduction in the protrusion of
459 grains at the surface^{129, 153}. Laboratory experiments with a unimodal grain size distribution demonstrate that high-intensity
460 bed-load, however, appears to dilate sediment beds and destroy memory resulting in a decrease in τ_c ¹⁵³.

461 It is beyond the scope of this paper to dive deeper into the origins of variation in τ_c , but this behavior raises challenging
462 questions for near-threshold rivers. Does channel geometry adjust to some time-averaged τ_c integrated over many flood

463 events; or does channel adjustment require severe disruption of the bed structure, in which case the maximum τ_c may be more
464 appropriate? Further, the linkage of τ_c with the frequency and magnitude of flood events suggests that potential changes in
465 hydroclimate may alter the entrainment threshold itself — with knock-on consequences for channel geometry. The critical filter
466 effect of channel geometry on bed-stress⁶, however, may limit the impact of high-magnitude floods on τ_c . Moreover, the fact
467 that τ_c may adjust over a range of values implies a certain buffering capacity; a river may absorb some changes in hydroclimate
468 through reorganization of the river bed (and hence τ_c), without changes in channel geometry. As earlier, here we suggest that
469 adopting a probabilistic description of τ_c is a sensible next step. From a practical perspective, future work must endeavor to
470 determine how — and for how long — to measure τ_c in the field.

471 Conclusion

472 In his seminal paper⁵⁰ introducing the original $1 + \varepsilon$ model, Gary Parker concluded: “Natural rivers are extremely complicated
473 phenomena and it would be facile to assume that the present analysis provides a complete and accurate picture...Regime
474 relations apply only to straight reaches with bed and banks composed of similar non-cohesive coarse gravel at bankfull.” And
475 yet, decades of subsequent data have shown that the regime relations indeed apply to complex natural rivers that flagrantly
476 violate model assumptions. We have attempted to demonstrate, through appropriate parameterization and averaging, how and
477 why this ‘facile’ model also explains the mean state of alluvial river geometry. In doing so, our hope is that this Review also
478 serves as a guide for the practitioner in the proper application of the model to natural and engineered settings. The rich tapestry
479 of higher-order behaviors that make rivers dynamic — dunes, bars and meanders, collapsing banks, growth and erosion of
480 vegetation, and floods — are essentially fluctuations about the mean state. By analogy, the $1 + \varepsilon$ model describes the climate
481 of alluvial rivers, but says nothing about the weather. It is reasonable to assume that the inclusion of these fluctuations will
482 improve hydraulic geometry predictions. This Review concludes, however, that the foremost challenge is to determine the
483 appropriate entrainment threshold. An explosion in field studies characterizing timescales of channel adjustment, and the
484 emergence of probabilistic descriptions of river geometry and hydroclimate, promise the development of future statistical
485 models that will relax assumptions of stationarity. Such models are needed to predict the responses of alluvial rivers to rapidly
486 changing boundary conditions, such as climate and watershed management.

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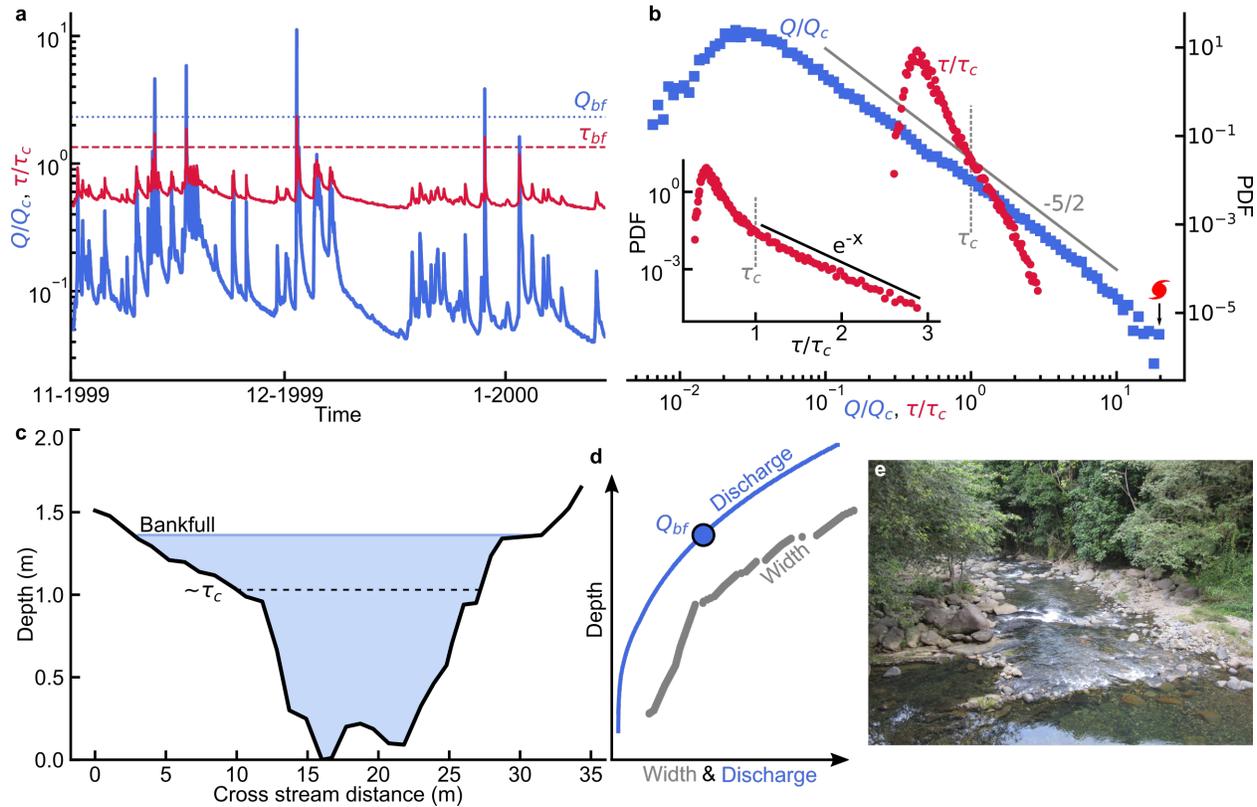
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Box 1: The threshold of motion constrains fluid stress through channel geometry



Box 1. The threshold of motion constrains fluid stress through channel geometry. **a** | Hydrograph for the Mameyes River (USGS gage 50065500) normalized by the threshold of motion. Due to frequent storms and steep topography the Mameyes floods frequently, note the occurrence of four bankfull floods (dashed and dotted lines) within two months during the dry season. **b** | Probability density functions (PDF) for discharge (blue squares) and shear stress (red circles) normalized by the threshold of motion (vertical dashed line) for Water years 1995-2020. The peak in each PDF represents baseflow, values greater than one indicate flows capable of transporting the bed material, and the highest flows are primarily hurricanes (red symbol) at values of $20Q_c$ ($3\tau_c$). Discharge beyond baseflow is well described by a power law with slope of $-5/2$, while shear stress contains a subtle scaling break at approximately τ_c . The inset shows τ/τ_c on a semi-log plot where a straight line represents an exponential function. **c** | Cross section of the Mameyes River^{227,228} with the approximate location of the threshold (τ_c) and bankfull indicated. **d** | Relations between depth and discharge (blue line) and width (gray points). These data share the same vertical depth scale as the cross section. The relation between depth and width is informative in understanding the relation between depth and discharge. Depth increases rapidly initially but gives way to increases in width as the cross section expands. **e** | Photograph of the section of the Mameyes River downstream of the gaging station where the cross section was measured (wetted width is 12 m across).

Box 2: Application of the near-threshold model

Box 2 | Given an imposed water (Q) and sediment (Q_s) discharge, the bankfull geometry of a natural channel can be designed with the threshold-limited model through the following five relations. Conservation of mass for the fluid yields the bankfull discharge:

$$Q_{bf} = U_{bf} H_{bf} W_{bf}, \quad (1)$$

983 where U_{bf} , H_{bf} , and W_{bf} are the bankfull flow velocity, width and depth, respectively. Conservation of momentum under the
 984 assumption of normal flow provides the bankfull shear stress (τ_{bf}) through the depth-slope product:

$$985 \quad \tau_{bf} = \rho g H_{bf} S. \quad (2)$$

986 Velocity is related to shear stress through a flow resistance equation:

$$987 \quad U_{bf} = C_f \sqrt{\tau_{bf}/\rho}. \quad (3)$$

988 Here we present a Darcy-Weisbach flow resistance equation, though one could use any number of relations²²⁹. The coefficient
 989 of flow resistance $C_f = \sqrt{8/f}$, where f is the Darcy-Weisbach friction factor. Sediment flux can be approximated through a
 990 bed-load flux equation:

$$991 \quad Q_s = k(\tau_{bf} - \tau_c)^{3/2} W_{bf}, \quad (4)$$

992 where k is a fitting coefficient and τ_c is the sediment threshold entrainment stress. Similar to flow resistance, a myriad of
 993 bed-load flux equations^{78,230} exist depending on the sediment grain size and distribution. The choice of equation may depend
 994 on the practitioner's situation. The $1 + \varepsilon$ model provides the final relation required to close this set of hydraulic equations by
 995 relating the threshold of sediment entrainment to the bankfull shear stress:

$$996 \quad \tau_{bf} = (1 + \varepsilon)\tau_c. \quad (5)$$

997 For the following derivations, we set $\varepsilon = 0.2$ as it provides good predictions for natural rivers^{6,52,66}. We note, however, that
 998 other values are possible and depend on the formulation for the lateral transfer of downstream momentum and the choice of
 999 flow resistance relation in the derivation of the $1+\varepsilon$ model. These five equations can be rearranged to provide solutions for the
 1000 bankfull width and depth:

$$1001 \quad W_{bf} = \frac{gQS}{C_f \left(\frac{1.2\tau_c}{\rho}\right)^{3/2}} \quad (6)$$

1002 and

$$1003 \quad H_{bf} = \frac{1.2\tau_c}{\rho g S}. \quad (7)$$

1004 The slope of a reach is often considered an imposed condition, however with an imposed water and sediment discharge equations
 1005 (2) and (4) can be rewritten to solve for the river slope:

$$1006 \quad S = \frac{6}{\rho g H_{bf}} \left(\frac{Q_s}{kW_{bf}}\right)^{2/3}. \quad (8)$$

1007 These equations may be an oversimplification of the vast number of variables at play within a river corridor, however they
1008 provide a physically rational set of relations consistent with natural rivers and laboratory channels to estimate the average
1009 channel geometry.

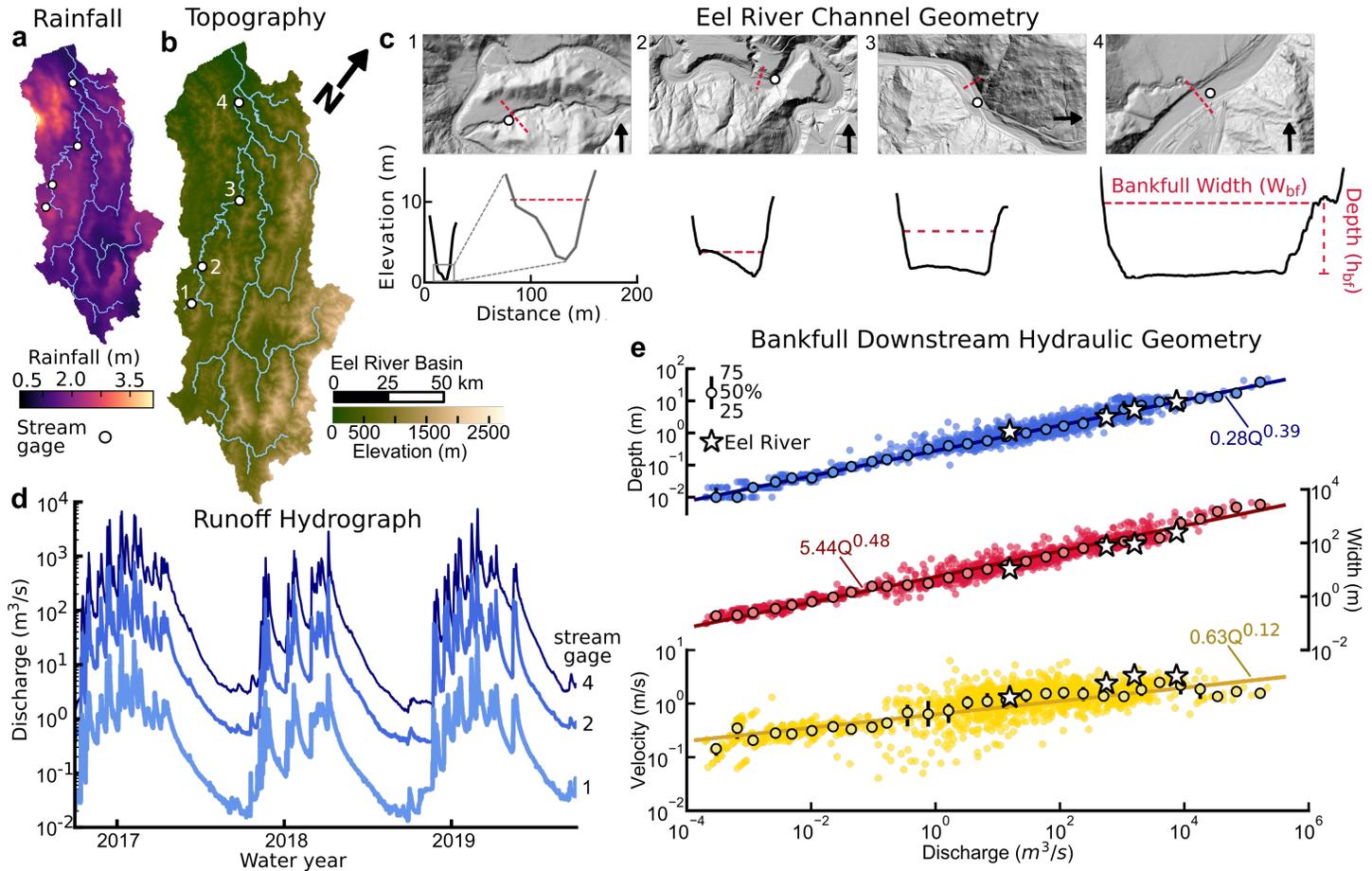


Figure 1. Downstream hydraulic geometry within a catchment. **a** | Annual precipitation (30 year normals²³¹) within the Eel River watershed. **b** | Topography of the Eel River watershed. **c** | (top row) High resolution lidar hillshade²³² showing USGS gage and cross section locations. The scale is the same for each image. (bottom row) Cross sections near the gaging sites. The size of the channel grows considerably as drainage area and discharge increase downstream. The bankfull width (red dashed line) is labelled on each cross section. **d** | Runoff hydrographs for three gages (labeled 1, 2, and 4). Notice that in this watershed the shape of the hydrographs changes minimally between upstream and downstream, but gains considerable flow. **e** | Bankfull downstream hydraulic geometry (width, depth, and velocity) for 1,652 rivers (small points)¹¹⁰. The best fit power relations are fit to the binned median values (large circles). Error is shown through the interquartile range and is typically smaller than the plotting symbol. The stars represent the four gages from the Eel River. (top row) Bankfull depth scaling against discharge. (middle row) Bankfull width scaling against discharge. (bottom row) Bankfull velocity scaling against discharge. Here the velocity $U_{bf} = Q_{bf}/(W_{bf}H_{bf})$. Velocity shows considerably more scatter than either width or depth data, but is also not independently measured. The fitted trend lines are remarkable in that they provide first order approximations for river hydraulic geometry across 10 orders of magnitude in discharge.

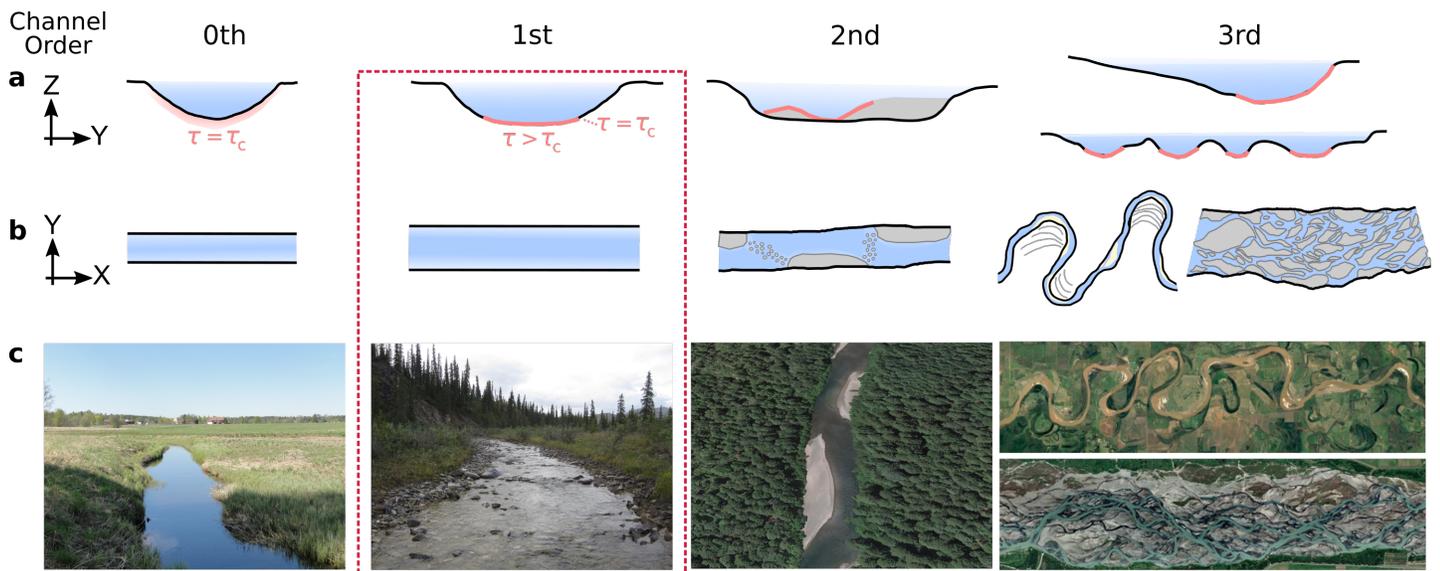


Figure 2. Schematic illustration of the proposed orders of channel behavior. **a** | Schematic channel cross sections representing the orders of channel behavior from left to right: a threshold channel, near-threshold channel, channel with morphology, and meandering or braided morphology. The red dashed box represents the 1st order near-threshold channel that is the focus of this review. Light red regions represent regions of the channel at or above the threshold of motion. **b** | Planform or map view of the channel cross sections. **c** | Photographs and satellite images of (from left to right) a grass lined canal in Sweden, a cobble river in Alaska, the Eel River with alternating bars in California, and meandering and braided rivers in Indiana (USA) and New Zealand, respectively. The photo of the canal is courtesy of B. Neilson, and the alternate bar, meandering and braided rivers are from Google Earth.

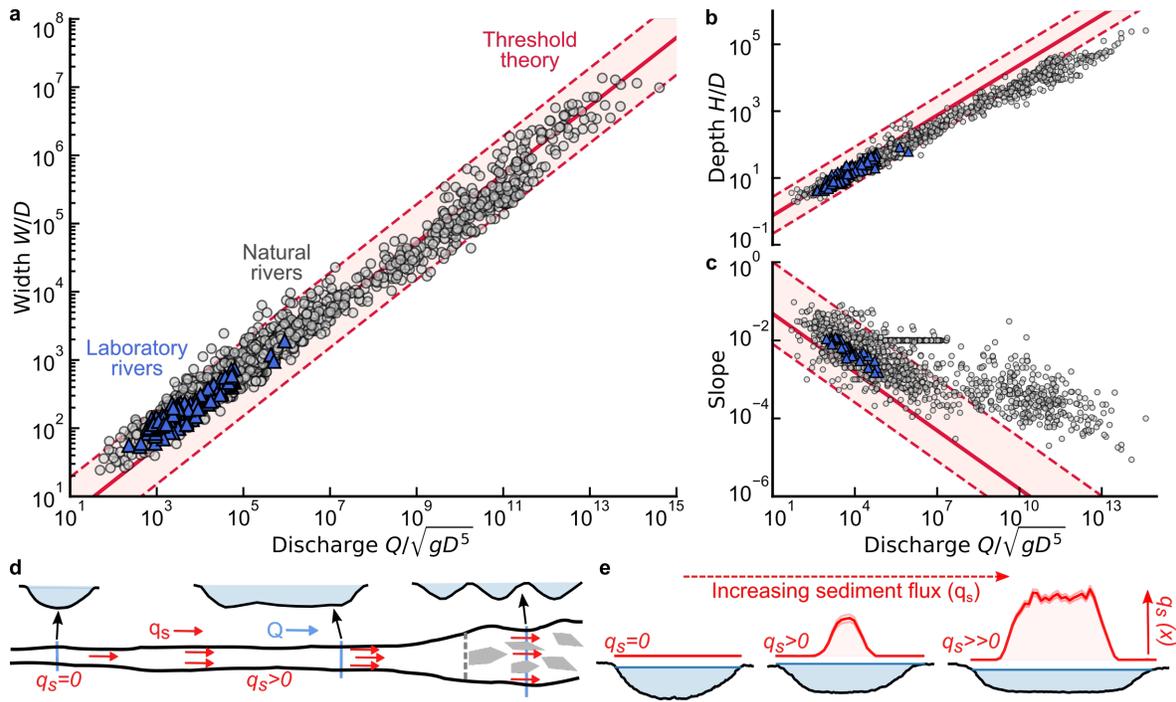


Figure 3. The width of natural and laboratory alluvial rivers follow near threshold predictions. **a** | Natural (1,581 gray points¹¹⁰) and laboratory alluvial rivers⁵³ dimensionless width (W/D_{50}) and discharge scaling compared with threshold theory (red line represents a threshold channel with $\tau_c^* = 0.05$ and $C_f = 0.1$). Both data sets sit slightly offset from the threshold theory. The shaded area denotes uncertainty within the possible parameter estimates for a threshold channel. **b** | Dimensionless depth (H/D_{50}) against dimensionless discharge. Fine-grained rivers are significantly shallower than threshold theory predicts. **c** | River slope against dimensionless discharge. The threshold channel is less steep than coarse-grained rivers and significantly lower gradient than fine-grained rivers. **d** | Schematic of the evolution of a transient experimental channel illustrating the transition from threshold, to increasing sediment flux to the point of channel instability. The early experiments of Stebbings¹¹⁵ illustrate the end member conditions of single thread alluvial channels and the importance of sediment flux. **e** | Recent experimental efforts under laminar flow conditions directly measure the influence of increasing sediment flux on channel geometry⁵¹. From left to right, under no sediment flux ($q_s = 0$) and constant discharge the channel cross section nearly exactly matches the cosine prediction from threshold theory, increasing sediment flux (middle and right) drives a stark increase in channel aspect ratio (W/H) and a steepening of the channel banks.

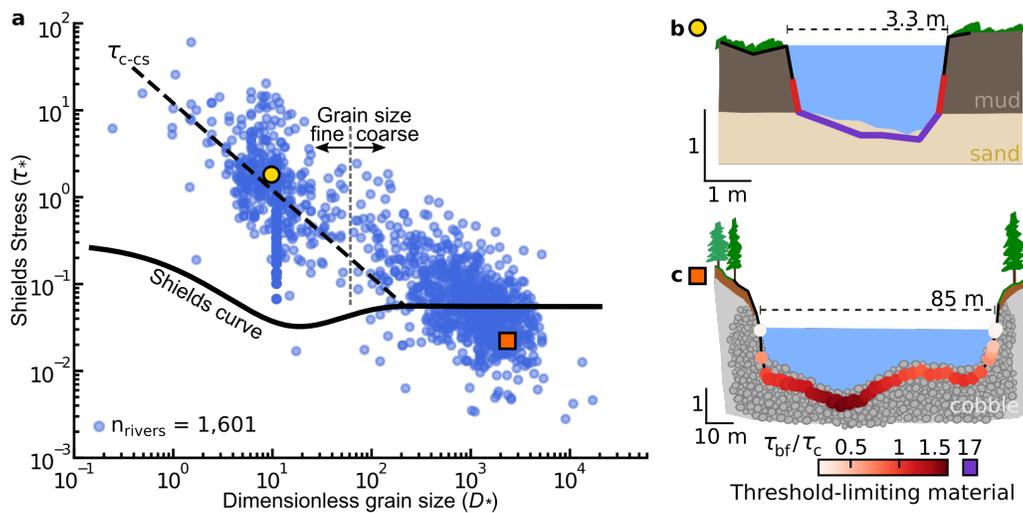


Figure 4. Shields diagram and illustrations of the near-threshold and threshold limiting models. **a** | Variation of bankfull dimensionless shear stress (Shields stress) with dimensionless grain size. The compiled rivers create two clouds of data between coarse and fine-grained rivers (dotted vertical line is 2.5 mm). The coarse grained rivers cluster near the threshold of motion as defined by the Shields curve, while fine-grained rivers cluster about the average threshold for clay sand mixtures (τ_{c-CS} , dashed diagonal line). **b** | Illustration of the threshold-limited model for the Mullica River (yellow circle), a fine-grained river with cohesive banks. The black line represents the surveyed cross section. The cohesive banks (mud) are near the threshold at the channel center ($\tau_{bf}/\tau_c = 1.13$), while the fine-grained bed material (sand) is well above its respective threshold of motion ($\tau_{bf}/\tau_c = 17$). **c** | Illustration of the near-threshold model for a surveyed cross section (black line) the Salmon River, a cobble lined river in rural Idaho. Surveyed points below the bankfull flow are shaded according to the bankfull transport capacity (average $\tau_{bf}/\tau_c = 1.17$). Shear stresses are computed via the depth slope product using the hydraulic radius due to the narrow channel aspect ratio for the Mullica River and using the local depth for the Salmon River.

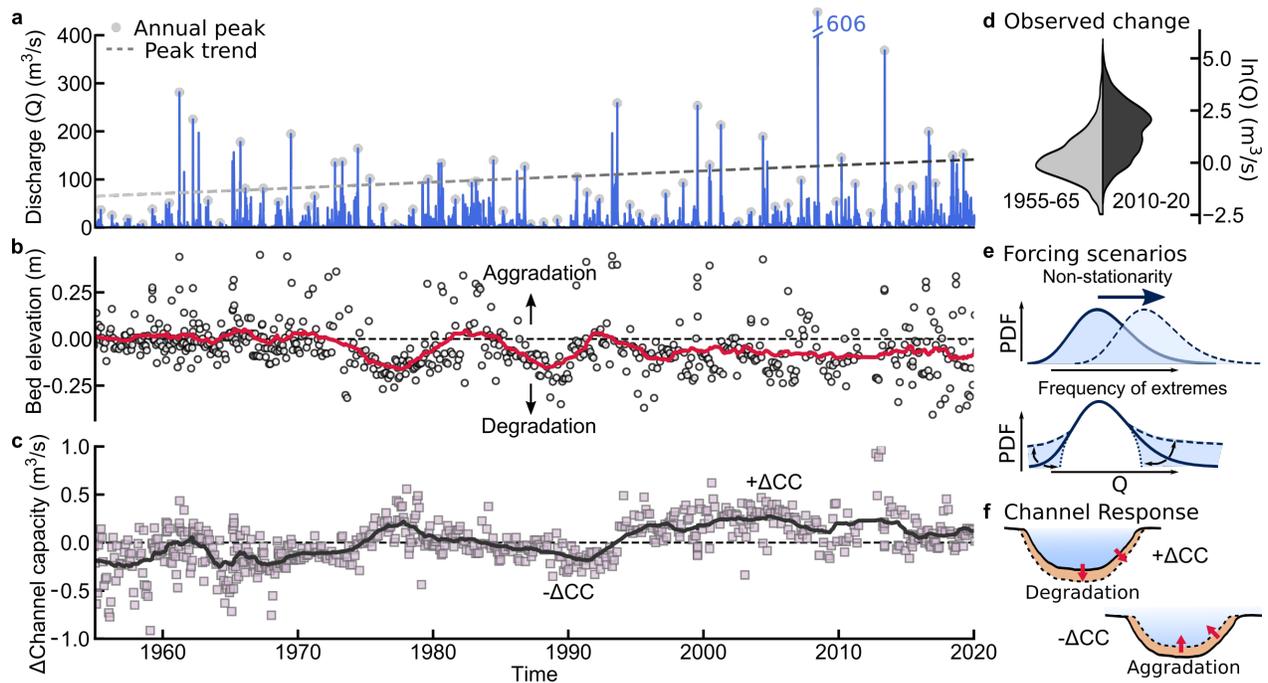


Figure 5. River channel size grows and shrinks in response to hydroclimatic cycles and persistent changes in hydroclimate. **a** Daily mean discharge (Q) records from 1955-2020 for Little Cedar River near Ionia, Iowa, USA (Gage No. 05458000). At this gage the annual peak flow (gray circles) has increased over time (dashed trend line). Periodicity within the discharge record is correlated with the Arctic Oscillation¹⁰⁹. **b** Mean bed elevation measurements over time showing a gradual degradation of the bed. The rolling average (red line) highlights periods of persistent scour or fill relative to the start of the record (dashed black line). **c** Changes in flood stage channel capacity (ΔCC , m^3/s) over time. Increases in discharge were accommodated by channel bed degradation resulting in increased channel capacity since the 1950s. The rolling average (black line) represents periods of increased/decreased channel capacity relative to the average stage-discharge rating curve. **d** Observed change in discharge frequency between the initial (1955-65, light gray) and final (2010-20, dark gray) ten years of the record. The probability density functions (PDF) are represented by the kernel density estimates of the natural log-transformed discharge data and show an overall shift in the discharge distribution. **e** Potential statistical changes within the discharge record (non-stationarity) as a result of changes in landuse or hydroclimate include a shift in the mean and/or the frequency of extreme values. These changes result in differing forcing scenarios for channel adjustment. **f** Schematic showing increases in channel capacity (conveyance) through degradation, increased widening and/or declining roughness (i.e., τ_{bf}), and decreases through aggradation, narrowing and/or increasing roughness.

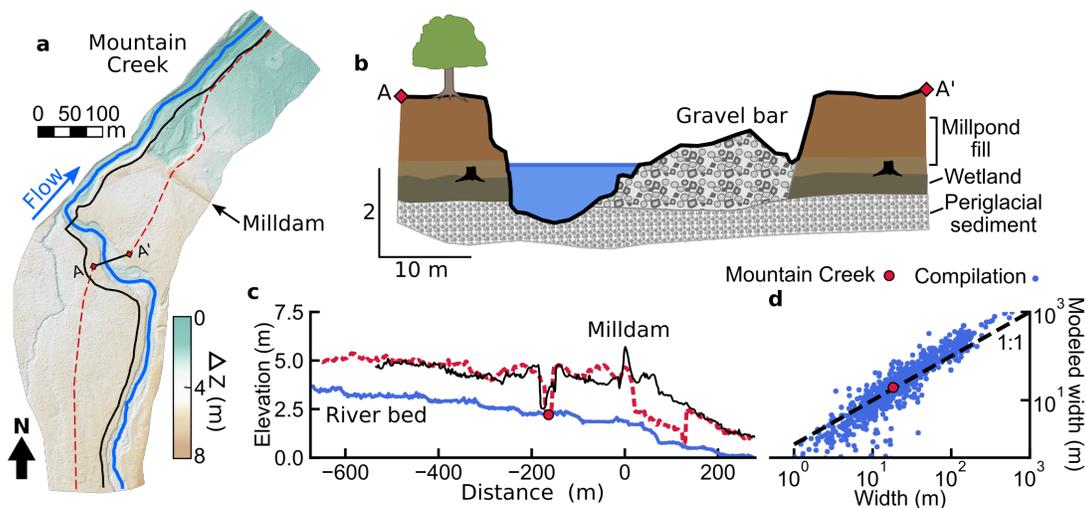


Figure 6. Historic land use can alter river geometry over long timescales. **a** | Lidar topography of Mountain Creek near Mt. Holly Springs, PA²³³. The presence of a historic milldam resulted in reduced flow velocities and significant upstream sediment deposition. The resulting deposition can be seen in the elevation difference (ΔZ) above and below the breached dam. The traced lines represent the longitudinal profiles shown in **c**. **b** | Cross section of Mountain Creek showing buried precolonial wetland sediment characterized by relic tree stumps and wetland vegetation. The increased mobile sediment following the breach of the dam resulted in rapid incision down to the coarser periglacial sediment below. Modern inset gravel bars are a result of current sediment mobility. **c** | Longitudinal profiles of the modern river bed (blue) and two profiles (black solid and red dashed lines) showing the elevation up and downstream of the milldam. **d** | Modeled bankfull width for Mountain Creek (red circle) and coarse-grained rivers ($D_{50} > 5$ mm, blue points) from the data compilation¹¹⁰. The $1 + \varepsilon$ model provides an accurate prediction of the modern channel width based on the periglacial sediment diameter ($D_{50} = 68$ mm) indicating that the current channel is well described by the near-threshold model. Modeled predictions follow from Box 2, with a coarse-grained river average $C_f=7$ and $\tau_c = \tau_{bf}/1.2$. The misalignment at low and larger widths for the compilation is a consequence of the use of a single value for C_f .

1011 Glossary terms

- 1012 • **sediment**: particulate material that has been moved and deposited by some natural process
- 1013 • **water discharge**: the flow rate of water measured in terms of volume per unit time
- 1014 • **entrainment**: dislodging of grains from the surface of a sediment bed by a current
- 1015 • **fluvial geomorphology**: study of the form and process of rivers
- 1016 • **bankfull**: the flow which brings a river to the brink of overbank flooding
- 1017 • **universality**: robust statistical or scaling properties that are independent of system details
- 1018 • **dimensionless**: groupings of variables that have no units, often used to reduce the number of free parameters
- 1019 • **morphodynamic**: the feedback between (fluid) flow and (bed-surface) form that evolves landscape patterns
- 1020 • **braided rivers**: channels with myriad bars that divide flow into numerous smaller (and typically unstable) threads
- 1021 • **stationary**: behavior or process that does not depend on time
- 1022 • **singular perturbation analysis**: mathematical approach for problems containing a small parameter that cannot be
1023 approximated by setting the parameter value to zero
- 1024 • **bed load**: fluid-driven motion of particles that are in close contact with, and are supported by, the bed
- 1025 • **mud**: wet particulate mixture with significant cohesion, typically from clay and/or organics
- 1026 • **form drag**: the portion of fluid momentum that is dissipated by (turbulent) interaction of the flow with the bed
- 1027 • **closure scheme**: a parameterized relation among two (or more) variables that closes a set of equations, allowing a unique
1028 solution
- 1029 • **parameterization**: representing a complex process with a simpler mathematical expression, or constant parameter, for
1030 the purposes of modeling