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# Combining shallow-water and analytical wake models for tidal array micro-siting

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#### Abstract

For tidal-stream energy to become a competitive renewable energy source, clustering multiple turbines into arrays is paramount. As a result, array optimisation is critical for achieving maximum power performance and reducing cost of energy. However, ascertaining an optimal array layout is a highly complex problem, subject to specific site hydrodynamics and multiple inter-disciplinary constraints. In this work, we present a novel optimisation approach that combines an analytical-based wake model, FLORIS, with an ocean model, Thetis. The approach is demonstrated with applications of increasing complexity. By utilising the method of analytical wake superposition, the addition or alteration of turbine position does not require re-calculation of the entire flow field, thus allowing the use of simple heuristic techniques to perform optimisation at a fraction of the computational cost of more sophisticated methods. Using a custom condition-based placement algorithm, this methodology is applied to the Pentland Firth for arrays with turbines of  $3.05 \,\mathrm{m\,s^{-1}}$  rated speed, demonstrating practical implications

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whilst considering the temporal variability of the tide. For a 24 turbine array case, micro-siting using this technique delivered an array 15.8% more productive on average than a staggered layout, despite flow speeds regularly exceeding the rated value. Performance was evaluated through assessment of the optimised layout within the ocean model that represents the turbines through a discrete turbine representation. Used iteratively, this methodology could be applied to deliver improved array configurations in a manner that accounts for local hydrodynamic effects.

**Keywords:** Array optimisation, Tidal turbines, *FLORIS*, Shallow water equations

## 1 Introduction

The levelised cost of energy (LCOE), defined as the average net present cost of electricity generation for a power plant over its lifetime, is often cited as a key metric for the competitiveness of an energy technology. Unless there is a rapid increase in installations, the LCOE for tidal-stream is set to remain at more than £150/MWh by 2025 (Smart and Noonan, 2018; Topper et al., 2021), whilst the LCOE for solar and both onshore and offshore wind will fall to approximately £25–£32/MWh (U.S. Energy Information Administration, 2020). Reducing LCOE is paramount if tidal-stream energy is to become a competitive, sustainable energy source (Coles et al., 2021). This could be achieved through several measures (Coles and Walsh, 2019; Goss et al., 2020, 2021a,b): (i) physical infrastructure improvements, which could involve optimisation of the turbine design and operation, (ii) economies of scale in turbine design, (iii) economies of volume in manufacturing, operation and maintenance, (iv) technology innovation, (v) learning, and (vi) financing mechanisms. Turbines have now reached technology readiness levels of 7–8 (Chozas, 2015; SIMEC Atlantis Energy, 2020a) and need to be tested in large arrays for extended periods of time in order to reach full maturity and facilitate implementation of the aforementioned cost reduction mechanisms. In supporting this, strategies should be investigated and developed for the reliable assessment of the tidal resource (Neill et al., 2014; Robins et al., 2015; Neill et al., 2018; Mackie et al., 2021b) to reduce investment uncertainty, as well as array design optimisation to maximise performance. Array optimisation has already shown potential to increase array power by up to 33% relative to a regular aligned layout, albeit with power capping removed (Funke et al., 2014). Hence developing more robust, yet practical optimisation methods could be a key step to achieving further LCOE reductions (Coles et al., 2021).

Array power can be associated with up to eight controlling array effects, as outlined in Vennell et al. (2015). These include the reduction of free-stream velocity by the introduction of turbines and the relative size of the array in the channel. This leads to conflicting design performance interactions among

turbines, particularly for large arrays that dominate channel dynamics. For example, minimising environmental impacts such as sediment transport may restrict array placement (Fairley et al., 2015; du Feu et al., 2019). Likewise, maintaining navigation routes through clearance constraints prevents exploitation of channel blockage, a beneficial phenomenon for larger arrays. As such, array optimisation is often posed as a multi-objective problem, adding additional constraints to an already complex problem (Nash et al., 2014; Culley et al., 2016; du Feu et al., 2017, 2019; González-Gorbeña et al., 2018; Phoenix and Nash, 2019).

Establishing the optimal array layout becomes computationally intensive when interlinked with the hydrodynamics at the installation area as it presents a partial differential equation (PDE) constrained optimisation problem. Early work therefore involved simplified hydrodynamic models, as 'in-concert' tuning of tidal turbines in an array would necessitate multiple runs which would require appreciable time in more detailed models (Vennell, 2011, 2012). Investigations of channel-scale optimisation, such as a systematic exploration of optimal array layouts by conducting large numbers of 2-D simulations for different layouts and turbine tunings have been carried out, but are notably time and memory intensive (Divett et al., 2016). An alternative has been proposed by using gradient-based optimisation that makes use of adjoint methods to efficiently calculate the objective function gradient, leading to immense reductions in the number of evaluations required (Funke et al., 2014, 2016). This enables optimisation with a capacity to account for impacts to the hydrodynamics, at a much lower computational cost than techniques that estimate the gradient. The same approach has been adopted for wind farms to capture nonlinear turbulent flow physics, as the adjoint method allows inclusion of higher fidelity 3-D computational fluid dynamics (CFD) (King et al., 2017). Nevertheless, adjoint optimisation remains fairly intensive as demonstrated by examples in the literature, which are largely constrained to idealised and semi-idealised cases (Funke et al., 2014; Barnett et al., 2014). Similarly, the integration of 3-D modelling with optimisation algorithms beyond idealised cases (as in King et al. (2017)) is scarce. Recent work on discrete turbine array optimisation has relied on 2-D coastal hydrodynamics models (Piggott et al., 2021), employing simplified turbine parameterisations whilst being constrained by either the attainable model structure or resolution, as in Phoenix and Nash (2019).

To circumvent intense computational effort, inspiration can be taken from wind energy research, where surrogate models are used to simplify the governing physics. These models may ignore important hydrodynamic effects such as blockage that can augment power production for tidal energy (Nishino and Willden, 2012; Chen et al., 2019). For example, a "duct effect" may be exploited by placing turbines in a staggered arrangement, funnelling and accelerating the flow onto downstream turbines, as shown in Funke et al. (2014). Aside from certain examples restricted in idealised domains (Stansby and Stallard, 2016), semi-analytical methods based on turbine wake superposition principles are often constrained to a structured turbine placement (Lo Brutto et al., 2016). Nevertheless, the use of wake superposition methods has been found to give reasonable agreement to laboratory measurements for model tidal turbines. Rapid optimisation in idealised low-blockage cases has provided significant increases in array efficiency (Stansby and Stallard, 2016). González-Gorbeña et al. (2016) took a similar approach, applying a surrogate-based optimisation strategy in combination with a hydrodynamic model; there, turbine placement was limited to spacing parameters applied uniformly across structured arrays to avoid high computational times associated with a greater number of parameters within the optimisation framework.

In setting out this study, we outline our overarching goal: an array optimisation strategy that is computationally efficient and extensible to the multi-objective optimisation settings sought thereafter. Additionally, it must be reliable, accurate and acknowledging important hydrodynamic factors and turbine characteristics that affect the optimal array design and performance. This paper aims to demonstrate a novel optimisation approach, retrofitting an analytical wake model designed for wind array optimisation (*FLORIS* from the US National Renewable Energy Laboratory) for use in conjunction with a coastal ocean model (*Thetis*). We provide details on an optimisation approach which includes the option of a custom greedy algorithm for micro-siting purposes. This is applied to a suite of representative idealised cases, progressing to a practical study of the Inner Sound of the Pentland Firth, UK.

## 2 Methodology

We combine a depth-averaged hydrodynamic model,  $Thetis^1$ , with an analytical wake model, FLORIS (FLOw Redirection and Induction in Steady-state<sup>2</sup>). FLORIS is used to perform array optimisation by importing ambient flow fields from *Thetis*, returning an optimised set of turbine coordinates. Sequentially, *Thetis* evaluates initial and optimised layouts, by representing the presence of turbines parameterised through momentum sink terms, quantifying impacts on flow field and overall array power. Both models rely on actuator disc theory to represent the tidal turbine rotor, however, differences between the two models exist, which necessitates the introduction of an intermediate calibration step. A schematic of the combined approach is shown in Figure 1.

### 2.1 Shallow Water Equation Modelling with Thetis

Thetis is a 2-D/3-D model for coastal and estuarine flows based on the generalpurpose finite element partial differential equation (PDE) solver *Firedrake* (Rathgeber et al., 2016; Kärnä et al., 2018). It has been used for several studies on the feasibility and optimisation of tidal energy (Angeloudis et al., 2018; Baker et al., 2020; Goss et al., 2020; Harcourt et al., 2019). In this particular study we solve the non-conservative form of the nonlinear shallow-water equations in two-dimensions,

<sup>&</sup>lt;sup>1</sup>http://thetisproject.org/

<sup>&</sup>lt;sup>2</sup>https://floris.readthedocs.io/en/main/

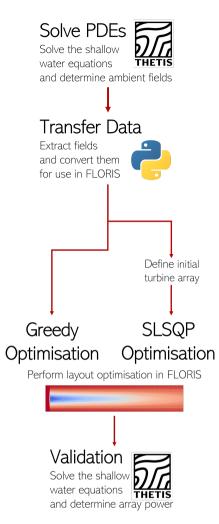


Fig. 1 Schematic representation of the model combination forming the optimisation sequence.

$$\frac{\partial \eta}{\partial t} + \nabla \cdot (H_d \mathbf{u}) = 0, \tag{1}$$

$$\frac{\partial \mathbf{u}}{\partial t} + \mathbf{u} \cdot \nabla \mathbf{u} + g \nabla \eta = \nabla \cdot \left(\nu (\nabla \mathbf{u} + \nabla \mathbf{u}^T)\right) - \frac{\tau_b}{\rho H_d} - \frac{c_t}{\rho H_d} |\mathbf{u}| \,\mathbf{u} + f \mathbf{u}^\perp, \quad (2)$$

where  $\eta$  is the water elevation,  $H_d$  is the total water depth, **u** is the depthaveraged velocity vector, and  $\nu$  is the kinematic viscosity of the fluid. The term  $f\mathbf{u}^{\perp}$  represents the Coriolis "force" included in non-idealised cases. In this term,  $\mathbf{u}^{\perp}$  is the velocity vector rotated counter-clockwise over 90° so that  $\mathbf{u}^{\perp} = (-v, u)$ , where u, v are respectively the longitudinal and transverse components of  $\mathbf{u}$ . In turn,  $f = 2\Omega \sin(\zeta)$  with  $\Omega$  the angular frequency of the Earth's rotation and  $\zeta$  the latitude. In idealised cases, bed shear-stress  $(\boldsymbol{\tau}_b)$  effects are represented through a quadratic drag formulation,

$$\frac{\tau_b}{\rho} = C_D |\mathbf{u}| \,\mathbf{u} \tag{3}$$

For practical simulations undertaken in this work the Manning's  $n_M$  formulation is adopted, given as

$$\frac{\boldsymbol{\tau_b}}{\rho} = g n_M^2 \frac{|\mathbf{u}| \, \mathbf{u}}{H_d^{\frac{1}{3}}},\tag{4}$$

and applied as per Mackie et al. (2021a). When applicable, inter-tidal processes are treated using the wetting and drying formulation of Karna et al. (2011). The shallow-water equations are discretised using the discontinuous Galerkin finite element method (DG-FEM) and the semi-implicit Crank-Nicolson scheme is selected for time-marching the solution. The resulting discrete system of equations is solved iteratively by Newton's method as implemented in PETSc (Balay et al., 2016). Finally,  $c_t$  is an additional parameterisation used to represent the turbines' thrust as described in the following section.

#### 2.2 Discrete turbine representation in *Thetis*

Turbine rotors are represented in *Thetis* as areas of increased bed friction, adopting the linear momentum actuator disc theory (Kramer and Piggott, 2016). In the 2-D depth-averaged form of the shallow-water equations, the force as a result of an array of turbines is:

$$F_{\text{array}} = \int_{\Omega_{\text{array}}} \frac{1}{2} \rho \, c_t(\mathbf{x}) \left| \mathbf{u}(\mathbf{x}) \right| \, \mathbf{u}(\mathbf{x}) \, \mathrm{d}\mathbf{x},\tag{5}$$

where  $c_t(\mathbf{x})$  is a thrust coefficient function given as:

$$c_t(\mathbf{x}) = C_t(\mathbf{u}(\mathbf{x}))A_t d(\mathbf{x}),\tag{6}$$

where  $A_t$  is the turbine swept area,  $C_t$  is the thrust coefficient as a function of the velocity  $\mathbf{u}(\mathbf{x})$ , and  $d(\mathbf{x})$  is the local turbine density. The turbine density  $d(\mathbf{x})$  is constructed using a vector  $\mathbf{m}$  comprising the turbine coordinates of the array. This discrete turbine representation employs the exponential bump function as in Funke et al. (2014), which in 1-D takes the form

$$\psi_{p,r}(x) \equiv \begin{cases} e^{1-1/\left(1-\left|\frac{x-p}{r}\right|^2\right)} & \text{for } \left|\frac{x-p}{r}\right| < 1\\ 0 & \text{otherwise} \end{cases},$$
(7)

where r is the radius of the bump, set by default to D/2, where D is the diameter of the turbine. Equation (7) is employed in defining the turbine density

 $d_i$  for a turbine *i* at a position  $m_i = (x_i, y_i)$  as the normalised product of 1-D bump functions:

$$d_i(\mathbf{x}) = \frac{\psi_{x_i,r}(x)\psi_{y_i,r}(y)}{\Xi r^2},\tag{8}$$

where  $\Xi = \int_{-1}^{1} \int_{-1}^{1} e^{\left(\frac{-1}{1-x^2} - \frac{1}{1-y^2} + 2\right)} dx dy \approx 1.45661$  is the integral of the bump function when r = 1. The aggregate of the individual turbine densities  $d_i$  provides the overall  $d(\mathbf{x})$  function:

$$d(\mathbf{x}) = \sum_{i=1}^{N} d_i(\mathbf{x}),\tag{9}$$

where N is the number of turbines deployed.

Following the notation in (5), the power extracted at any given moment by the array can be approximated as

$$P_{\text{array}} = \int_{\Omega_{\text{array}}} \frac{1}{2} \rho c_p(\mathbf{x}) \left| \mathbf{u}(\mathbf{x}) \right|^3 \, \mathrm{d}\mathbf{x},\tag{10}$$

where  $c_p(\mathbf{x})$  is a power coefficient function given as

$$c_p(\mathbf{x}) = C_p(\mathbf{u}(\mathbf{x}))A_t d(\mathbf{x}), \tag{11}$$

where  $C_p$  is a power coefficient which is related to the thrust coefficient through the formulation (Martin-Short et al., 2015b)

$$C_p(\mathbf{u}(\mathbf{x})) = \frac{1}{2} \left( 1 + \sqrt{1 - C_t(|\mathbf{u}(\mathbf{x})|)} \right) C_t(|\mathbf{u}(\mathbf{x}|).$$
(12)

In equations (5) and (10) it is assumed that the ambient velocity is the same as the velocity through the turbine,  $\mathbf{u}(\mathbf{x})$  (i.e. the velocity once the turbine is operating). This is a reasonable approximation for relatively coarse meshes with distributed rather than discrete turbine density fields (Schwedes et al., 2017). However, for micro-siting arrays where the thrust force is concentrated at the turbine location, this assumption becomes invalid. In addressing this we adopt the correction for deriving a relationship between free-stream and through-turbine velocities as derived in more detail by Kramer and Piggott (2016). In summary, denoting as  $U_{\infty}$  the magnitude of the approaching streamwise velocity the turbine experiences, it can be established using the continuity, momentum and Bernoulli's principles that

$$U_{\infty}(\mathbf{x}) = \frac{1}{1 + \frac{1}{4} \frac{A_t}{\hat{A}_t} C_t \left( |\mathbf{u}(\mathbf{x})| \right)} |\mathbf{u}(\mathbf{x})|$$
(13)

where  $\hat{A}_t = H_d \Delta y$  is the numerical cross-section of the turbine. The corrected velocity  $U_{\infty}$  is sequentially applied to correct the thrust  $(c_t)$  and power  $(c_p)$  coefficient values, compensating for the velocity drop by the introduction of the turbine momentum sink over the deployed area of the turbine.

#### 2.3 Analytical wake modelling using FLORIS

*FLORIS* contains analytical models to predict the mean wake velocities and power output of turbine arrays (NREL, 2020). In the present study, we apply *FLORIS*'s Gaussian model (Bastankhah and Porté-Agel, 2014) which computes the normalised velocity deficit via the expression

$$\frac{\Delta U}{U_{\infty}} = \left(1 - \sqrt{1 - \frac{C_T}{8\left(k^* x/D + \epsilon\right)^2}}\right) \cdot e^{\left(-\frac{1}{2\left(k^* x/D + \epsilon\right)^2} \left\{\left(\frac{z-z_h}{D}\right)^2 + \left(\frac{y}{D}\right)^2\right\}\right)},\tag{14}$$

where z is the wall-normal coordinate with  $z_h$  the turbine hub height,  $k^*$  is the growth rate of the wake  $(\partial \sigma / \partial x)$ , and  $\epsilon$  is the normalised Gaussian velocity deficit at the rotor plane. For our calculations the local wake growth rate  $k^*$ is estimated using the local streamwise turbulence intensity,  $\mathcal{I}$  (Niayifar and Porté-Agel, 2016). We should note here that the Gaussian velocity model has been selected instead of the more traditionally used Jensen model (Jensen, 1983) which is of similar computational cost but is known to overestimate the velocity deficit in the outskirts of the wake (Dufresne and Wosnik, 2013; Chamorro and Porté-Agel, 2009). This is due to the Jensen model's approach of setting a uniform velocity deficit across the wake width. In turn, turbine power output is calculated using a power thrust-velocity relationship specified for each individual turbine. This requires a combination model to account for the contributing wake velocity deficit from upstream and other neighbouring turbines. We use the free-stream linear superposition (FLS) method to account for the cumulative wake effects within the tidal array. Accordingly, the velocity deficit,  $\Delta \mathbf{u}(x, y)$ , at a downstream location (x, y) is calculated as,

$$\overline{\Delta \mathbf{u}}(x,y) = \sum_{i=1}^{N} \left( \overline{\Delta \mathbf{u}}_{i} |_{(x,y)} \right), \tag{15}$$

where  $\overline{\Delta \mathbf{u}}_i|_{(x,y)}$  is the contribution the wake of each turbine *i* at the downstream location (x, y) (Machefaux et al., 2015). Alternative superposition methods include summing the square of the velocity deficits (Katic et al., 1987) as well as the more recent work by Lanzilao and Meyers (2021) which takes into account the heterogeneity of the background velocity field.

#### 2.4 Optimisation approach

We seek to maximise energy from our tidal array system. In doing so, the existing layout optimisation procedure in *FLORIS* (Fleming et al., 2016) is adapted to maximise the average power computed using several input flow fields, rather than the average annual energy production from a single wind rose. The latter is typical of wind farm optimisation and would not apply to tidal-array optimisation. To this end, we approach the tidal-array micro-siting problem by

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employing an initial *Thetis* simulation of the tidal channel and extract ambient velocity fields for a number of instances, or 'frames', over a tidal cycle. This ambient flow field data is then imported into *FLORIS*. If necessary, an initial (e.g. aligned/staggered) turbine layout is introduced to *FLORIS* and micro-siting is performed using an appropriate optimisation strategy subject to spatial constraints, and minimum turbine separation restrictions. As such, the objective function can be expressed as

$$\max_{\mathbf{m}} \qquad \frac{1}{N_F} \sum_{j=1}^{N_F} P_{\text{array}}(\mathbf{u}, \mathbf{m})$$
(16)

subject to 
$$au_l \leq au_i \leq au_u$$

where  $N_F$  is the number of flow field frames considered and  $\mathbf{m}, \tau_i$  are vectors including turbine coordinates and optimisation constraints (e.g. minimum distance between turbines, array deployment area limits) respectively.  $\tau_l, \tau_u$  correspond to the lower and upper limits for each of these constraints.

Upon optimisation in *FLORIS*, a derived turbine layout **m** is evaluated in *Thetis* to assess its performance. A similar approach to optimisation has previously been undertaken to determine wind plant control strategy, with the objective of optimising yaw settings to minimise wake interaction and increase overall farm power (Gebraad et al., 2014). In a deviation from the study of Gebraad et al. (2014) which pioneered the blending of a CFD flow solver with *FLORIS*, we present herein a first attempt to combine *FLORIS* with a shallow-water solver for tidal applications.

As we aim to demonstrate a proof-of-concept for the optimisation *approach*, investigations on specific optimisation algorithms are beyond the scope of this work. FLORIS's default optimisation is initially performed using the SciPy minimise function for the idealised models (see Section 4), through the SLSQP (Sequential Least SQuares Programming) method (Kraft, 1988). The number of iterations for each SLSQP minimisation problem was limited to the default value of 50. Altering 2N variables (i.e. x-, y- coordinates for N turbines) for each flow field over 50 iterations becomes highly time-consuming as the number of turbines N increases beyond a small array. An increased array size also entails a larger optimisation space, further stressing conventional optimisation, increasing the likelihood of converging to local maxima. To address the above, a heuristic-based greedy optimisation technique is tested which positions each turbine sequentially. This allows the imposition of constraints which form acceptance criteria, sequentially adding turbines until either desired capacity is installed or no feasible positions remain. This proposed alternative approach allows for the rejection of proposed turbine placements based on aspects such as bathymetric gradient, forming a basis for non-trivial objective functions. The simplified sequence is described in Algorithm 1.

**Algorithm 1** Sequential addition of turbines to domain using greedy optimisation.

INPUTS: Number of turbines to be positioned N, maximum number of optimisation iterations and ambient flow fields.

CONDITIONS  $(A, B, \Gamma)$ : Each turbine must meet a minimum A-% average turbine capacity factor, have maximum reduction of power to any other turbine of B-% and maximum reduction of power to the sum of individual turbines (that face power output reductions) of  $\Gamma$ -%.

CONSTRAINTS  $(\Delta, E)$ : Minimum distance constraints for turbine placement, specified in turbine diameters away from considered coordinate.

- 1: Select ambient flow fields from hydrodynamic simulations performed in *Thetis*.
- 2: while (iteration no. < maximum no. of iterations) and (no. of turbines < maximum no. of turbines)  ${\bf do}$
- 3: Calculate and superimpose turbine wakes to each flow field of selected tidal cycles.
- 4: Calculate a field of moving average flow magnitude (a moving average deters turbine placement on wake edges).
- 5: Identify coordinate of maximum average velocity magnitude as a candidate turbine location.
- 6: Add turbine at candidate site and superimpose wake on each flow field.
- 7: Calculate the average power (using each individual field) for all turbines including the new turbine.
- 8: **if** CONDITIONS are met **then**
- 9: Add candidate site to list of accepted coordinates.
- 10: Impose a restriction for turbine placement within a limiting distance  $\Delta$  around new coordinate.
- 11: **else**
- 12: Add candidate site to list of blocked coordinates.
- 13: Impose a restriction for turbine placement within a limiting distance of E around blocked coordinate.

```
14: end if
```

```
15: end while
```

# 3 Turbine specifications and analytical wake calibration

Before considering the optimisation case studies it is instructive to outline the turbine specifications and calibration process in configuring the analytical wake model parameters of *FLORIS* so that shallow-water wakes are adequately represented. For the tidal turbines, consistent specifications summarised in Table 1 are applied across all case studies. Turbine dimensions, cut-in and rated speeds ( $u_{rated}$ ) are based on known parameters for the SIMEC Atlantis

2 MW AR2000 turbine (SIMEC Atlantis Energy, 2016, 2020b). Combining equations (10), (11) and (12) allows the determination of the thrust coefficient at  $u_{\text{rated}}$  ( $C_{t,\text{rated}}$ ), considering the reported AR2000 turbine size (20 m) and its reported power output (2 MW). Given these specifications  $C_{t,\text{rated}} = 0.516$ , noting that this is lower than the value of  $C_t = 0.8$  determined in lab-scale experiments (Bahaj et al., 2007; Stallard et al., 2015). Figure 2 shows the theoretical tailing of the thrust coefficient for higher velocities. This has been approximated by Cardano's formula (Wituła and Słota, 2010) to produce a simpler equation preventing the need for third-order polynomial inversion that is otherwise required to calculate  $C_p$  throughout the hydrodynamic simulation. Below the cut-in speed,  $C_t$  is ramped up exponentially to avoid discontinuities which may cause instabilities within the hydrodynamic model without affecting the total power produced. For consistency,  $C_t$  and  $C_p$  are applied uniformly for both *FLORIS* and *Thetis*, with the resultant power curve illustrated in Figure 2.

 Table 1
 Common input parameters.

Fluid density, $\rho$	$1025 \text{ kg/m}^3$
Rotor swept diameter, $D$	$20 \mathrm{m}$
Hub height, $z_{\rm hub}$	$18 \mathrm{m}$
Turbine cut-in speed, $u_{in}$	1  m/s
Turbine rated speed, $u_{\rm rated}$	$3.05 \mathrm{~m/s}$

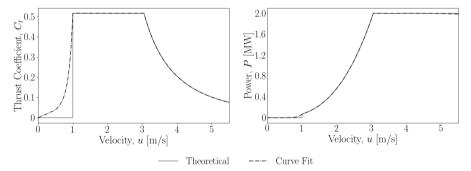


Fig. 2 Left: thrust coefficient function combining the tailing approximated by Cardano's formula relative to theoretical thrust coefficients for a 2 MW turbine. Right: corresponding power curves.

#### 3.1 FLORIS-specific inputs

As we apply *FLORIS* in the tidal-energy "domain", a number of *FLORIS*specific parameters are altered to appropriate values for a tidal setting (Table 2). The flow shear power law exponent and veer which describe the change in

Table 2FLORIS input parameters.

Flow shear power law exponent	0
Flow veer	0
Axial induction factor $(\alpha)$ exponent	0.8325
Normalised downstream distance $(x/d)$ exponent	-0.32
Initial turbulence intensity, $\mathcal{I}_0$	12%
Ambient turbulence intensity, $\mathcal{I}$	20%

vertical velocity and direction, respectively, are both set to 0, omitting vertical variability for consistency with *Thetis*. The turbulence model selected in FLORIS is documented by Crespo and Hernández (1996) and default coefficients in calculating the added streamwise turbulence intensity,  $I_{+}$ , are used. Accordingly, inputs for the axial induction factor exponent and the normalised downstream distance (x/d) exponent are set to the empirically determined values of 0.832 and -0.32 respectively. The initial turbulence intensity at the turbines,  $\mathcal{I}_0$ , has been determined experimentally at smaller scales to be 12% at the rotor plane for three-blade model tidal turbines (Stallard et al., 2015). Hub height streamwise turbulence intensity has been determined from ADCP deployments upstream of the Meygen Phase 1A turbines to be approximately 10% and 12% for peak flood and ebb flows respectively (Coles et al., 2018). Measured data in the Inner Sound of the Pentland Firth suggests the ambient turbulence intensities at peak flow speeds are 13% and 17% during flood and ebb tides, increasing linearly as the flow speed reduces (Hardwick et al., 2015). As the turbulence intensity is assumed uniform for simplicity, the initial ambient turbulence intensity is estimated to be 20%, as flow speeds (for optimisation purposes) will typically range from  $\approx 2-5.5$  m/s.

#### 3.2 Calibration of FLORIS wake effects

Wake-specific parameters are calibrated to replicate the depth-averaged velocity deficits exhibited by *Thetis* to render the evaluation of the 3-D *FLORIS*based optimal array designs in *Thetis* meaningful. Through the representation of turbines by momentum sinks (Section 2.2), *Thetis* acknowledges essential hydrodynamic interactions in the assessment of tidal stream arrays (e.g. turbine wake evolution, array blockage). Importantly, this is done within coastal ocean models that acknowledge complex morphologies as well as farfield forcings that drive the oscillatory flow over tidal array development areas. As *FLORIS* does not consider flow interaction processes via the wakesuperposition approach, its use to optimise arrays in *Thetis* can also be seen as a test for its potential application when linked with more computationally intensive models and real-world scenarios.

Parameters calibrated herein include  $k_a$  and  $k_b$ , which specifically relate to turbulence intensity and wake width. These combine and determine the value of the wake growth rate,  $k^*$ , which eventually enters the Gaussian velocity deficit equation (14) calculated as,

$$k^* = k_a \cdot \mathcal{I} + k_b. \tag{17}$$

The second set of parameters  $\alpha$  and  $\beta$  are used for the quantity,  $x_0$ , which defines the onset of the far wake,

$$x_0 = D \frac{1 + \sqrt{1 - C_t}}{\sqrt{2} \left( 4\alpha \cdot \mathcal{I} + 2\beta \left( 1 - \sqrt{1 - C_t} \right) \right)}.$$
(18)

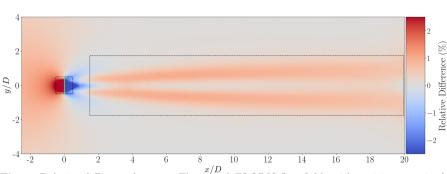
Calibration is performed using differential evolution (as implemented within SciPy's optimisation library (Virtanen et al., 2020)) to optimise the wake parameters  $k_a$ ,  $k_b$ ,  $\alpha$  and  $\beta$  such that the r.m.s. error between wakes in *Thetis* and *FLORIS* is minimised. It should be noted that the velocity deficit magnitudes in FLORIS are averaged over regular depth increments to produce an equivalent depth-averaged FLORIS wake, used to optimise model parameters. Calibration is performed for  $u_{\text{rated}}$  only, and then compared to results from calibration exercises for speeds below and above  $u_{\text{rated}}$  to gauge the extent of deviations. An idealised model consisting of a single turbine in the channel described in Section 4.1 is used to create a velocity deficit to be investigated over 20 diameters downstream for this purpose.

Wake calibration results are shown in Table 3, with the r.m.s. error between Thetis and FLORIS fields below 0.6% in the area of interest from 1.5-20Ddownstream. The difference in turbine representation is presented in Figure 3, clearly showing higher values of the FLORIS flow field velocity compared to *Thetis* as the velocity reduces over the bump function that represents the presence of the device. Immediately downstream of the FLORIS turbine, the velocity is lower than in *Thetis* due to the greater deficit imposed by *FLORIS*, which comes as a result of the discontinuous superposition of the analytical wake model at the turbine location. This discrepancy in turbine representation leads to the decision to calibrate based on the flow field from 1.5D downstream in a zone of width 3D to also capture the expansion width of the far wake. It should be noted that this is typically the region of highest error between not only differing turbine representation methods, but also to measured data; existing research has already demonstrated that accurately capturing the wake dynamics may require investigation of several different approaches to turbine modelling (Sandoval et al., 2021). The central region of the wake is well calibrated, with increased r.m.s. error bands on the edges of the wake, though within margins of 1%. At  $u_{\text{rated}}$ , this representation is considered acceptable, with a 1% velocity variation on the outer wake unlikely to impact optimisation, considering the assumptions within these parameterisations.

Table 3 Calibrated wake parameters for Gaussian model.

$k_a$	0.1087
$k_b$	0.006912
$\alpha$	0.4886
$\beta$	0.2496

A comparative analysis of the wake parameters for  $u_{\text{rated}}$  against calibrations at varying flow speeds (Table 4) demonstrates that the overall r.m.s. error is still acceptable as the analytical wake model is applied within its expected



**Fig. 3** Relative difference between *Thetis* and *FLORIS* flow fields with positive magnitude (red) representing a higher *FLORIS* estimation of velocity. The area indicated by a black square on the left shows *Thetis* area of increased friction whilst the solid green line shows the *FLORIS* turbine representation. Black box on the right defines area over which r.m.s. error is calculated for calibration at  $u_{\text{rated}}$  alone, see Table 4.

range. With decreasing velocity, the wake width increases and as the velocity approaches cut-in speed the immediate wake width begins to exceed the turbine diameter, increasing the r.m.s. error, albeit within acceptable levels.

 Table 4 Comparison of the r.m.s. error between Thetis and FLORIS flow fields for calibration at rated speed alone vs. direct calibration at the velocity specified.

Velocity, $u \text{ (m/s)}$	r.m.s. error (%)									
	Rated Speed Calibration   Direct Velocity Calibration									
1.5	1.243	0.379								
2.5	0.756	0.325								
3.25	0.575	-								
4.5	0.130	0.099								

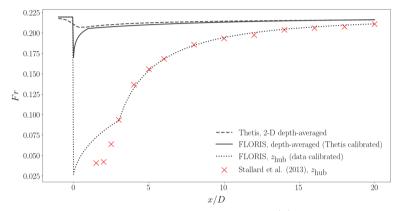
For completeness we present results of a separate calibration performed between the analytical wake model and flume data, which capture the full 3-D wake turbulence dynamics that are not captured within 2-D depth-averaged models. A comparison between the different wake behaviour and the respective FLORIS calibrated solutions are shown in Figure 4. Specifically, Figure 4 shows

- Thetis vs Thetis-calibrated FLORIS depth-averaged wake profiles;
- Stallard et al. (2013) data vs the corresponding calibrated *FLORIS* prediction for an isolated turbine at hub-height  $z_{\text{hub}}$ .

Froude-scaling has been applied for comparison against laboratory data (Stallard et al., 2013). Calibration to this data shows excellent agreement beyond  $\approx 3D-3.5D$  and therefore the potential to calibrate to 3-D data.

Even here however, the analytical representation of the near wake could be improved. This further highlights the challenge of calibration between *Thetis* and *FLORIS* as even on the depth-averaged profile, the immediate deficit

downstream of the turbine is substantially greater relative to *Thetis*. Nevertheless, the *Thetis* calibrated wake has been well-calibrated beyond 1.5D; since turbines are unlikely to be placed in such close proximities in the streamwise direction, this is unlikely to impact optimisation. For real world applications the initial wake calibration step should be conducted against the best possible wake data available, from observations and/or high-resolution CFD.



**Fig. 4** Longitudinal profile for wake Froude number  $Fr = \frac{|\mathbf{u}|}{\sqrt{gH_d}}$ , both depth-averaged for comparison between calibrated *FLORIS* vs *Thetis* and at hub height ( $z_{\text{hub}}$ ) for comparison between calibrated *FLORIS* vs experimental data of Stallard et al. (2013).

## 4 Case Studies

In demonstrating this tidal-array optimisation framework, we consider models of increasing complexity and denser turbine placement. First, we examine the micro-siting of aligned and staggered  $2 \times 4$  turbine arrays of three rotor diameter (3D) spacing between rows and columns; the array itself is situated within an idealised channel with and without a headland. These exercises are then repeated with denser  $3 \times 5$  turbine arrays of 2D lateral (between rows) spacing and 3D longitudinal (between columns) spacing. The idealised cases imitate two examples from Funke et al. (2014) and serve in validating the performance of the current approach prior to assessing a more realistic full-scale optimisation problem. For our realistic flow problem we consider the Pentland Firth region with aligned and staggered array sizes of  $4 \times 6$  turbines of 5D lateral and longitudinal spacing, followed by a staggered  $8 \times 6$  case of 3D spacing.

The cases are illustrated in Figures 5 and 6 respectively, including the computational meshes used by *Thetis* for the hydrodynamic simulations. In all cases, the mesh generation process employs the open-source code *qmesh* (Avdis et al., 2018), featuring a 3 m element resolution for the idealised cases, and 5 m for the Pentland Firth case within the allocated tidal array. This element size

was selected using a mesh sensitivity study on the wake resolution confirming the mesh resolution independence of the results within the array.

The optimisation approach is informed by several spatial conditions/ constraints. The "greedy" optimisation approach features an initial minimum of three diameters (3D) distance separating each turbine and a blocked radius of one diameter (1D) for each "failed iteration" ( $\Delta = 3D, E = 1D$ ). Initially, the 3D separation constraint between turbines is applied for all optimisation techniques to prevent high-magnitude flow deficits impacting closely spaced turbines (the spacing is typically 1.5D-5D for tidal turbines (Stallard et al., 2013)). However, towards making better use of the deployment area, the spatial constraint is then reduced to 1.5D to maximise the number of turbines within the domain. The conditions for 3D spacing are specified as a minimum 17.5% capacity factor per turbine, a 5% maximum reduction of power for individual turbines and a 9% maximum reduction in the cumulative power of the particular turbines subject to a power reduction ( $A = 17.5\%, B = 5\%, \Gamma =$ 9%) as required. As turbine interactions are inevitable for 1.5D spacing, constraint limits need to be less stringent with A = 17.5%, B = 15%,  $\Gamma = 25\%$ . Specific details for each case study are expanded below.

#### 4.1 Steady-state flow through an idealised channel

An idealised channel of dimension (640 m × 320 m) featuring a (320 m × 160 m) region where a tidal array is to be deployed, provides sufficient space to tightly pack turbines across the width of the channel, but is short enough to prevent substantial wake recovery. The bathymetry is constant across the full domain at 50 m depth. Eddy viscosity is set to a constant value of  $1 \text{ m}^2/\text{s}$  across the domain away from the boundaries as per previous studies (Vouriot et al., 2019). For simplicity, a quadratic drag coefficient  $C_D = 0.0025$  (which represents a fairly smooth bed is selected), following previous investigations of the Pentland Firth, e.g. (Draper et al., 2014). For this steady case, the imposed flow is constant and can be represented by a single flow field, which was determined in *Thetis* with an inflow velocity,  $u = 3.175 \text{ m s}^{-1}$  (close to  $u_{\text{rated}}$ ), and a constant elevation of 0 m at the outflow.

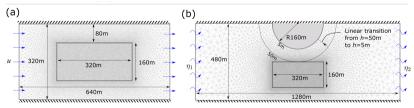


Fig. 5 a) Idealised rectangular channel; b) Idealised headland channel indicating bathymetric changes in the proximity of the headland.

#### 4.2 Transient flow around an idealised headland channel

Headlands and islands are key in providing highly energetic channels that make tidal streaming a feasible prospect. A simple headland model provides a location of concentrated higher energy density for turbines to be placed. In this case, the overall channel width and length are increased to  $480 \text{ m} \times 1280 \text{ m}$  for the headland (represented by a 160 m radius semi-circle) to be introduced. Velocity becomes greater due to flow conservation at the constriction from 480 m to 320 m, which acts in a similar manner as a Venturi flume, accelerating the flow. A bathymetric gradient is applied radially, with the depth reduced gradually from 50 m to 5 m along the headland, imitating a shore. Simple harmonic signals are defined (Eq. 19, Eq. 20) to drive the oscillatory flow to verify *FLORIS*'s capability to optimise for multiple fields of data. The following equations

$$\eta_1 = A_{\text{tide}} \cdot \sin\left(\frac{2\pi t}{T}\right),\tag{19}$$

$$\eta_2 = -A_{\rm tide} \cdot \sin\left(\frac{2\pi t}{T}\right),\tag{20}$$

provide the assigned local elevation  $\eta_1, \eta_2$ , at each of the boundaries and are signals of equal magnitude and opposite direction. Here,  $A_{\text{tide}}$  is the tidal amplitude, t is the simulation time and T is the tidal period. Values of T = 1 h, and  $A_{\text{tide}} = 0.275$  m, deliver a velocity profile with a peak magnitude close to  $u_{\text{rated}}$  (i.e. 2.5–3 m/s). Following a spin-up time of 1.5 hours, fields exported for optimisation are between the cut-in and maximum speeds, over a single tidal cycle.

#### 4.3 Application to the Pentland Firth, Scotland, UK

The Orkney archipelagos in north Scotland, UK (Figure 6) features sites characterised by high tidal energy levels. This is especially pronounced in the area of Pentland Firth, a strait separating mainland Scotland from the Orkney Islands. There, flow speeds regularly exceed  $5 \text{ m s}^{-1}$  (Draper et al., 2014) and thus the Inner Sound of the Pentland Firth is a prime site for tidal array deployment as discussed by several studies investigating the energy resource (Adcock et al., 2013; Draper et al., 2014; O'Hara Murray and Gallego, 2017), potential environmental impacts (Martin-Short et al., 2015a) as well as the micro-siting of turbines within arrays (Funke et al., 2014). At that location is the Meygen site, where a subset of a larger array has already been deployed.

The regional hydrodynamic model shown in Fig. 6a makes use of one arcsecond resolution bathymetry, acquired from Edina Digimap Service (Edina Digimap Service, 2020). Open boundaries are tidally forced using eight tidal constituents (Q1, O1, P1, K1, N2, M2, S2, K2) derived from TPXO (Egbert and Erofeeva, 2002). The model, subjected to 2 days of spin-up time, hindcasts 32 days from 01/08/2017 to 01/09/2017. This timeframe is selected according to the availability of ADCP data (Coles et al., 2018), spanning sufficient

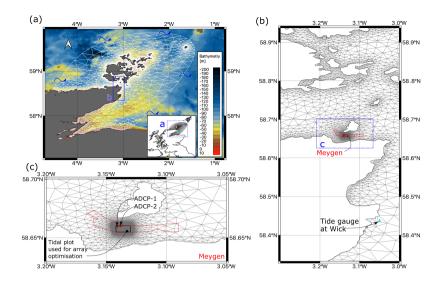


Fig. 6 Computational domain for the Pentland Firth case study. a) Domain extents and Marine Digimap bathymetry dataset (Edina Digimap Service, 2020) interpolated to elements; b) close-up to island scale; c) close-up to channel scale. The tide gauge and ADCP locations used to calibrate the model are also indicated, alongside the tidal array deployment zone considered for array optimisation.

duration to resolve the principal constituents driving the flow (i.e. M2 and S2). Over this period, predictions are simultaneously compared against UK Hydrographic Office data recorded at a tide gauge located at Wick (Table 5).

Table 5         Comparison between observed and predicted values of principal tide constituents
M2 and S2 at Wick tide gauge and ADCP locations.

Location	Constituent	Amplitu	de $\alpha$ (m)	Phase $\phi$ (°)					
		Observed	Predicted	Observed	Predicted				
Wiels	M2	1.02	1.03	322.30	322.45				
Wick	S2	0.35	0.37	0.30	359.56				
ADCP-1	M2	2.59	2.86	239.90	236.94				
ADOF-1	S2	1.02	1.12	278.22	293.60				
ADCP-2	M2	2.66	2.66	235.90	237.07				
ADCP-2	S2	0.92	0.97	297.32	300.47				

The Pentland Firth and Orkney Isles model for our optimisation study has an element size ( $\wedge h$ ) ranging between 300–1,500 m near-shore subject to proximity to the Meygen tidal site or certain island features. This resolution gradually increases to 20,000 m towards the open seaward boundaries. Increased refinement of a uniform element size  $\wedge h = 5$  m has been imposed to resolve individual turbines within the Meygen tidal site. The simulation results are produced using a variable Manning's  $n_M$  across the domain based on bed

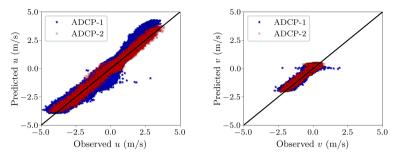


Fig. 7 Left: correlation between observed and predicted *u*-velocity at Pentland Firth monitoring station ( $R^2 = 0.93$  and  $R^2 = 0.98$  for ADCP-1 and ADCP-2, respectively). Right: correlation between the observed and predicted *v*-velocity ( $R^2 = 0.95$  and  $R^2 = 0.84$  for ADCP-1 and ADCP-2, respectively). ADCP data provided by SIMEC Atlantis Energy with further details in Coles et al. (2018).

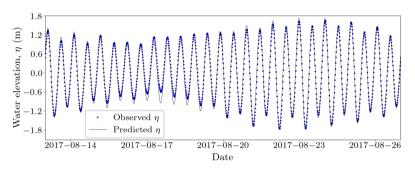


Fig. 8 Water elevation at the Wick tide gauge from 13/08/2017 till 27/08/2017. Thetis predictions (continuous line) are compared against observed water elevation obtained (circles) with  $R^2 = 0.982$  and r.m.s. error = 0.11 m.

classification data provided by the British Geological Survey, as described by Mackie et al. (2021a), and a timestep  $\Delta t = 100$  s.

Table 6 Results of optimisation methodology applied to each case. The average power $P$ calculated based on testing layouts in $Thetis$ .	$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	3 <i>D</i> - 13.54 -	3D - $15.85 (+17.0%)$ -	1  15.99 (+18.1%)	1 15.98	N/A $ $ 16.00 $(+18.2%)$		2D -   28.97 (+15.9%) -	$1  1  29.81 \ (+19.2\%)$	.5D 1 1 29.22 $(+16.9%)$ 3.3	N/A 29.96 $(+19.8\%)$ 1		- 5.44 (	6 5.61 (	3D 6 5.53 $(+27.4%)$ 0.3 0.3	1	- 8.83 (	6   10.06 (+28.6%)   3		•	5D - 20.55 (+0.4%) -	18 21.72 $(+6.1%)$ 4	18 23.01 (	18 23.71 (+15.8%)	-	
'erage power $P$ calculated based o	Minimum Spacing L <sub>min</sub>	3D -	3D -	LSQP) $3D$ 1		) 3D	2D -	2D -			) 1.5D	3D -	3D -	) $3D$	) $3D$	2D -		) 1.5D	1.5D	5D -	5D -	) $3D$	(Greedy) $3D$ $18$	1.5D		15D 15D 10
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able 6 Results of opt.	ID Case	A.1 Channel <sup>4</sup>	A.2 Channel	A.3 Channel	A.4 Channel	A.5 Channel	A.6 Channel	A.7 Channel	A.8 Channel	A.9 Channel	A.10 Channel	B.1 Headland <sup>5</sup>	B.2 Headland		B.4 Headland		B.6 Headland	B.7 Headland	B.8 Headland	C.1 Pentland Firth	C.2   Pentland Firth		C.4 Pentland Firth	C.5 Pentland Firth	C.6 Pentland Firth	C 7 Dontland D:

<sup>3</sup>All optimisation simulations were run on a single core. Specification: Intel (R) Xeon (R) Gold 5118 CPU 2:30GHz. <sup>4</sup>Thetis run for 39.3 min of time on 1 core for confirmed convergence to steady state (3.5 hour simulation time).

 $^5\mathrm{Thetis}$  run time of 27.4 min on 4 cores, for 8 hours simulation time.

 $^{6}\mathrm{Thetis}$  run time of 1337.1 min on 12 cores, for a 30 day simulation time.

Model calibration is more sensitive against measured velocity rather than elevation data. Velocity comparisons were established against observed data at the locations of ADCP-1 and ADCP-2 (Figure 6). In Figure 7 a misalignment can be observed in ADCP-1 during flood tide. This is attributed to several modelling decisions, such as the coarse model resolution surrounding the Meygen site and the rest of the computational domain. In addition, the relatively low resolution of the available bathymetric dataset used in the vicinity is influential. Nevertheless, the overall model accuracy is deemed appropriate for demonstrating the optimisation method within a practical scenario, acknowledging that these deviations between observational and model data would render further field observation and analysis essential to characterise the local dynamics more accurately. In terms of water elevation predictions, results agree well (Figure 8) with observed values at Wick. There, correlation among observed and predicted water elevation data is approximately 0.982, while the root-mean-square (r.m.s.) error is equal to 0.11 m.

## 5 Optimisation Results

#### 5.1 Steady-state flow through an idealised channel

The maximum power possible for setups of  $N_t = 8$  and  $N_t = 15$  turbines would be 16 MW and 30MW respectively, given that the inflow  $(3.175 \text{ m s}^{-1})$ exceeds  $u_{\text{rated}} = 3.05 \text{ m/s}$ . Indicatively, an aligned turbine placement (Figure 9a) yields power of > 15% below the maximum extractable power for both 8 and 15 turbine setups. Placing the turbines in the staggered arrangement of Figure 9a leads to improved power output as anticipated, slightly below the maximum achievable.

Layout optimisation in *FLORIS* using SLSQP leads to a distribution of turbines across the channel width. This is shown in Figure 9b for  $N_t = 15$  setups (A.8-10), forming two rows of turbines separated by a sufficient longitudinal distance that allows velocity deficit recovery from upstream devices. Using the greedy approach provides a similar result in both cases, with turbines positioned to avoid wake interaction where possible. The SLSQP approach performed best in completing optimisation due to the simplicity of the input, whilst the greedy algorithm demonstrates sensitivity to the naive initial turbine placement. This is particularly notable on the  $N_t = 15$  setup (A.9), whereby two columns of turbines are required and therefore poor initial placement could negatively impact array power to a much greater extent.

A Thetis adjoint-based tidal farm optimisation (Funke et al., 2014) (A.10) provides similar distributions of turbines across the channel as in Figure 9, but with placement that appears to exploit the "duct effect". Adjoint optimisation results in maximum power obtained across all approaches (as per *Thetis* simulations) since optimisation avoids inconsistencies in turbine representation, whilst also capitalising on beneficial hydrodynamic effects. The adjoint/gradient-based method was anticipated to be more effective in this case for the above reasons, particularly around (or below)  $u_{rated}$ , whereby the

velocity deficits can reduce the power produced. This is emphasised for the denser 15-turbine layout, where consideration of more devices places greater stress on the optimisation technique, while blockage and funnelling provide opportunities for greater power augmentation. Nevertheless, despite different layouts, the power performance is near identical among 8-turbine cases (A.3-5) and very similar between *FLORIS*'s SLSQP (A.8) and the adjoint (A.10) for the 15-turbine cases. This suggests that multiple solutions achieve the criterion of maximising power output, but with the adjoint taking significantly longer than either *FLORIS* approach.

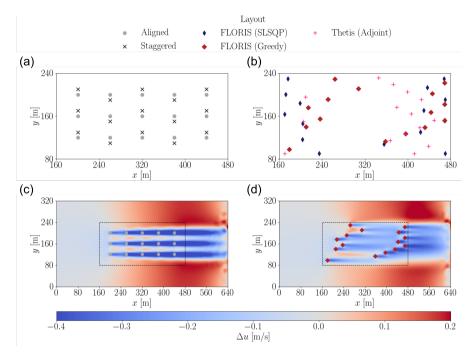


Fig. 9 Array layouts  $(N_t=15)$  and *Thetis*-predicted velocity deficits  $(\Delta u)$  for rectangular steady-state idealised channel flow. a) standard (i.e. unoptimised) layouts (A.6-A.7, Table 6); b) optimised layouts (A.8-A.10); c) aligned layout (A.6)  $\Delta u$ ; d) greedy optimisation layout (A.9)  $\Delta u$ .

#### 5.2 Transient flow around an idealised headland

The idealised headland case considers oscillatory flow to demonstrate optimisation features over unsteady conditions. Three flow fields from each flood and ebb tide are exported to be used within *FLORIS*. For each of these sets, one is at peak velocity magnitude and two between cut-in and rated speeds. As flow direction and magnitude does not vary significantly, the total of six flow 'frames' performs sufficiently for optimisation in this case, with more frames delivering negligible benefit. Velocity contours for the peak flood flow without

turbines are shown in Figure 10 with layouts of different headland cases for the  $N_t = 15$  configurations (B.5, B.6, and B.7) of Table 6 superimposed. As the flow develops around the headland, the combination of the *vena contracta* effect and the bathymetric gradient contribute to a velocity acceleration that diminishes away from the headland constriction. This provides radial bands of higher energy potential for turbine placement, with only the regions closest to the headland allowing maximum power production at peak flow speeds.

FLORIS's SLSQP based optimisation leads to placement of three turbines within these first two bands (i.e. in flow greater than  $3 \text{ m s}^{-1}$ ) for the  $N_t = 8$ setup (B.3), with the remainder of turbines spread across the width of the channel avoiding wake interaction in a similar manner to the steady-state idealised channel case. Meanwhile, greedy optimisation places the first turbine in the centre of the first band, subsequently leading to lower power production for the surrounding turbines, which can not be placed as closely within the first two bands due to the separation constraint. A similar trend is observed with 15 turbines and a reduced separation constraint of 1.5*D*; five turbines are placed within the first two bands by SLSQP and only two by the greedy algorithm (Figure 10).

The average power produced by the greedy optimisation array after only 20 iterations (for the  $N_t = 8$  setup of B.4) exceeds the staggered arrangement (B.2), which itself performs particularly well due to the flow direction. However, the greedy optimisation technique leads to 1.8% lower average power than SLSQP, although it does require almost 0.1% and 2% of the computational time for  $N_t = 8$  and  $N_t = 15$  turbine setups respectively. Given the required time for a steady state channel optimisation, the reduction in computational time becomes appreciable relative to adjoint optimisation, which has not been explored further in this work for unsteady cases.

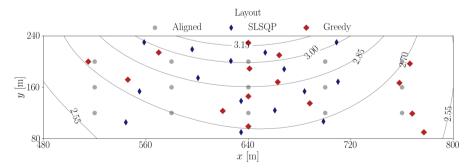


Fig. 10 Aligned (B.5) and optimised layouts (B.7, B.8) overlaid on velocity contours for peak flow within the idealised headland channel.

#### 5.3 Application to the Pentland Firth, Scotland, UK

Using three frames from each flood and ebb tide for spring, intermediate and neap cycles, optimisation of  $N_t=24$  turbines subject to a minimum spacing of 3.0D (C.4, Table 6) resulted to increased average P relative to an aligned case (C.1) by 12.4% over a month's period. Optimisation made use of 18 flow frames, with additional frames delivering no substantial benefit to the overall performance. This is attributed to the generally consistent flow direction at peak magnitudes over flood and ebb tides (as illustrated by the flow fields in Figure 11). The importance of using representative frames for a full tidal cycle (as well as a spring-neap cycle) is demonstrated by Figure 13. Optimisation is less effective during flood flows, and even less so during spring tides. This is due to the deployment of turbines that experience flow velocities noticeably greater than  $u_{\text{rated}}$  for a large proportion of the velocity magnitudes expected within the allocated plot. As a result, these are predicted to deliver maximum power irrespective of compounded wakes. This would suggest that within this area, neglecting structural constraints and metrics such as the capacity factor, it may be worth using turbines of higher  $u_{\text{rated}}$  to fully exploit the potential energy available. Nevertheless, a positive increase in capacity factor from 42.6%to 47.9% is achieved for a minimum spacing of 3D that could have a significant influence on the feasibility of such an installation.

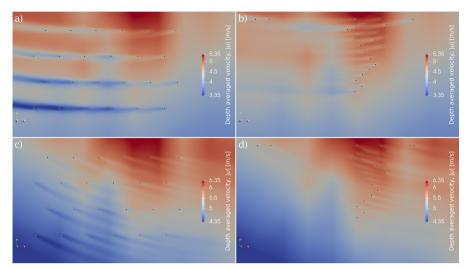
Greedy optimisation accomplished this improvement within 27 iterations, taking less than a minute. In loosening the minimum spacing constraint to 1.5D, a capacity factor increase of 49.4% is achieved, at the cost of additional iterations and computational time in the order of a few minutes. As Table 6 shows, in a practical case where velocities exceed  $u_{\text{rated}}$  and the flow field is more varied and complex largely due to the local bathymetric profile, a staggered arrangement (C.2) is less effective than in idealised geometries (e.g. A.2 and B.2). With more turbines within the staggered arrangement, the interaction of multiple wakes has a far more profound effect as shown in Figure 12a,c; the variation of flow velocities means that turbines perform better packed into regions of higher average kinetic power density ( $\overline{\text{KPD}}$ , where KPD  $=\frac{1}{2}\rho|\mathbf{u}|^{3}$ ). Again, this emphasizes the optimisation's sensitivity to turbine  $u_{\text{rated}}$ ; in regions where peak flow speeds regularly exceed  $u_{\text{rated}}$ , turbines will operate at their maximum capacity, despite wake impingement. For an array of doubled size  $(N_t = 48)$ , the relative increase in power becomes less significant for this reason. Although the initial staggered arrangement at 3D spacing (C.6) appears to allow for greater wake avoidance than the  $N_t = 24$  array of 5D spacing (C.2) due to the localised flood direction, the increased density of turbines at the northern, more energy dense, section of the site in combination with low  $u_{\text{rated}}$  relative to the flow speed, corresponds to lesser noticeable gains. This issue leads to denser turbine arrangements, but is specific to layout optimisation seeking energy maximisation rather than mitigating hydrodynamic (wake) interactions. Figure 11d sees turbines positioned in areas of high KPD, whereby the southern parts of the site are avoided on the grounds of

lower flow magnitudes. Notably, beyond the allocated area for turbine deployment, flow speed exceeds  $5.5 \,\mathrm{m \, s^{-1}}$  towards the island of Stroma (Figure 6). Harnessing the kinetic energy there would be technically challenging due to the shallower bathymetry and sharper bed gradients. In the optimised configurations, few turbines lie within the high velocity deficit region of upstream wakes due to conditions preventing the reduction of individual turbine power. This is particularly critical during neap tide when flow speeds are low enough to place emphasis onto wake interaction, hence the benefit of optimising for several varying tidal cycles.

A key consideration when examining practical cases is the impedance of the flow due to the presence of turbines (array blockage). The change in volumetric flow over a 1-day period is presented in Table 7 to quantify the impact of the turbine drag on the channel flux, as per Coles et al. (2017). Through the array width only, the reduction in volumetric flow remains around 4% for the 24 turbine cases, which is not particularly significant for a spring tide and results partially from the low global blockage of the array and the spaced out distribution of turbines to minimise velocity deficits. As the array size and its turbine density is relatively small when compared to the size of the channel, the influence on the Inner Sound is localised suggesting minimal diversion of flow on a regional scale. With increasing array size (Figure 12e-h), the impact of global blockage effects will likely become more critical, particularly considering site-to-site interactions over the entire Pentland Firth and the Orkney Islands (De Dominicis et al., 2018). Although still sparse relative to the size of the Inner Sound, the reduction in volumetric flow through both the array width and the Inner Sound itself doubles when  $N_t = 48$  in case C.7, demonstrating a proportional increase in blockage effects.

Table 7         Change in volume flux with the introduction of the greedy array layout, over
transects of the array width and Inner Sound for a Spring cycle. Different transects are
used for the ebb, flood and overall volume flux changes to best account for the impact of
the array in each case. A negative change represents a decrease in flow through the channel
when turbines are introduced.

Optimised Layout	Cycle	Volume flux change, $\Delta Q/Q_{\rm amb}$ (%)					
		Array Width	Inner Sound				
	Ebb	-3.33	-0.47				
C.4 (24  turbines, 3D)	Flood	-3.59	-0.17				
	24 hours	-3.66	-0.57				
	Ebb	-4.19	-0.55				
C.5 (24 turbines, $1.5D$ )	Flood	-3.31	-0.20				
	24 hours	-3.86	-0.61				
	Ebb	-7.72	-1.17				
C.7 (48 turbines, $1.5D$ )	Flood	-6.76	-0.32				
	24 hours	-7.39	-1.00				



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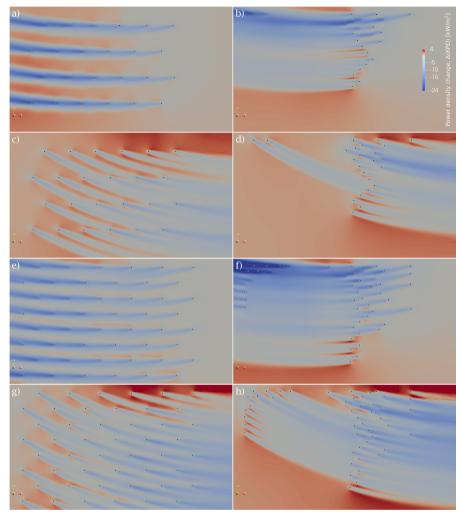
Fig. 11 Flood and ebb flow for Pentland Firth case study at peak spring tide computed using the *Thetis* model with turbine drag simulated. a) staggered arrangement (C.2), ebb; b) greedy optimised arrangement (C.5), ebb; c) staggered arrangement (C.2), flood; b) greedy optimised arrangement (C.5), flood.

## 6 Discussion

### 6.1 On the turbine representation and the consideration of local and global blockage

The general array micro-siting pattern returned by the optimisation approaches (SLSQP and greedy alike) sees turbines positioned within high power density regions and otherwise spread to maintain separation whilst avoiding wake interaction. This also agrees with results reported previously by Stansby and Stallard (2016) that emphasise wake avoidance within the optimisation process.

Under operational conditions below  $u_{\text{rated}}$ , variation in wake representation can compromise optimisation, as key velocity deficit areas may not be captured accurately. If the wake width is underestimated in the analytical model, then some of the turbines may become partially immersed in upstream wakes when evaluated by the hydrodynamic model. This highlights the significance of calibrating the wake model parameters as per Section 3.2. Additional parameters can be considered to improve accuracy, such as varying the turbulent intensity,  $\mathcal{I}$ , as a function of the flow magnitude, for better agreement against data. These were assumed constant in this study for simplicity, but should be calibrated accordingly as they are subject to inflow conditions and varying turbulence levels. The inclusion of local blockage effects, which has been shown to be possible through ad-hoc corrections in analytical approaches such as FLORIS (Branlard and Meyer Forsting, 2020), could also benefit optimisation in high-density, confined scenarios.



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Fig. 12 Flood and ebb change in kinetic power density (KPD) for Pentland Firth case study at peak spring tide computed using the *Thetis* model with turbine drag simulated. a) staggered arrangement (C.2), ebb; b) greedy optimised arrangement (C.5), ebb; c) staggered arrangement (C.2), flood; d) greedy optimised arrangement (C.5), flood; e) staggered arrangement (C.6), ebb; f) greedy optimised arrangement (C.6), ebb; g) staggered arrangement (C.7), flood; h) greedy optimised arrangement (C.7), flood.

In the case study within the Pentland Firth we consider a turbine array subject to a rating curve, varying flow directionality and assuming practical clearance constraints. Firstly, we note the necessity of the ambient flow model to correctly capture the flow magnitude and directionality, particularly over the tidal array deployment area as discussed in Section 4.3. Otherwise, minor departures from the actual flow direction could lead to a suboptimal

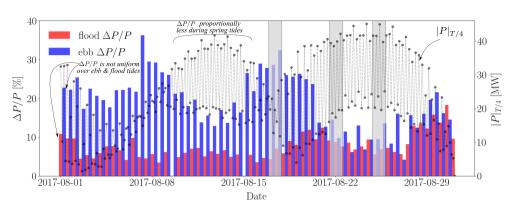


Fig. 13 Relative power increase  $(\Delta P/P)$  comparing greedy (C.4) to staggered (C.2) case of the Pentland Firth for 24 turbines. Shaded areas indicate periods used to extract ambient flow fields for optimisation.  $|P|_{T/4}$  corresponds to the average power in increments of T/4. Similar trends are observed comparing optimised layouts based on minimum 1.5D spacing (C.5 vs C.2, C.7 vs C.6).

array design (see Figure 7). These deviations are typically attributed to underresolved spatial features as discussed by Mackie et al. (2021a). We also observe that because of the varying flow direction over ebb and flood tides, blockage becomes challenging to exploit. Considering the flow velocity across the array, we note the persistent exceedance of the rated velocity,  $u_{\text{rated}}$ , across spatial and temporal scales. When velocities regularly exceed  $u_{\text{rated}}$ , wake effects become locally constrained, and compounded by changes of the flow direction by rendering the "duct effect" difficult to take advantage of and thus less influential on power production. Based on standard practices, turbines are proposed to be placed in channels where peak flow speeds comfortably exceed  $u_{\rm rated}$ . This is due to alternative objectives, such as maintaining a competitive capacity factor over the device lifetime. Therefore, if a significant proportion of power is to be extracted above  $u_{rated}$ , local blockage and the "duct effect" become less important. In addition, during spring tides when  $|\mathbf{u}| > u_{\text{rated}}$  over a longer fraction of the tidal cycle, optimal siting of turbines becomes less beneficial in terms of power output.

Furthermore, minimum turbine spacing may likely be forced to exceed 3D (Ouro et al., 2019) in the in-stream direction (the value used in this study as typical separation constraint). Moreover, the minimum spacing of 1.5D that enables closer packing of turbines laterally, as in Ouro et al. (2019), can be challenging to accommodate due to O&M practicalities, complex terrain constraints and non-rectilinear oscillatory flow. These considerations can restrict turbine placement and reduce to an extent the positive influence that local blockage may achieve. On that, Nishino and Willden (2013) analytically found that with increasing turbine density in a partial tidal fence, the optimal local blockage will increase for both low and high global blockage cases. The benefit of exploiting blockage effects was demonstrated numerically in Funke et al.

(2014) where an adjoint-optimisation approach promoted positioning of idealised turbines (i.e. not subject to a  $u_{rated}$ ) to form highly dense fence-like structures. It must be remarked that in that case, the resistance introduced by individual turbines was exaggerated (as the focus was instead on demonstrating the adjoint optimisation methodology), amplifying the benefits of local blockage. However, within the steady-state flow through an idealised channel, and whilst considering more practical representations of turbine resistance and turbine density (3D), the adjoint optimisation in *Thetis* delivers minimal gains over our greedy or *FLORIS*'s SLSQP-based approaches.

A feasible placement of turbines within a channel such as the Pentland Firth is likely to be highly dependent on a number of factors including the bathymetry gradient, bedrock hardness, turbulence loading and a variety of installation and maintenance challenges. The initial turbine density for the Pentland Firth case study was based on an initial separation of 5D. If the density is increased so that maximum initial separation is reduced to 3D (whilst increasing the number of turbines in the initial array), local blockage effects become more prominent, as indicated by the increase in power density around devices in Figure 12. Nevertheless, quantification of the blockage effects by monitoring fluxes and the power density changes over the array (Table 7) suggests that this remains a low blockage case.

#### 6.2 On the characterisation of array hydrodynamics

The optimisation approach described here relies on the use of analytical wake models that typically assume steady-state conditions. The practice of wake superposition itself introduces a mass and momentum deficit that is not compensated without additional corrections (e.g. Branlard and Meyer Forsting (2020); these necessitate the assessment of *FLORIS* derived layouts within hydrodynamics models. On the other hand, the hydrodynamics model (in this case *Thetis*) does not capture horizontal flow structures below the mesh-size scale which means that many unsteady and quasi-steady flow phenomena are not considered in our analysis. This may have implications for the final prediction of the wake deficits and therefore also affect the optimal array layout solution. One phenomenon of relevance which is not captured in our simulations is dynamic wake meandering. As turbine wakes interact with the larger tidal-channel turbulent structures, such as near-wall high- and low-speed streaks, near-wake vortices start breaking down giving way to the generation of a cascade of turbulent scales. Additionally, the wake experiences lateral and vertical displacements caused by the larger-scales leading to their significant lateral expansion. These effects are not encapsulated within hydrodynamic models unless the model spatial and temporal resolution is increased and/or combined with more robust turbulence models that capture these effects while avoiding excessive dissipation in the solution. Inherently, all 2-D models are limited in their ability to capture dispersion effects due to the assumed uniform vertical velocity. 3-D shallow-water models on the other hand, can improve the representation of such scales as shown by  $\frac{1}{2003}$  through the addition of a horizontal mixing length scale which alters the velocity profile over the water column, resulting in greater vertical shear; however, further research is required in order to quantify their impact on wake dynamics.

Regarding the global array wake, experimental studies on turbulent wakes downstream of a two-dimensional porous obstruction (Zong and Nepf, 2012) show that the steady wake region increases with increasing porosity whereas the unsteady von Kármán vortex street may be delayed until well beyond the steady wake region. Given the low turbine density, as demonstrated by the global array volume flux (see Table 7), the array's equivalent porosity is small, thus we argue that no further quasi-steady effects are likely to be present in the array-scale wake region. Turbine-scale unsteadiness in the individual turbine's near wake region may be accounted for by a locally modified eddy-viscosity.

An alternative approach for the local and global hydrodynamics may be undertaken using higher-fidelity models such as those that utilise threedimensional Reynolds-averaged Navier–Stokes (RANS) (Abolghasemi et al., 2016: Deskos et al., 2017) or large-eddy simulation (LES) methods (Churchfield et al., 2013; Ouro and Nishino, 2021) which inherently allow for greater insight and accuracy in the near-wake region by allowing both horizontal and vertical wake dispersion through scale-resolving simulations. Such simulations emphasise how wake avoidance is not only critical for maximum exploitation of the channel potential, but also in reducing turbulence onto downstream turbines which may compromise the devices' lifetime due to fatigue (Thiébaut et al., 2020). Nevertheless, 2-D models are currently the standard option for regional assessments (Coles et al., 2020) and help counteract the computational cost within an optimisation framework. As demonstrated in Section 3.2, and in particular Figure 4, it would be entirely possible to apply the same methodology to a 3-D higher-fidelity either coastal ocean or turbulence-resolving model to acquire greater consistency with measured data.

## 6.3 On the potential applications for large tidal array optimisation

Whilst a number of optimisation approaches have been proposed for the micrositing of tidal turbines, these have been limited to idealised setups, or limited control parameters in terms of turbine placement. Some of the more sophisticated methods (e.g. Funke et al. (2016); Culley et al. (2016)) that consider blockage effects remain computationally and memory intensive. Taking our practical example of the Pentland Firth, an earlier approach required 24– 48 hours on a 64-core supercomputer for a steady state simulation (Funke et al., 2014). Though pioneering, practical constraints including rated turbines, transient tidal flows and realistic bathymetry were not considered in early studies despite having an influence on the interactions between devices and the resource. Similarly, optimisation methods that estimate the objective function gradient iteratively (e.g. SLSQP), quickly become computationally demanding due to the quadratic complexity ( $\mathcal{O}(n^2)$ ) of the optimisation algorithm. In the optimisation problem presented by the practical case (C.3), SLSQP becomes

significantly more costly as the domain size and turbine number  $N_t$  increase. Given a tendency to converge to local minima it becomes distinctively ineffective for complex domains in the absence of a reliable gradient calculation strategy. The customised greedy approach developed here overcomes these computational constraints and offers a route to also incorporate additional features. These may include cabling constraints (Culley et al., 2016), seabed gradient restrictions, several turbine options and other factors such as wake steering which are considered in the optimisation of offshore wind farm operation (Deskos et al., 2020). However, greedy optimisation strategies possess shortcomings (Bang-Jensen et al., 2004), and whilst they can deliver a locally optimal solution in reasonable time, they must be applied and interpreted with their limitations in mind.

Adjoint-based and greedy methods could be combined in a cyclic manner for optimisation in larger domains whereby a greedy approach acts as a precursor that delivers an initial design to improve upon through adjointoptimisation. This will sequentially seek to exploit hydrodynamics effects by exploring the parameter space through localised turbine displacements starting from a decent design that should result in the requirement for less optimisation iterations than a pure adjoint-based approach. It may also mitigate the issue of getting stuck in a sub-optimal local optima. Alternatively, given the computational efficiency of the customised greedy optimisation, opportunities can be explored to optimise for N turbines at a time in subsets of the turbine deployment area. Turbines introduced can then be included in forward hydrodynamics simulations to account for hydrodynamic impact and blockage effects when designing the rest of the array. Extensions can thus be made towards multi-objective optimisation that balance cost against environmental impacts such as sediment transport or implications for benthic species habitats. This could follow recent work on environmentally constrained optimisation by du Feu et al. (2019).

## 7 Conclusions

A novel optimisation method was demonstrated by retrofitting an analytical wake superposition model, in this case *FLORIS*, for use with a coastal hydrodynamics model, *Thetis*. The method is motivated upon reflection on the bottlenecks observed in existing array optimisation approaches, which depending on acceptable computational costs may be constrained to (a) simplified flow geometries, (b) steady-state flow conditions and (c) idealised turbine representations. The work is driven by the complexity of the array micro-siting problem, where an effective optimisation method should be able to deal with complex flows caused by local bathymetric features and regional coastline, the transient tidal flows over spring neap cycles, and the technical specifications and performance characteristics of the turbine technology that is to be deployed. Once a hydrodynamic model delivers the spatially and temporally varying flow information over a prospective development area, application of a custom greedy placement algorithm within an analytical wake superposition model allows for rapid optimisation.

The methodology was applied to three cases of increasing complexity (in terms of geometry, oscillatory flow, and array turbine number) and was able to demonstrate its potential and highlight multiple considerations emerging as we progress from idealised to practical settings. For a simple steady-state rectangular channel, turbines were arranged in a longitudinally staggered fashion across the domain, utilising the full width of the domain whilst maintaining separation constraints, consistently with alternative optimisation strategies (e.g. SLSQP and adjoint-based optimisation). The headland case demonstrated the capacity to deal with more complicated flows and emphasised the trend of turbines being positioned in areas of higher power density, whilst avoiding wake effects from upstream turbines during ebb and flood flows. The optimisation scenario of 24 turbines in a confined region within the Pentland Firth demonstrated the ineffectiveness of staggered arrangements for nonrectilinear oscillatory flows, and the computationally efficient application of this methodology for complex geometries and flow dynamics. It was found that the resultant method yielded an overall improvement in power output in the order of 12% for 3D minimum spacing and up to almost 16% when reduced to 1.5D.

Finally, it was observed that flow asymmetry in conjunction with minimum distance requirements may render the exploitation of local blockage effects rather challenging. Case studies using 24 and then 48 turbines respectively within the Meygen site at the Pentland Firth indicated low levels of global blockage. However, as the number of turbines doubles to 48 in the latter case, blockage effects start to become more noticeable. Given the extensions expected as tidal arrays expand, it is proposed that the optimisation approach presented can be operated iteratively enabling the hydrodynamic model to account for array-scale blockage as the size of the array is extended.

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- Conflict of interest/Competing interests The authors have no conflict of interests to declare
- Ethics approval Not applicable

- Consent to participate All authors have provided their consent to participate in this study
- Consent for publication All authors provide consent for publication of the present work.
- Availability of data and materials Data is available upon request.
- Code availability The software used can be accessed in https://github.com/thetisproject/thetis and https://github.com/NREL/floris. Further information can be provided upon request.
- Authors' contributions Connor Jordan: Methodology, Formal Analysis, Investigation, Writing Original Draft, Writing Review & Editing, Software, Visualisation. Davor Dundovic: Methodology, Formal Analysis, Software, Writing Review & Editing. Anastasia Fragkou: Validation, Writing Review & Editing. Georgios Deskos: Conceptualisation, Software, Writing Review & Editing. Daniel Coles: Data Curation, Writing Review & Editing. Matthew Piggott: Conceptualisation, Methodology, Writing Review & Editing, Supervision, Funding Acquisition. Athanasios Angeloudis: Methodology, Software, Investigation, Funding Acquisition.

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