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Combining shallow-water and analytical wake models for tidal array micro-siting

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Abstract

For tidal-stream energy to become a competitive renewable energy source, clustering multiple turbines into arrays is paramount. Array optimisation is thus critical for achieving maximum power performance and reducing cost of energy. However, ascertaining an optimal array layout is a complex problem, subject to specific site hydrodynamics and multiple inter-disciplinary constraints. In this work, we present a novel optimisation approach that combines an analytical-based wake model, *FLORIS*, with an ocean model, *Thetis*. The approach is demonstrated through applications of increasing complexity. By utilising the method of analytical wake superposition, the addition or alteration of turbine position does not require re-calculation of the entire flow field, thus allowing the use of simple heuristic techniques to perform optimisation at a fraction of the computational cost of more sophisticated methods. Using a custom condition-based placement algorithm, this methodology is applied to the Pentland Firth for arrays with turbines of 3.05 m s^{-1} rated speed, demonstrating practical implications whilst considering the temporal variability

of the tide. For a 24 turbine array case, micro-siting using this technique delivered an array 15.8% more productive on average than a staggered layout, despite flow speeds regularly exceeding the rated value. Performance was evaluated through assessment of the optimised layout within the ocean model that treats turbines through a discrete turbine representation. Used iteratively, this methodology could deliver improved array configurations in a manner that accounts for local hydrodynamic effects.

Keywords: Array optimisation, Tidal turbines, *FLORIS*, Shallow water equations

1 Introduction

The levelised cost of energy (LCOE), defined as the average net present cost of electricity generation for a power plant over its lifetime, is often cited as a key metric for the competitiveness of an energy technology. Unless there is a rapid increase in installations, the LCOE for tidal-stream is set to remain at more than £150/MWh by 2025 (Smart and Noonan, 2018; Topper et al., 2021), whilst the LCOE for solar and both onshore and offshore wind will fall to approximately £25–£32/MWh (U.S. Energy Information Administration, 2020). Reducing LCOE is paramount if tidal-stream energy is to become a competitive, sustainable energy source (Coles et al., 2021). This could be achieved through several measures (Coles and Walsh, 2019; Goss et al., 2020, 2021a,b): (i) physical infrastructure improvements, which could involve optimisation of the turbine design and operation, (ii) economies of scale in turbine design, (iii) economies of volume in manufacturing, operation and maintenance, (iv) technology innovation, (v) learning, and (vi) financing mechanisms. Turbines have now reached technology readiness levels of 7–8 (Chozas, 2015; SIMEC Atlantis Energy, 2020a) and need to be tested in large arrays for extended periods of time in order to reach full maturity and facilitate implementation of the aforementioned cost reduction mechanisms. In supporting this, strategies should be investigated and developed for the reliable assessment of the tidal resource (Neill et al., 2014; Robins et al., 2015; Neill et al., 2018; Mackie et al., 2021b) to reduce investment uncertainty, as well as array design optimisation to maximise performance. Array optimisation has already shown potential to increase array power by up to 33% relative to a regular aligned layout, albeit with power capping removed (Funke et al., 2014). Hence developing more robust, yet practical optimisation methods could be a key step to achieving further LCOE reductions (Coles et al., 2021).

Array power can be associated with up to eight controlling array effects, as outlined in Vennell et al. (2015). These include the reduction of free-stream velocity by the introduction of turbines and the relative size of the array in the channel. This leads to conflicting design performance interactions among turbines, particularly for large arrays that dominate channel dynamics. For

81 example, minimising environmental impacts such as sediment transport may
82 restrict array placement (Fairley et al., 2015; du Feu et al., 2019). Likewise,
83 maintaining navigation routes through clearance constraints prevents exploita-
84 tion of channel blockage, a beneficial phenomenon for larger arrays. As such,
85 array optimisation is often posed as a multi-objective problem, adding addi-
86 tional complexity (Nash et al., 2014; Culley et al., 2016; du Feu et al., 2017,
87 2019; González-Gorbeña et al., 2018; Phoenix and Nash, 2019).

88 Establishing the optimal array layout becomes computationally intensive
89 when interlinked with the hydrodynamics as it presents a partial differential
90 equation (PDE) constrained optimisation problem. Early work involved sim-
91 plified hydrodynamic models, since ‘in-concert’ tuning of tidal turbines in an
92 array would necessitate multiple runs which would require appreciable time
93 in more detailed models (Vennell, 2011, 2012). Investigations of channel-scale
94 optimisation by large numbers of 2-D simulations for different array layouts
95 and turbine tunings have been carried out, but are notably time and mem-
96 ory intensive (Divett et al., 2016). An alternative has been proposed by using
97 gradient-based optimisation that makes use of adjoint methods to efficiently
98 calculate the objective function gradient, leading to immense reductions in
99 the number of evaluations required (Funke et al., 2014, 2016). This enables
100 optimisation with a capacity to account for impacts to the hydrodynamics, at
101 a lower computational cost than techniques that estimate the gradient. The
102 same approach has been adopted for wind farms to capture non-linear tur-
103 bulent flow physics, as the adjoint method allows inclusion of higher fidelity
104 3-D computational fluid dynamics (CFD) (King et al., 2017). Nevertheless,
105 adjoint optimisation remains fairly intensive as demonstrated by examples in
106 the literature, which are largely constrained to idealised and semi-idealised
107 cases (Funke et al., 2014; Barnett et al., 2014). Similarly, the integration of 3-
108 D modelling with optimisation algorithms beyond idealised cases (as in King
109 et al. (2017)) is scarce. Recent work on discrete turbine array optimisation has
110 relied on 2-D coastal hydrodynamics models (Piggott et al., 2021), employ-
111 ing simplified turbine parameterisations whilst being constrained by either the
112 attainable model structure or resolution, as in Phoenix and Nash (2019).

113 To circumvent intense computational effort, inspiration can be taken from
114 wind energy research, where surrogate models are used to simplify the govern-
115 ing physics. These models may ignore important hydrodynamic effects such
116 as blockage that can augment power production for tidal energy (Nishino
117 and Wilden, 2012; Chen et al., 2019). For example, a “duct effect” may be
118 exploited by placing turbines in a staggered arrangement, funnelling and accel-
119 erating the flow onto downstream turbines, as shown in Funke et al. (2014).
120 Aside from certain examples restricted in idealised domains (Stansby and
121 Stallard, 2016), semi-analytical methods based on turbine wake superposition
122 principles are often constrained to a structured turbine placement (Lo Brutto
123 et al., 2016). Nevertheless, wake superposition methods have led to reason-
124 able agreement against laboratory measurements for model tidal turbines and

125 rapid optimisation within idealised low-blockage cases has predicted significant
126 increases in array efficiency (Stansby and Stallard, 2016).

127 In setting out this study, we outline our overarching goal: an array opti-
128 misation strategy that is computationally efficient and extensible to the
129 multi-objective optimisation settings sought thereafter. Additionally, it must
130 be reliable, accurate and acknowledging important hydrodynamic factors and
131 turbine characteristics that affect the optimal array design and performance.
132 This paper aims to demonstrate a novel optimisation approach, retrofitting an
133 analytical wake model designed for wind array optimisation (*FLORIS* from
134 the US National Renewable Energy Laboratory) for use in conjunction with a
135 coastal ocean model (*Thetis*). We provide details on an optimisation approach
136 which includes the option of a custom greedy algorithm for micro-siting pur-
137 poses. This is applied to a suite of representative idealised cases, progressing
138 to a practical study of the Inner Sound of the Pentland Firth, UK.

139 2 Methodology

140 We combine a depth-averaged hydrodynamic model, *Thetis*¹, with an analyti-
141 cal wake model, *FLORIS* (FLOW Redirection and Induction in Steady-state²).
142 *FLORIS* is used to perform array optimisation by importing ambient flow fields
143 from *Thetis*, returning an optimised set of turbine coordinates. Sequentially,
144 *Thetis* evaluates initial and optimised layouts, by representing the presence of
145 turbines parameterised through momentum sink terms, quantifying impacts on
146 flow field and overall array power. Both models rely on actuator disc theory to
147 represent the tidal turbine rotor. However, differences between the two models
148 necessitate the introduction of an intermediate calibration step. A schematic
149 of the combined approach is shown in Fig. 1.

150 2.1 Shallow Water Equation Modelling with *Thetis*

151 *Thetis* is a 2-D/3-D model for coastal and estuarine flows based on the general-
152 purpose finite element partial differential equation (PDE) solver *Firedrake*
153 (Rathgeber et al., 2016; Kärnä et al., 2018). It has been used for several stud-
154 ies on the feasibility and optimisation of tidal energy (Angeloudis et al., 2018;
155 Baker et al., 2020; Zhang et al., 2022; Harcourt et al., 2019). We solve the
156 non-conservative form of the nonlinear shallow-water equations in 2-D,

¹<http://thetisproject.org/>

²<https://floris.readthedocs.io/en/main/>

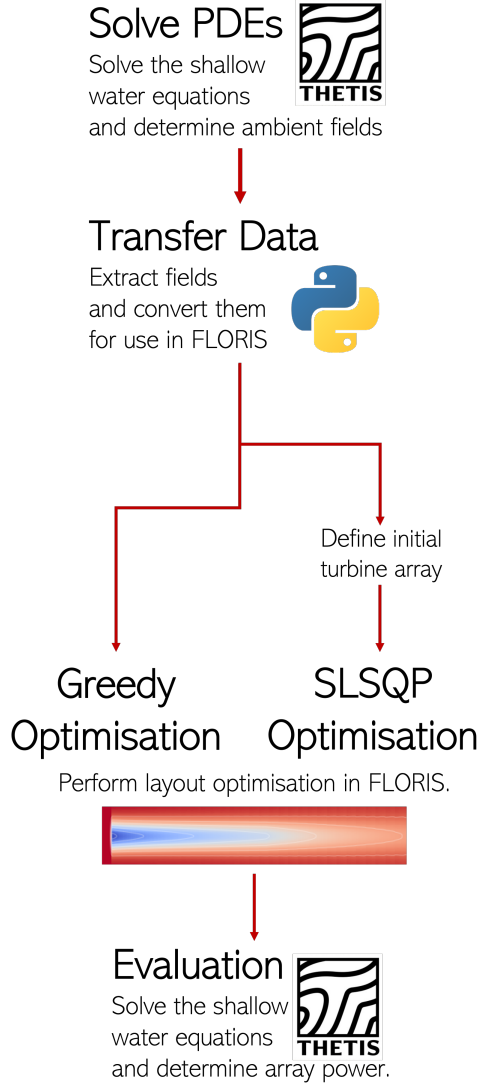


Fig. 1 Schematic representation of the model combination forming the optimisation sequence.

$$\frac{\partial \eta}{\partial t} + \nabla \cdot (H_d \mathbf{u}) = 0, \quad (1)$$

$$\frac{\partial \mathbf{u}}{\partial t} + \mathbf{u} \cdot \nabla \mathbf{u} + f \mathbf{u}^\perp + g \nabla \eta = \nabla \cdot (\nu (\nabla \mathbf{u} + \nabla \mathbf{u}^T)) - \frac{\boldsymbol{\tau}_b}{\rho H_d} - \frac{c_t}{\rho H_d} |\mathbf{u}| \mathbf{u}, \quad (2)$$

157 where η is the water elevation, H_d is the total water depth, \mathbf{u} is the depth-
 158 averaged velocity vector, and ν is the kinematic viscosity of the fluid. The term
 159 $f\mathbf{u}^\perp$ represents the Coriolis “force” included in non-idealised cases. In this
 160 term, \mathbf{u}^\perp is the velocity vector rotated counter-clockwise over 90° so that $\mathbf{u}^\perp =$
 161 $(-v, u)$, where u, v are respectively the longitudinal and transverse components
 162 of \mathbf{u} . In turn, $f = 2\Omega\sin(\zeta)$ with Ω the angular frequency of the Earth’s
 163 rotation and ζ the latitude. In idealised cases, bed shear-stress ($\boldsymbol{\tau}_b$) effects are
 164 represented through a quadratic drag formulation,

$$\frac{\boldsymbol{\tau}_b}{\rho} = C_D |\mathbf{u}| \mathbf{u}. \quad (3)$$

165 For realistic cases the Manning’s n_M formulation is adopted, given as

$$\frac{\boldsymbol{\tau}_b}{\rho} = gn_M^2 \frac{|\mathbf{u}| \mathbf{u}}{H_d^{\frac{1}{3}}}, \quad (4)$$

166 and applied as in Mackie et al. (2021a). When applicable, inter-tidal processes
 167 are treated using the wetting and drying formulation of Karna et al.
 168 (2011). The shallow-water equations are discretised using the discontinuous
 169 Galerkin finite element method (DG-FEM) and the semi-implicit Crank-
 170 Nicolson scheme is selected for time-marching the solution. The resulting
 171 discrete system of equations is solved iteratively by Newton’s method as imple-
 172 mented in PETSc (Balay et al., 2016). Finally, c_t is the parameterisation added
 173 to represent the turbines’ thrust as follows.

174 2.2 Discrete turbine representation in *Thetis*

175 Turbine rotors are represented in *Thetis* as areas of increased bed friction,
 176 adopting the linear momentum actuator disc theory (Kramer and Piggott,
 177 2016). In the 2-D depth-averaged form of the shallow-water equations, the
 178 force as a result of an array of turbines is:

$$F_{\text{array}} = \int_{\Omega_{\text{array}}} \frac{1}{2} \rho c_t(\mathbf{x}) |\mathbf{u}(\mathbf{x})| \mathbf{u}(\mathbf{x}) \, d\mathbf{x}, \quad (5)$$

179 where $c_t(\mathbf{x})$ is a thrust coefficient function given as:

$$c_t(\mathbf{x}) = C_t(|\mathbf{u}(\mathbf{x})|) A_t d(\mathbf{x}), \quad (6)$$

180 where A_t is the turbine swept area, C_t is the thrust coefficient as a function
 181 of the velocity $\mathbf{u}(\mathbf{x})$, and $d(\mathbf{x})$ is the local turbine density. The turbine density
 182 $d(\mathbf{x})$ is constructed using a vector \mathbf{m} comprising the turbine coordinates of
 183 the array. This discrete turbine representation adopts the exponential bump
 184 function of Funke et al. (2014), which in 1-D takes the form

$$\psi_{p,r}(x) \equiv \begin{cases} e^{1-1/(1-|x-p|^2/r^2)} & \text{for } \left| \frac{x-p}{r} \right| < 1, \\ 0 & \text{otherwise} \end{cases}, \quad (7)$$

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185 where r is the radius of the bump, set by default to $D/2$, where D is the diam-
 186 eter of the turbine. Equation (7) is employed in defining the turbine density
 187 d_i for a turbine i at a position $m_i = (x_i, y_i)$ as the normalised product of 1-D
 188 bump functions:

$$d_i(\mathbf{x}) = \frac{\psi_{x_i,r}(x)\psi_{y_i,r}(y)}{\Xi r^2}, \quad (8)$$

189 where $\Xi = \int_{-1}^1 \int_{-1}^1 e^{\left(\frac{-1}{1-x^2} - \frac{1}{1-y^2} + 2\right)} dx dy \approx 1.45661$ is the integral of the
 190 bump function when $r = 1$. The aggregate of the individual turbine densities
 191 d_i provides the overall $d(\mathbf{x})$ function:

$$d(\mathbf{x}) = \sum_{i=1}^N d_i(\mathbf{x}), \quad (9)$$

192 where N is the number of turbines deployed.

193 Following the notation in (5), the power extracted at any given moment
 194 by the array can be approximated as

$$P_{\text{array}} = \int_{\Omega_{\text{array}}} \frac{1}{2} \rho c_p(\mathbf{x}) |\mathbf{u}(\mathbf{x})|^3 d\mathbf{x}, \quad (10)$$

195 where $c_p(\mathbf{x})$ is a power coefficient function given as

$$c_p(\mathbf{x}) = C_p(\mathbf{u}(\mathbf{x})) A_t d(\mathbf{x}), \quad (11)$$

196 where C_p is a power coefficient which is related to the thrust coefficient through
 197 the formulation (Martin-Short et al., 2015b)

$$C_p(\mathbf{u}(\mathbf{x})) = \frac{1}{2} \left(1 + \sqrt{1 - C_t(|\mathbf{u}(\mathbf{x})|)} \right) C_t(|\mathbf{u}(\mathbf{x})|). \quad (12)$$

198 In equations (5) and (10) it is assumed that the ambient velocity is the
 199 same as the velocity through the turbine, $\mathbf{u}(\mathbf{x})$ (i.e. the velocity once the tur-
 200 bine is operating). This is a reasonable approximation for relatively coarse
 201 meshes with distributed rather than discrete turbine density fields (Schwedes
 202 et al., 2017). However, for micro-siting arrays where the thrust force is concen-
 203 trated at the turbine location, this assumption becomes invalid. In addressing
 204 this we adopt the correction for deriving a relationship between free-stream
 205 and through-turbine velocities as derived in more detail by Kramer and Pig-
 206 gott (2016). In summary, denoting as U_∞ the magnitude of the approaching
 207 streamwise velocity the turbine experiences, it can be established using the
 208 continuity, momentum and Bernoulli's principles that

$$U_\infty(\mathbf{x}) = \frac{1}{1 + \frac{1}{4} \frac{A_t}{A_t} C_t(|\mathbf{u}(\mathbf{x})|)} |\mathbf{u}(\mathbf{x})|, \quad (13)$$

209 where $\hat{A}_t = H_d D$ is the numerical cross-section of the turbine. Equation (13)
 210 stems from the classical 1-D actuator disc theory, with the correction returning

211 an approximation of the ambient velocity as per the relationship between U_∞
 212 and \mathbf{u} . It is noted that this process assumes that local blockage and shear
 213 effects are negligible (Garrett and Cummins, 2007). The corrected velocity
 214 U_∞ from (13) is applied to correct the thrust (c_t) and power (c_p) coefficient
 215 values, compensating for the velocity drop by the introduction of the turbine
 216 momentum sink over the deployed area of the turbine.

217 2.3 Analytical wake modelling using *FLORIS*

218 *FLORIS* contains analytical models to predict the mean wake velocities and
 219 power output of turbine arrays (NREL, 2020). In the present study, we
 220 apply *FLORIS*'s Gaussian model (Bastankhah and Porté-Agel, 2014) which
 221 computes the normalised velocity deficit via the expression

$$\frac{\overline{\Delta \mathbf{u}}}{U_\infty} = \left(1 - \sqrt{1 - \frac{C_T}{8(k^*x/D + \epsilon)^2}} \right) \cdot e^{\left(-\frac{1}{2(k^*x/D + \epsilon)^2} \left\{ \left(\frac{z - z_h}{D} \right)^2 + \left(\frac{y}{D} \right)^2 \right\} \right)}, \quad (14)$$

222 where z is the wall-normal coordinate with z_h the turbine hub height, k^* is the
 223 growth rate of the wake ($\partial\sigma/\partial x$), and ϵ is the normalised Gaussian velocity
 224 deficit at the rotor plane. For our calculations the local wake growth rate k^*
 225 is estimated using the local streamwise turbulence intensity, \mathcal{I} (Niayifar and
 226 Porté-Agel, 2016). We should note here that the Gaussian velocity model has
 227 been selected instead of the more traditionally used Jensen model (Jensen,
 228 1983) which is of similar computational cost but is known to overestimate
 229 the velocity deficit in the outskirts of the wake (Chamorro and Porté-Agel,
 230 2009; Dufresne and Wosnik, 2013). This is due to the Jensen model's approach
 231 of setting a uniform velocity deficit across the wake width. In turn, turbine
 232 power output is calculated using a power thrust-velocity relationship specified
 233 for each individual turbine. This requires a combination model to account for
 234 the contributing wake velocity deficit from upstream and other neighbouring
 235 turbines. We use the free-stream linear superposition (FLS) method to account
 236 for the cumulative wake effects within the tidal array. Accordingly, the velocity
 237 deficit, $\overline{\Delta \mathbf{u}}(x, y)$, at a downstream location (x, y) is calculated as,

$$\overline{\Delta \mathbf{u}}(x, y) = \sum_{i=1}^N \left(\overline{\Delta \mathbf{u}}_i|_{(x,y)} \right), \quad (15)$$

238 where $\overline{\Delta \mathbf{u}}_i|_{(x,y)}$ is the contribution the wake of each turbine i at the down-
 239 stream location (x, y) (Machefaux et al., 2015). Alternative superposition
 240 methods include summing the square of the velocity deficits (Katic et al., 1987)
 241 as well as the more recent work by Lanzilao and Meyers (2021) which takes
 242 into account the heterogeneity of the background velocity field.

2.4 Optimisation approach

We seek to maximise energy from our tidal array system. In doing so, the existing layout optimisation procedure in *FLORIS* (Fleming et al., 2016) is repurposed to maximise the average power computed using several input flow fields, rather than the average annual energy production from a single wind rose. The latter is typical of wind farm optimisation and would not apply to tidal-array optimisation. To this end, we approach the tidal-array micro-siting problem by employing an initial *Thetis* simulation of the tidal channel and extract ambient velocity fields for a number of instances, or ‘frames’, over a tidal cycle. This ambient flow field data is then imported into *FLORIS*. If necessary, an initial (e.g. aligned/staggered) turbine layout is introduced to *FLORIS* and micro-siting is performed using an appropriate optimisation strategy subject to spatial constraints, and minimum turbine separation restrictions. As such, the objective function can be expressed as

$$\max_{\mathbf{m}} \quad \frac{1}{N_F} \sum_{j=1}^{N_F} P_{\text{array}}(\mathbf{u}, \mathbf{m}) \quad (16)$$

$$\text{subject to } \tau_l \leq \tau_i \leq \tau_u$$

where N_F is the number of flow field frames considered and \mathbf{m}, τ_i are vectors including turbine coordinates and optimisation constraints (e.g. minimum distance between turbines, array deployment area limits) respectively. τ_l, τ_u correspond to the lower and upper limits for each of these constraints.

Upon optimisation in *FLORIS*, a derived turbine layout \mathbf{m} is evaluated in *Thetis* to assess its performance. A similar approach to optimisation has previously been undertaken to determine wind plant control strategy, with the objective of optimising yaw settings to minimise wake interaction and increase overall farm power (Gebraad et al., 2014). In a deviation from the study of Gebraad et al. (2014) which pioneered the blending of a CFD flow solver with *FLORIS*, we present herein a first attempt to combine *FLORIS* with a shallow-water solver for tidal applications.

As we aim to demonstrate a proof-of-concept for the optimisation approach, investigations on specific optimisation algorithms are beyond the scope of this work. *FLORIS*’s default optimisation is initially performed using the SciPy minimise function for the idealised models (see Section 4), through the SLSQP (Sequential Least Squares Programming) method (Kraft, 1988). The number of iterations for each SLSQP minimisation problem was limited to the default value of 50. Altering $2N$ variables (i.e. x -, y - coordinates for N turbines) for each flow field over 50 iterations becomes highly time-consuming as the number of turbines N increases beyond a small array. An increased array size also entails a larger optimisation space, further stressing conventional optimisation, increasing the likelihood of converging to local maxima. To address the above, a heuristic-based greedy optimisation technique is tested which positions each turbine sequentially. This allows the imposition of constraints which form acceptance criteria, sequentially adding turbines until either desired capacity

283 is installed or no feasible positions remain. This alternative approach allows
 284 for the rejection of proposed turbine placements based on aspects such as
 285 bathymetric gradient, forming a basis for non-trivial objective functions. The
 286 simplified sequence is described in Algorithm 1.

Algorithm 1 Sequential addition of turbines to domain using greedy optimisation.

INPUTS: Number of turbines to be positioned N , maximum number of optimisation iterations and ambient flow fields.

CONDITIONS (A, B, Γ): Each turbine must meet a minimum A -% average turbine capacity factor, have maximum reduction of power to any other turbine of B -% and maximum reduction of power to the sum of individual turbines (that face power output reductions) of Γ -%.

CONSTRAINTS (Δ, E, Z): Minimum distance constraints for turbine placement, specified in turbine diameters away from considered coordinate (Δ, E) and turbine deployment area bounds (Z).

- 1: Select ambient flow fields from hydrodynamic simulations performed in *Thetis*.
 - 2: **while** (iteration no. < maximum no. of iterations) **and** (no. of turbines < maximum no. of turbines) **do**
 - 3: Calculate and superimpose turbine wakes to each flow field of selected tidal cycles.
 - 4: Calculate a field of moving average flow magnitude (a moving average deters turbine placement on wake edges).
 - 5: Identify as a candidate turbine location the unrestricted coordinate $\in Z$ of maximum average velocity magnitude.
 - 6: Add turbine at candidate site and superimpose wake on each flow field.
 - 7: Calculate the average power (using each individual field) for all turbines including the new turbine.
 - 8: **if** *CONDITIONS* are met **then**
 - 9: Add candidate site to list of accepted coordinates.
 - 10: Impose a restriction for turbine placement within a limiting distance Δ around new coordinate.
 - 11: **else**
 - 12: Add candidate site to list of blocked coordinates.
 - 13: Impose a restriction for turbine placement within a limiting distance of E around blocked coordinate.
 - 14: **end if**
 - 15: **end while**
-

3 Turbine specifications and analytical wake calibration

Before considering the optimisation case studies it is instructive to outline the turbine specifications and calibration process in configuring the analytical wake model parameters of *FLORIS* so that shallow-water wakes are adequately represented. For the tidal turbines, consistent specifications summarised in Table 1 are applied across all case studies. Turbine dimensions, cut-in and rated speeds (u_{rated}) are based on known parameters for the SIMEC Atlantis 2 MW AR2000 turbine (SIMEC Atlantis Energy, 2016, 2020b). Combining equations (10), (11) and (12) allows the determination of the thrust coefficient at u_{rated} ($C_{t,\text{rated}}$), considering the reported AR2000 turbine size (20 m) and its reported power output (2 MW). Given these specifications $C_{t,\text{rated}} = 0.516$, noting that this is lower than the value of $C_t = 0.8$ determined in lab-scale experiments (Bahaj et al., 2007; Stallard et al., 2015). Fig. 2 shows the theoretical tailing of the thrust coefficient for higher velocities. This has been approximated by Cardano’s formula (Wituła and Słota, 2010) to produce a simpler equation preventing the need for third-order polynomial inversion that is otherwise required to calculate C_p throughout the hydrodynamic simulation. Below the cut-in speed, C_t is ramped up exponentially to avoid discontinuities which may cause instabilities within the hydrodynamic model without affecting the total power produced. For consistency, C_t and C_p are applied uniformly for both *FLORIS* and *Thetis*, with the resultant power curve of Fig. 2.

Table 1 Common input parameters.

Fluid density, ρ	1025 kg/m ³
Rotor swept diameter, D	20 m
Hub height, z_{hub}	18 m
Turbine cut-in speed, u_{in}	1 m/s
Turbine rated speed, u_{rated}	3.05 m/s

3.1 FLORIS-specific inputs

Table 2 *FLORIS* input parameters.

Flow shear power law exponent	0
Flow veer	0
Axial induction factor (α) exponent	0.8325
Normalised downstream distance (x/d) exponent	-0.32
Initial turbulence intensity, \mathcal{I}_0	12%
Ambient turbulence intensity, \mathcal{I}	20%

As we apply *FLORIS* in the tidal-energy “domain”, *FLORIS*-specific parameters are altered to appropriate values for a tidal setting (Table 2). The

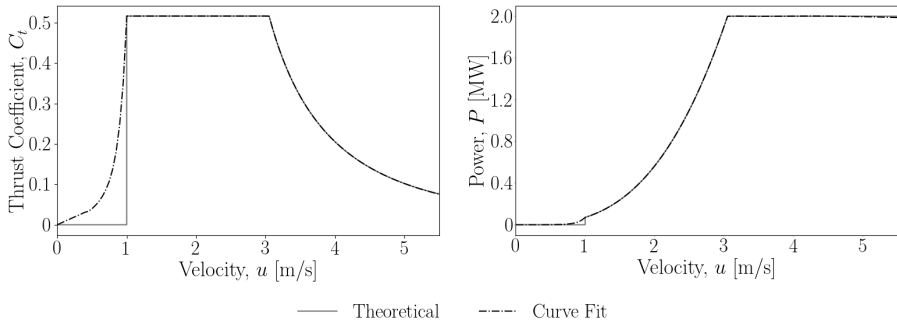


Fig. 2 Left: thrust coefficient function combining the tailing approximated by Cardano’s formula relative to theoretical thrust coefficients for a 2 MW turbine. Right: corresponding power curves.

312 flow shear power law exponent and veer which describe the change in vertical
 313 velocity and direction, respectively, are both set to 0, omitting vertical vari-
 314 ability for consistency with *Thetis*. The turbulence model selected in *FLORIS*
 315 is documented by Crespo and Hernández (1996) and default coefficients in cal-
 316 culating the added streamwise turbulence intensity, \mathcal{I}_+ , are used. Accordingly,
 317 inputs for the axial induction factor exponent and the normalised downstream
 318 distance (x/d) exponent are set to the empirically determined values of 0.832
 319 and -0.32 respectively. The initial turbulence intensity at the turbines, \mathcal{I}_0 ,
 320 has been determined experimentally at smaller scales to be 12% at the rotor
 321 plane for three-blade model tidal turbines (Stallard et al., 2015). Hub height
 322 streamwise turbulence intensity has been determined from ADCP deployments
 323 upstream of the Meygen Phase 1A turbines to be approximately 10% and
 324 12% for peak flood and ebb flows respectively (Coles et al., 2018). Measured
 325 data in the Inner Sound of the Pentland Firth suggests the ambient turbu-
 326 lence intensities at peak flow speeds are 13% and 17% during flood and ebb
 327 tides, increasing linearly as the flow speed reduces (Hardwick et al., 2015). As
 328 the turbulence intensity is assumed uniform for simplicity, the initial ambient
 329 turbulence intensity is estimated to be 20%, as flow speeds (for optimisation
 330 purposes) will typically range from ≈ 2 –5.5 m/s.

331 3.2 Calibration of FLORIS wake effects

332 Wake-specific parameters are calibrated to replicate the depth-averaged veloc-
 333 ity deficits exhibited by *Thetis* to render the evaluation of the 3-D *FLORIS*-
 334 based optimal array designs in *Thetis* meaningful. Through the representation
 335 of turbines by momentum sinks (Section 2.2), *Thetis* acknowledges essen-
 336 tial hydrodynamic interactions in the assessment of tidal stream arrays (e.g.
 337 turbine wake evolution, array blockage). Importantly, this is done within
 338 coastal ocean models that acknowledge complex morphologies as well as far-
 339 field forcings that drive the oscillatory flow over tidal array development
 340 areas. As *FLORIS* does not consider flow interaction processes via the wake-
 341 superposition approach, its use to optimise arrays in *Thetis* can also be seen

342 as a test for its potential application when linked with more computationally
 343 intensive models and real-world scenarios.

344 Parameters calibrated herein include k_a and k_b , which specifically relate to
 345 turbulence intensity and wake width. These combine and determine the value
 346 of the wake growth rate, k^* , which eventually enters the Gaussian velocity
 347 deficit equation (14) calculated as,

$$k^* = k_a \cdot \mathcal{I} + k_b. \quad (17)$$

348 The second set of parameters α and β are used for the quantity, x_0 , which
 349 defines the onset of the far wake,

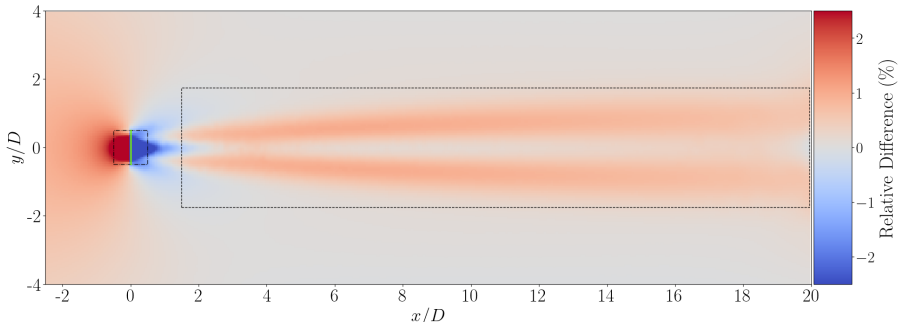
$$x_0 = D \frac{1 + \sqrt{1 - C_t}}{\sqrt{2} \left(4\alpha \cdot \mathcal{I} + 2\beta (1 - \sqrt{1 - C_t}) \right)}. \quad (18)$$

350 Calibration is performed using differential evolution (as implemented within
 351 SciPy's optimisation library (Virtanen et al., 2020)) to optimise the wake
 352 parameters k_a , k_b , α and β such that the r.m.s. error between wakes in *Thetis*
 353 and *FLORIS* is minimised. It should be noted that the velocity deficit mag-
 354 nitudes in *FLORIS* are averaged over regular depth increments to produce an
 355 equivalent depth-averaged *FLORIS* wake, used to optimise model parameters.
 356 Calibration is performed for u_{rated} only, and then compared to results from
 357 calibration exercises for speeds below and above u_{rated} to gauge the extent of
 358 deviations. An idealised model consisting of a single turbine in the channel
 359 described in Section 4.1 is used to create a velocity deficit to be investigated
 360 over 20 diameters downstream for this purpose.

361 Wake calibration results are shown in Table 3, with the r.m.s. error between
 362 *Thetis* and *FLORIS* fields below 0.6% in the area of interest from 1.5–20*D*
 363 downstream. The difference in turbine representation is presented in Fig. 3,
 364 clearly showing higher values of the *FLORIS* flow field velocity compared to
 365 *Thetis* as the velocity reduces over the bump function that represents the
 366 presence of the device. Immediately downstream of the *FLORIS* turbine, the
 367 velocity is lower than in *Thetis* due to the greater deficit imposed by *FLORIS*,
 368 which comes as a result of the discontinuous superposition of the analytical
 369 wake model at the turbine location. This discrepancy in turbine representation
 370 leads to the decision to calibrate based on the flow field from 1.5*D* downstream
 371 in a zone of width 3*D* to also capture the expansion width of the far wake.
 372 It should be noted that this is typically the region of highest error between
 373 not only differing turbine representation methods, but also to measured data;
 374 existing research has already demonstrated that accurately capturing the wake
 375 dynamics may require investigation of several different approaches to turbine
 376 modelling (Sandoval et al., 2021). The central region of the wake is well cal-
 377 ibrated, with increased r.m.s. error bands on the edges of the wake, though
 378 within margins of 1%. At u_{rated} , this representation is considered acceptable,
 379 with a 1% velocity variation on the outer wake unlikely to impact optimisation,
 380 considering the assumptions within these parameterisations.

Table 3 Calibrated wake parameters for Gaussian model.

k_a	0.1087
k_b	0.006912
α	0.4886
β	0.2496

**Fig. 3** Relative difference between *Thetis* and *FLORIS* flow fields with positive magnitude (red) representing a higher *FLORIS* estimation of velocity. The area indicated by a black square on the left shows *Thetis* area of increased friction whilst the solid green line shows the *FLORIS* turbine representation. Black box on the right defines area over which r.m.s. error is calculated for calibration at u_{rated} alone, see Table 4.

381 A comparative analysis of the wake parameters for u_{rated} against calibrations
 382 at varying flow speeds (Table 4) demonstrates that the overall r.m.s. error
 383 is still acceptable as the analytical wake model is applied within its expected
 384 range. With decreasing velocity, the wake width increases and as the velocity
 385 approaches cut-in speed the immediate wake width begins to exceed the
 386 turbine diameter, increasing the r.m.s. error, albeit within acceptable levels.

Table 4 Comparison of the r.m.s. error between *Thetis* and *FLORIS* flow fields for calibration at rated speed alone vs. direct calibration at the velocity specified.

Velocity, u (m/s)	r.m.s. error (%)	
	Rated Speed Calibration	Direct Velocity Calibration
1.5	1.243	0.379
2.5	0.756	0.325
3.25	0.575	-
4.5	0.130	0.099

387 For completeness we present results of a separate calibration performed
 388 between the analytical wake model and flume data (Stallard et al., 2013) capturing
 389 3-D wake turbulence dynamics that are not present within 2-D depth-
 390 averaged models. A comparison between the different wake behaviour and the
 391 respective *FLORIS* calibrated solutions are shown in Fig. 4. Specifically, Fig.
 392 4 shows

- 393 • *Thetis* vs Thetis-calibrated *FLORIS* depth-averaged wake profiles;

394 • Stallard et al. (2013) data vs the corresponding calibrated *FLORIS* predic-
 395 tion for an isolated turbine at hub-height z_{hub} .

396 Froude-scaling has been applied for comparison against laboratory data (Stal-
 397 lard et al., 2013). Calibration to this data shows excellent agreement beyond
 398 $\approx 3D$ – $3.5D$ and therefore the potential to calibrate to 3-D data.

399 Even here however, the analytical representation of the near wake could be
 400 improved. This further highlights the challenge of calibration between *Thetis*
 401 and *FLORIS* as even on the depth-averaged profile, the immediate deficit
 402 downstream of the turbine is substantially greater relative to *Thetis*. Neverthe-
 403 less, the *Thetis* calibrated wake has been well-calibrated beyond $1.5D$; since
 404 turbines are unlikely to be placed in such close proximities in the streamwise
 405 direction, this is unlikely to impact optimisation. For real world applications
 406 the initial wake calibration step should be conducted against the best possible
 407 wake data available, from observations and/or high-resolution CFD.

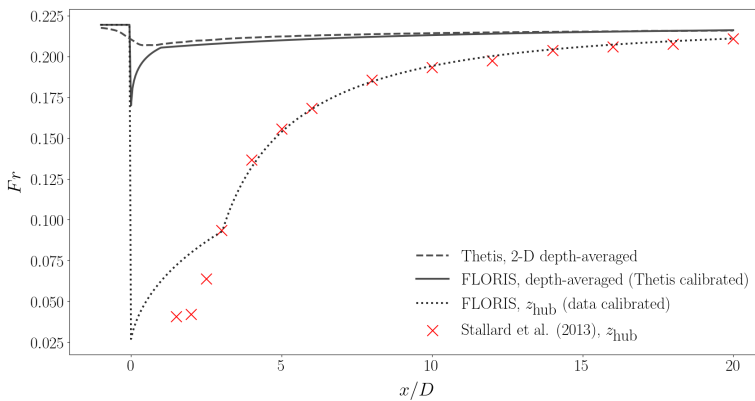


Fig. 4 Longitudinal profile for wake Froude number $Fr = \frac{|\mathbf{u}|}{\sqrt{gH_d}}$, both depth-averaged for comparison between calibrated *FLORIS* vs *Thetis* and at hub height (z_{hub}) for comparison between calibrated *FLORIS* vs experimental data of Stallard et al. (2013).

408 4 Case Studies

409 In demonstrating this tidal-array optimisation framework, we consider models
 410 of increasing complexity and denser turbine placement. First, we examine the
 411 micro-siting of aligned and staggered 2×4 turbine arrays of three rotor diam-
 412 eter ($3D$) spacing between rows and columns; the array itself is situated within
 413 an idealised channel with and without a headland. These exercises are then
 414 repeated with denser 3×5 turbine arrays of $2D$ lateral (between rows) spacing
 415 and $3D$ longitudinal (between columns) spacing. The idealised cases imitate
 416 two examples from Funke et al. (2014) and serve in validating the performance
 417 of the current approach prior to assessing a more realistic full-scale optimisa-
 418 tion problem. For our realistic flow problem we consider the Pentland Firth

419 region with aligned and staggered array sizes of 4×6 turbines of $5D$ lateral
 420 and longitudinal spacing, followed by a staggered 8×6 case of $3D$ spacing.

421 The cases are illustrated in Figures 5 and 6 respectively, including the
 422 computational meshes used by *Thetis* for the hydrodynamic simulations. In all
 423 cases, the mesh generation process employs the open-source code *qmesh* (Avdis
 424 et al., 2018), featuring a 3 m element resolution for the idealised cases, and 5 m
 425 for the Pentland Firth case within the allocated tidal array. This element size
 426 was selected using a mesh sensitivity study on the wake resolution confirming
 427 the mesh resolution independence of the results within the array.

428 The optimisation approach is informed by several spatial conditions/ constraints.
 429 The “greedy” optimisation approach features an initial minimum of
 430 three diameters ($3D$) distance separating each turbine and a blocked radius
 431 of one diameter ($1D$) for each “failed iteration” ($\Delta = 3D$, $E = 1D$). Initially,
 432 the $3D$ separation constraint between turbines is applied for all optimisation
 433 techniques to prevent high-magnitude flow deficits impacting closely spaced
 434 turbines (the spacing is typically $1.5D$ – $5D$ for tidal turbines (Stallard et al.,
 435 2013)). However, towards making better use of the deployment area, the spa-
 436 tial constraint is then reduced to $1.5D$ to maximise the number of turbines
 437 within the domain. The conditions for $3D$ spacing are specified as a minimum
 438 17.5% capacity factor per turbine, a 5% maximum reduction of power for indi-
 439 vidual turbines and a 9% maximum reduction in the cumulative power of the
 440 particular turbines subject to a power reduction ($A = 17.5\%$, $B = 5\%$, $\Gamma =$
 441 9%) as required. As turbine interactions are inevitable for $1.5D$ spacing, con-
 442 straint limits need to be less stringent with $A = 17.5\%$, $B = 15\%$, $\Gamma = 25\%$.
 443 Specific details for each case study are expanded below.

444 4.1 Steady-state flow through an idealised channel

445 An idealised channel of dimension (640 m \times 320 m) featuring a (320 m \times
 446 160 m) region where a tidal array is to be deployed, provides sufficient space
 447 to tightly pack turbines across the width of the channel, but is short enough
 448 to prevent substantial wake recovery. The bathymetry is constant across the
 449 full domain at 50 m depth. Eddy viscosity is set to a constant value of $1 \text{ m}^2/\text{s}$
 450 across the domain away from the boundaries as per previous studies (Vouriot
 451 et al., 2019). For simplicity, a quadratic drag coefficient $C_D = 0.0025$ (which
 452 represents a fairly smooth bed) is selected, following previous investigations
 453 of the Pentland Firth, e.g. (Draper et al., 2014). For this steady case, the
 454 imposed flow is constant and can be represented by a single flow field, which
 455 was determined in *Thetis* with an inflow horizontal velocity, $u = 3.175 \text{ m s}^{-1}$
 456 (close to u_{rated}), and a constant elevation of 0 m at the outflow.

457 4.2 Transient flow around an idealised headland channel

458 Headlands and islands are key in providing highly energetic channels that
 459 make tidal streaming a feasible prospect. A simple headland model provides
 460 a location of concentrated higher energy density for turbines to be placed.

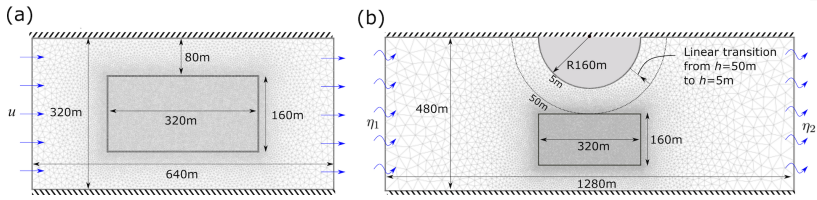


Fig. 5 a) Idealised rectangular channel; b) Idealised headland channel indicating bathymetric changes in the proximity of the headland.

461 In this case, the overall channel width and length are increased to 480 m ×
 462 1280 m for the headland (represented by a 160 m radius semi-circle) to be
 463 introduced (Fig. 5). Velocity becomes greater due to flow conservation at the
 464 constriction from 480 m to 320 m, which acts in a similar manner as a Venturi
 465 flume, accelerating the flow. A bathymetric gradient is applied radially, with
 466 the depth reduced gradually from 50 m to 5 m along the headland, imitating
 467 a shore. A viscosity ‘sponge’ is introduced at the open boundaries of 50 m²/s
 468 linearly transitioning to 1 m²/s within a distance of 10% of the channel length.
 469 Simple harmonic signals are defined (Eq. 19, Eq. 20) to drive the oscillatory
 470 flow to verify *FLORIS*’s capability to optimise for multiple fields of data. The
 471 following equations

$$\eta_1 = A_{\text{tide}} \cdot \sin\left(\frac{2\pi t}{T}\right), \quad (19)$$

$$\eta_2 = -A_{\text{tide}} \cdot \sin\left(\frac{2\pi t}{T}\right), \quad (20)$$

472 provide the assigned local elevation η_1, η_2 , at each of the boundaries and are
 473 signals of equal magnitude and opposite direction. Here, A_{tide} is the tidal
 474 amplitude, t is the simulation time and T is the tidal period. Values of $T = 1$ h,
 475 and $A_{\text{tide}} = 0.275$ m, deliver a velocity profile with a peak magnitude close to
 476 u_{rated} (i.e. 2.5–3 m/s). Following a spin-up time of 1.5 hours, fields exported
 477 for optimisation are between the cut-in and maximum speeds, over a single
 478 tidal cycle.

479 4.3 Application to the Pentland Firth, Scotland, UK

480 The Orkney archipelagos in north Scotland, UK (Fig. 6) features sites char-
 481 acterised by high tidal energy levels. This is especially pronounced in the area
 482 of Pentland Firth, a strait separating mainland Scotland from the Orkney
 483 Islands. There, flow speeds regularly exceed 5 m s⁻¹ (Draper et al., 2014) and
 484 thus the Inner Sound of the Pentland Firth is a prime site for tidal array
 485 deployment as discussed by several studies investigating the energy resource
 486 (Adcock et al., 2013; Draper et al., 2014; O’Hara Murray and Gallego, 2017),
 487 potential environmental impacts (Martin-Short et al., 2015a) as well as the
 488 micro-siting of turbines within arrays (Funke et al., 2014). At that location is
 489 the Meygen site, where a subset of a larger array has already been deployed.

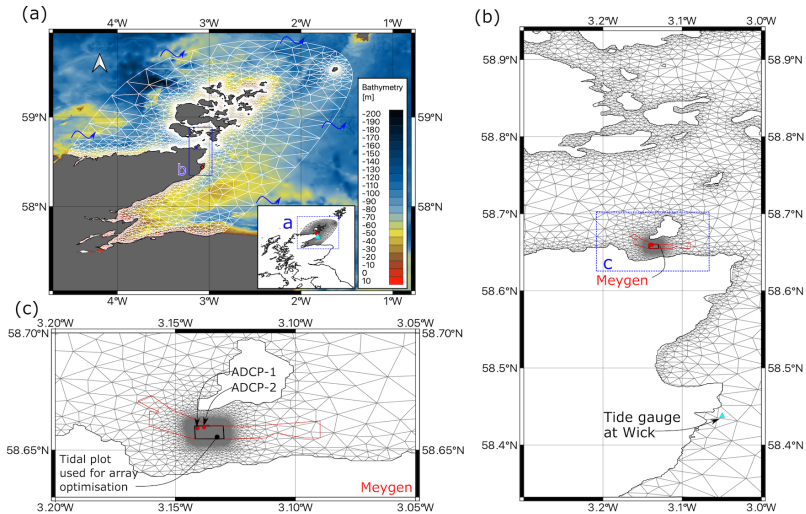


Fig. 6 Computational domain for the Pentland Firth case study. a) Domain extents and Marine Digimap bathymetry dataset (Edina Digimap Service, 2020) interpolated to elements; b) close-up to island scale; c) close-up to channel scale. The tide gauge and ADCP locations used to calibrate the model are also indicated, alongside the tidal array deployment zone considered for array optimisation.

490 The regional hydrodynamic model shown in Fig. 6a makes use of one arc-
 491 second resolution bathymetry, acquired from Edina Digimap Service (Edina
 492 Digimap Service, 2020). Open boundaries are tidally forced using eight tidal
 493 constituents (Q1, O1, P1, K1, N2, M2, S2, K2) derived from TPXO (Egbert
 494 and Erofeeva, 2002). The model, subjected to 2 days of spin-up time, hindcasts
 495 32 days from 01/08/2017 to 01/09/2017. This timeframe is selected accord-
 496 ing to the availability of ADCP data (Coles et al., 2018), spanning sufficient
 497 duration to resolve the principal constituents driving the flow (i.e. M2 and
 498 S2). Over this period, predictions are simultaneously compared against UK
 499 Hydrographic Office data recorded at a tide gauge located at Wick (Table 5).

Table 5 Comparison between observed and predicted values of principal tide constituents M2 and S2 at Wick tide gauge and ADCP locations.

Location	Constituent	Amplitude α (m)		Phase ϕ ($^{\circ}$)	
		Observed	Predicted	Observed	Predicted
Wick	M2	1.02	1.03	322.30	322.45
	S2	0.35	0.37	0.30	359.56
ADCP-1	M2	2.59	2.86	239.90	236.94
	S2	1.02	1.12	278.22	293.60
ADCP-2	M2	2.66	2.66	235.90	237.07
	S2	0.92	0.97	297.32	300.47

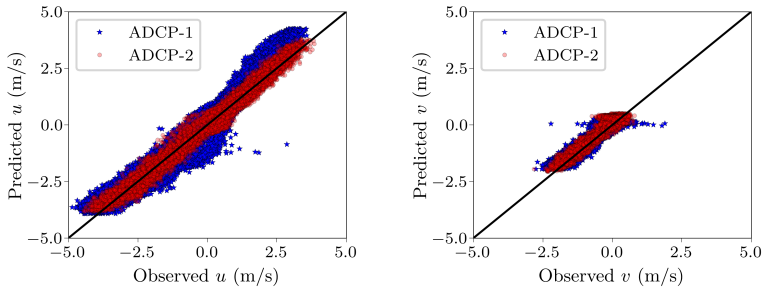


Fig. 7 Left: correlation between observed and predicted u -velocity at Pentland Firth monitoring station ($R^2 = 0.93$ and $R^2 = 0.98$ for ADCP-1 and ADCP-2, respectively). Right: correlation between the observed and predicted v -velocity ($R^2 = 0.95$ and $R^2 = 0.84$ for ADCP-1 and ADCP-2, respectively). ADCP data provided by SIMEC Atlantis Energy with further details in [Coles et al. \(2018\)](#).

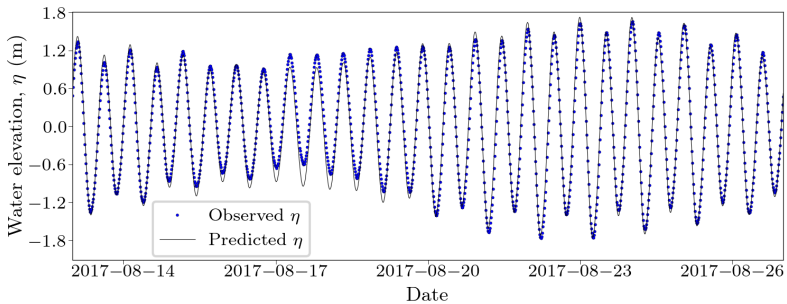


Fig. 8 Water elevation at the Wick tide gauge from 13/08/2017 till 27/08/2017. *Thetis* predictions (continuous line) are compared against observed water elevation obtained (circles) with $R^2 = 0.982$ and r.m.s. error = 0.11 m.

500 The Pentland Firth and Orkney Isles model for our optimisation study
 501 has an element size (Δh) ranging between 300–1,500 m near-shore subject
 502 to proximity to the Meygen tidal site or certain island features. This resolu-
 503 tion gradually increases to 20,000 m towards the open seaward boundaries.
 504 Increased refinement of a uniform element size $\Delta h = 5$ m has been imposed to
 505 resolve individual turbines within the Meygen tidal site. The simulation results
 506 are produced using a variable Manning’s n_M across the domain based on bed
 507 classification data provided by the British Geological Survey, as described by
 508 [Mackie et al. \(2021a\)](#), and a timestep $\Delta t = 100$ s.

Table 6 Results of optimisation methodology applied to each case. The average power P calculated based on testing layouts in *Thetis*.

ID	Case	No. of Turbines N_t	Initial Layout	Optimisation Technique	Minimum Spacing L_{\min}	No. of Frames	Average Power, $ P $ (MW)	Optimisation Time, t (minutes)
A.1	Channel ⁴	8	Aligned	-	3D	-	13.54	-
A.2	Channel	8	Staggered	-	3D	-	15.85 (+17.0%)	-
A.3	Channel	8	Aligned	FLORIS (SLSQP)	3D	1	15.99 (+18.1%)	0.5
A.4	Channel	8	-	FLORIS (Greedy)	3D	1	15.98 (+18.0%)	0.2
A.5	Channel	8	Aligned	Adjoint (SLSQP)	3D	N/A	16.00 (+18.2%)	53.8
A.6	Channel	15	Aligned	-	2D	-	25.00	-
A.7	Channel	15	Staggered	-	2D	-	28.97 (+15.9%)	-
A.8	Channel	15	Aligned	FLORIS (SLSQP)	1.5D	1	29.81 (+19.2%)	4.1
A.9	Channel	15	-	FLORIS (Greedy)	1.5D	1	29.22 (+16.9%)	3.3
A.10	Channel	15	Aligned	Adjoint (SLSQP)	1.5D	N/A	29.96 (+19.8%)	197.1
B.1	Headland ⁵	8	Aligned	-	3D	-	4.35	-
B.2	Headland	8	Staggered	-	3D	-	5.44 (+25.2%)	-
B.3	Headland	8	Aligned	FLORIS (SLSQP)	3D	6	5.61 (+29.0%)	196.4
B.4	Headland	8	-	FLORIS (Greedy)	3D	6	5.53 (+27.4%)	0.3
B.5	Headland	15	Aligned	-	2D	-	7.82	-
B.6	Headland	15	Staggered	-	2D	-	8.83 (+12.8%)	-
B.7	Headland	15	Aligned	FLORIS (SLSQP)	1.5D	6	10.06 (+28.6%)	361.9
B.8	Headland	15	-	FLORIS (Greedy)	1.5D	6	9.56 (+22.3%)	7.8
C.1	Pentland Firth ⁶	24	Aligned	-	5D	-	20.47	-
C.2	Pentland Firth	24	Staggered	-	5D	-	20.55 (+0.4%)	-
C.3	Pentland Firth	24	Aligned	FLORIS (SLSQP)	3D	18	21.72 (+6.1%)	4002.0
C.4	Pentland Firth	24	-	FLORIS (Greedy)	3D	18	23.01 (+12.4%)	0.8
C.5	Pentland Firth	24	-	FLORIS (Greedy)	1.5D	18	23.71 (+15.8%)	2.8
C.6	Pentland Firth	48	Staggered	-	3.0D	-	40.33	-
C.7	Pentland Firth	48	-	FLORIS (Greedy)	1.5D	18	42.43 (+5.2%)	12.1

³All optimisation simulations were run on a single core. Specification: Intel (R) Xeon (R) Gold 5118 CPU 2.30GHz.⁴Thetis run for 39.3 min of time on 1 core for confirmed convergence to steady state (3.5 hour simulation time).⁵Thetis run time of 27.4 min on 4 cores, for 8 hours simulation time.⁶Thetis run time of 1337.1 min on 12 cores, for a 30 day simulation time.

509 Model calibration is more sensitive against measured velocity rather than
 510 elevation data. Velocity comparisons were established against observed data at
 511 the locations of ADCP-1 and ADCP-2 (Fig. 6). In Fig. 7 a misalignment can be
 512 observed in ADCP-1 during flood tide. This is attributed to several modelling
 513 decisions, such as the coarse model resolution surrounding the Meygen site
 514 and the rest of the computational domain. In addition, the relatively low res-
 515 olution of the available bathymetric dataset used in the vicinity is influential.
 516 Nevertheless, the overall model accuracy is deemed appropriate for demon-
 517 strating the optimisation method within a practical scenario, acknowledging
 518 that these deviations between observational and model data would render fur-
 519 ther field observation and analysis essential to characterise the local dynamics
 520 more accurately. In terms of water elevation predictions, results agree well (Fig.
 521 8) with observed values at Wick. There, correlation among observed and pre-
 522 dicted water elevation data is approximately 0.982, while the root-mean-square
 523 (r.m.s.) error is equal to 0.11 m.

524 5 Optimisation Results

525 5.1 Steady-state flow through an idealised channel

526 The maximum power possible for setups of $N_t = 8$ and $N_t = 15$ turbines
 527 would be 16 MW and 30MW respectively, given that the inflow (3.175 m s^{-1})
 528 exceeds $u_{\text{rated}} = 3.05 \text{ m/s}$. Indicatively, an aligned turbine placement (Fig. 9a)
 529 yields power of $> 15\%$ below the maximum extractable power for both 8 and
 530 15 turbine setups. Placing the turbines in the staggered arrangement of Fig. 9a
 531 leads to improved power output as anticipated, slightly below the maximum
 532 achievable.

533 Layout optimisation in *FLORIS* using SLSQP leads to a distribution of
 534 turbines across the channel width. This is shown in Fig. 9b for $N_t = 15$ setups
 535 (A.8-10), forming two rows of turbines separated by a sufficient longitudinal
 536 distance that allows velocity deficit recovery from upstream devices. Using
 537 the greedy approach provides a similar result in both cases, with turbines
 538 positioned to avoid wake interaction where possible. The SLSQP approach
 539 performed best in completing optimisation due to the simplicity of the input,
 540 whilst the greedy algorithm demonstrates sensitivity to the naive initial turbine
 541 placement. This is particularly notable on the $N_t = 15$ setup (A.9), whereby
 542 two columns of turbines are required and therefore poor initial placement could
 543 negatively impact array power to a much greater extent.

544 A *Thetis* adjoint-based tidal farm optimisation (Funke et al., 2014) (A.10)
 545 provides similar distributions of turbines across the channel as in Fig. 9,
 546 but with placement that appears to exploit the “duct effect”. Adjoint opti-
 547 misation results in maximum power obtained across all approaches (as per
 548 *Thetis* simulations) since optimisation avoids inconsistencies in turbine rep-
 549 resentation, whilst also capitalising on beneficial hydrodynamic effects. The
 550 adjoint/gradient-based method was anticipated to be more effective in this
 551 case for the above reasons, particularly around (or below) u_{rated} , whereby the

552 velocity deficits can reduce the power produced. This is emphasised for the
 553 denser 15-turbine layout, where consideration of more devices places greater
 554 stress on the optimisation technique, while blockage and funnelling provide
 555 opportunities for greater power augmentation. Nevertheless, despite different
 556 layouts, the power performance is near identical among 8-turbine cases (A.3-
 557 5) and very similar between *FLORIS*'s SLSQP (A.8) and the adjoint (A.10)
 558 for the 15-turbine cases. This suggests that multiple solutions achieve the cri-
 559 terion of maximising power output, but with the adjoint taking significantly
 560 longer than either *FLORIS* approach.

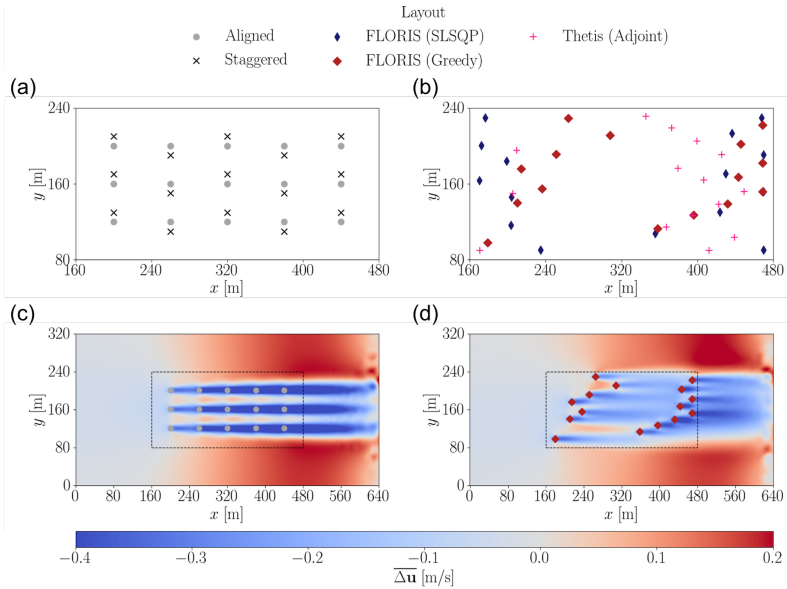


Fig. 9 Array layouts ($N_t=15$) and *Thetis*-predicted velocity deficits ($\overline{\Delta u}$) for rectangular steady-state idealised channel flow. a) standard (i.e. unoptimised) layouts (A.6-A.7, Table 6); b) optimised layouts (A.8-A.10); c) aligned layout (A.6) $\overline{\Delta u}$; d) greedy optimisation layout (A.9) $\overline{\Delta u}$.

561 5.2 Transient flow around an idealised headland

562 The idealised headland case considers oscillatory flow to demonstrate optimi-
 563 sation features over unsteady conditions. Three flow fields from each flood and
 564 ebb tide are exported to be used within *FLORIS*. For each of these sets, one
 565 is at peak velocity magnitude and two between cut-in and rated speeds. As
 566 flow direction and magnitude does not vary significantly, the total of six flow
 567 ‘frames’ performs sufficiently for optimisation in this case, with more frames
 568 delivering negligible benefit. Velocity contours for the peak flood flow with-
 569 out turbines are shown in Fig. 10 with layouts of different headland cases for
 570 the $N_t = 15$ configurations (B.5, B.6, and B.7) of Table 6 superimposed. As

571 the flow develops around the headland, the combination of the *vena contracta*
 572 effect and the bathymetric gradient contribute to a velocity acceleration that
 573 diminishes away from the headland constriction. This provides radial bands of
 574 higher energy potential for turbine placement, with only the regions closest to
 575 the headland allowing maximum power production at peak flow speeds.

576 *FLORIS*'s SLSQP based optimisation leads to placement of three turbines
 577 within these first two bands (i.e. in flow greater than 3 m s^{-1}) for the $N_t = 8$
 578 setup (B.3), with the remainder of turbines spread across the width of the
 579 channel avoiding wake interaction in a similar manner to the steady-state
 580 idealised channel case. Meanwhile, greedy optimisation places the first turbine
 581 in the centre of the first band, subsequently leading to lower power production
 582 for the surrounding turbines, which can not be placed as closely within the
 583 first two bands due to the separation constraint. A similar trend is observed
 584 with 15 turbines and a reduced separation constraint of $1.5D$; five turbines
 585 are placed within the first two bands by SLSQP and only two by the greedy
 586 algorithm (Fig. 10).

587 The average power produced by the greedy optimisation array after only
 588 20 iterations (for the $N_t = 8$ setup of B.4) exceeds the staggered arrangement
 589 (B.2), which itself performs particularly well due to the flow direction. How-
 590 ever, the greedy optimisation technique leads to 1.8% lower average power than
 591 SLSQP, although it does require almost 0.1% and 2% of the computational
 592 time for $N_t = 8$ and $N_t = 15$ turbine setups respectively. Given the required
 593 time for a steady state channel optimisation, the reduction in computational
 594 time becomes appreciable relative to adjoint optimisation, which has not been
 595 explored further in this work for unsteady cases.

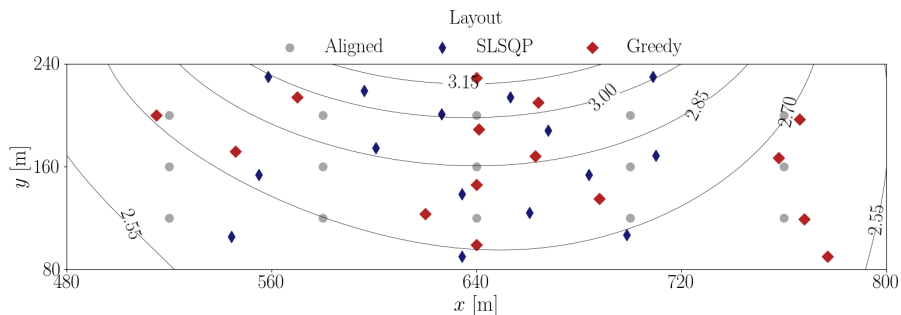


Fig. 10 Aligned (B.5) and optimised layouts (B.7, B.8) overlaid on velocity contours for peak flow within the idealised headland channel.

596 5.3 Application to the Pentland Firth, Scotland, UK

597 Using three frames from each flood and ebb tide for spring, intermediate and
 598 neap cycles, optimisation of $N_t=24$ turbines subject to a minimum spacing
 599 of $3D$ (C.4, Table 6) resulted to increased average P relative to an aligned

600 case (C.1) by 12.4% over a month's period. Optimisation made use of 18 flow
 601 frames, with additional frames delivering no substantial benefit to the overall
 602 performance. This is attributed to the generally consistent flow direction at
 603 peak magnitudes over flood and ebb tides (as illustrated by the flow fields in
 604 Fig. 11). The importance of using representative frames for a full tidal cycle
 605 (as well as a spring-neap cycle) is demonstrated by Fig. 12. Optimisation is
 606 less effective during flood flows, and even less so during spring tides. This is
 607 due to the deployment of turbines that experience flow velocities noticeably
 608 greater than u_{rated} for a large proportion of the velocity magnitudes expected
 609 within the allocated plot. As a result, these are predicted to deliver maximum
 610 power irrespective of compounded wakes. This would suggest that within this
 611 area, neglecting structural constraints and metrics such as the capacity factor,
 612 it may be worth using turbines of higher u_{rated} to fully exploit the potential
 613 energy available. Nevertheless, a positive increase in capacity factor from 42.6%
 614 to 47.9% is achieved for a minimum spacing of $3D$ that could have a significant
 615 influence on the feasibility of such an installation.

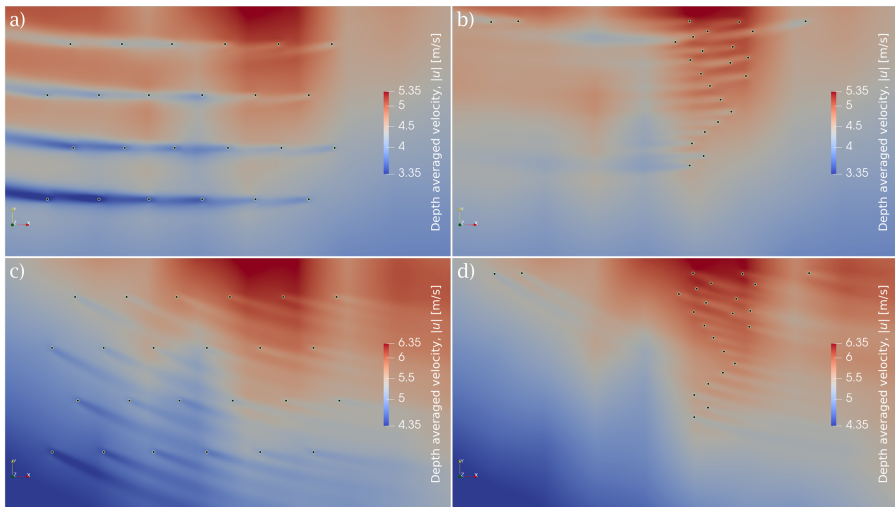


Fig. 11 Flood and ebb flow for Pentland Firth case study at peak spring tide computed using the *Thetis* model with turbine drag simulated. a) staggered arrangement (C.2), ebb; b) greedy optimised arrangement (C.5), ebb; c) staggered arrangement (C.2), flood; d) greedy optimised arrangement (C.5), flood.

616 Greedy optimisation accomplished this improvement within 27 iterations,
 617 taking less than a minute. In loosening the minimum spacing constraint to
 618 $1.5D$, a capacity factor increase to 49.4% is achieved, at the cost of additional
 619 iterations and computational time in the order of a few minutes. As
 620 Table 6 shows, in a practical case where velocities exceed u_{rated} and the flow
 621 field is more varied and complex largely due to the local bathymetric profile,
 622 a staggered arrangement (C.2) is less effective than in idealised geometries

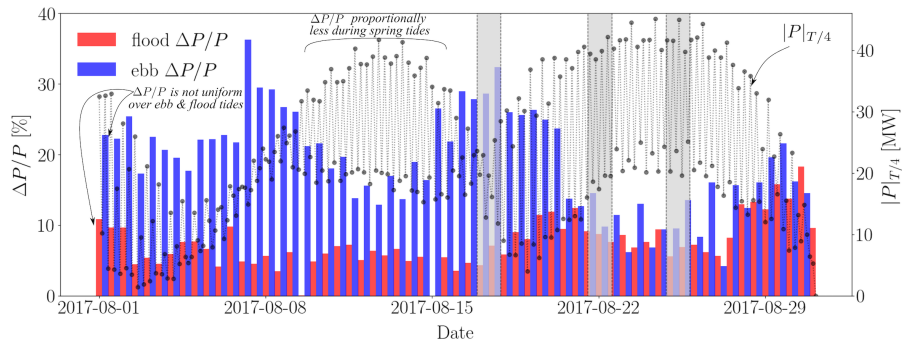


Fig. 12 Relative power increase ($\Delta P/P$) comparing greedy (C.4) to staggered (C.2) case of the Pentland Firth for 24 turbines. Shaded areas indicate periods used to extract ambient flow fields for optimisation. $|P|_{T/4}$ corresponds to the average power in increments of $T/4$. Similar trends are observed comparing optimised layouts based on minimum $1.5D$ spacing (C.5 vs C.2, C.7 vs C.6).

623 (e.g. A.2 and B.2). With more turbines within the staggered arrangement,
 624 the interaction of multiple wakes has a far more profound effect as shown in
 625 Fig. 13a,c; the variation of flow velocities means that turbines perform better
 626 packed into regions of higher average kinetic power density ($\overline{\text{KPD}}$, where KPD
 627 $= \frac{1}{2}\rho|\mathbf{u}|^3$). Again, this emphasizes the optimisation's sensitivity to turbine
 628 u_{rated} ; in regions where peak flow speeds regularly exceed u_{rated} , turbines will
 629 operate at their maximum capacity, despite wake impingement. For an array
 630 of doubled size ($N_t = 48$), the relative increase in power becomes less signifi-
 631 cant for this reason. Although the initial staggered arrangement at $3D$ spacing
 632 (C.6) appears to allow for greater wake avoidance than the $N_t = 24$ array of
 633 $5D$ spacing (C.2) due to the localised flood direction, the increased density of
 634 turbines at the northern, more energy dense, section of the site in combination
 635 with low u_{rated} relative to the flow speed, corresponds to lesser noticeable gains.
 636 This issue leads to denser turbine arrangements, but is specific to layout opti-
 637 misation seeking energy maximisation rather than mitigating hydrodynamic
 638 (wake) interactions. Fig. 11d sees turbines positioned in areas of high KPD,
 639 whereby the southern parts of the site are avoided on the grounds of lower
 640 flow magnitudes. Notably, beyond the allocated area for turbine deployment,
 641 flow speed exceeds 5.5 m s^{-1} towards the island of Stroma (Fig. 6). Harnessing
 642 the kinetic energy there would be technically challenging due to the shallower
 643 bathymetry and sharper bed gradients. In the optimised configurations, few
 644 turbines lie within the high velocity deficit region of upstream wakes due to
 645 conditions preventing the reduction of individual turbine power. This is par-
 646 ticularly critical during neap tide when flow speeds are low enough to place
 647 emphasis onto wake interaction, hence the benefit of optimising for several
 648 varying tidal cycles.

649 A key consideration when examining practical cases is the impedance of
 650 the flow due to the presence of turbines (array blockage). The change in volu-
 651 metric flow over a 1-day period is presented in Table 7 to quantify the impact

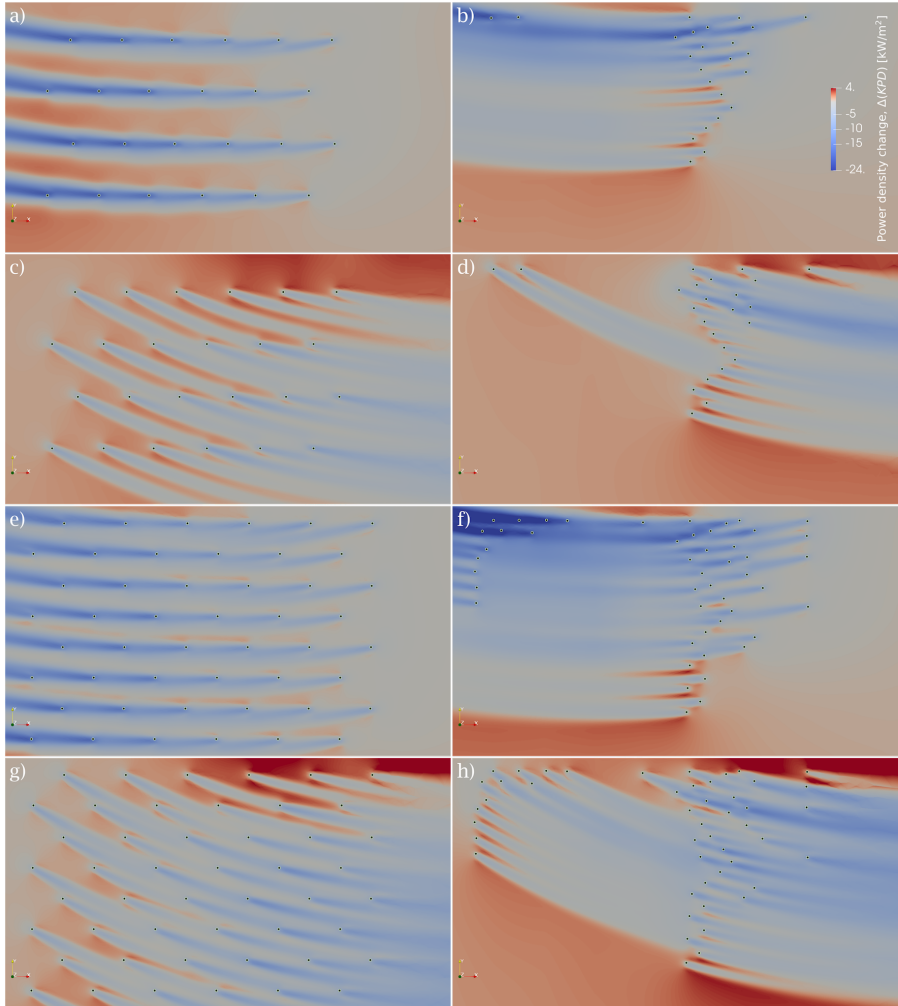


Fig. 13 Flood and ebb change in kinetic power density (KPD) for Pentland Firth case study at peak spring tide computed using the *Thetis* model with turbine drag simulated. a) staggered arrangement (C.2), ebb; b) greedy optimised arrangement (C.5), ebb; c) staggered arrangement (C.2), flood; d) greedy optimised arrangement (C.5), flood; e) staggered arrangement (C.6), ebb; f) greedy optimised arrangement (C.6), ebb; g) staggered arrangement (C.7), flood; h) greedy optimised arrangement (C.7), flood.

652 of the turbine drag on the channel flux, as per [Coles et al. \(2017\)](#). Through
 653 the array width only, the reduction in volumetric flow remains around 4% for
 654 the 24 turbine cases, which is not particularly significant for a spring tide and
 655 results partially from the low global blockage of the array and the spaced out
 656 distribution of turbines to minimise velocity deficits. As the array size and its
 657 turbine density is relatively small when compared to the size of the channel,
 658 the influence on the Inner Sound is localised suggesting minimal diversion of

659 flow on a regional scale. With increasing array size (Fig. 13e-h), the impact of
 660 global blockage effects will likely become more critical, particularly considering
 661 site-to-site interactions over the entire Pentland Firth and the Orkney Islands
 662 (De Dominicis et al., 2018). Although still sparse relative to the size of the
 663 Inner Sound, the reduction in volumetric flow through both the array width
 664 and the Inner Sound itself doubles when $N_t = 48$ in case C.7, demonstrating a
 665 proportional increase in blockage effects. As *FLORIS* does not consider block-
 666 age effects, it becomes instructive to compare velocity changes, $\overline{\Delta \mathbf{u}}$, relative
 667 to the equivalent *Thetis* setup, as in Fig. 14. *Thetis* predicts zones of velocity
 668 deficit and flow acceleration at the top and bottom of the array respectively.
 669 These effects indicate the onset of notable array-scale impacts as turbines form
 670 a denser configuration, with wakes of turbines eventually merging to form
 671 wider $\overline{\Delta \mathbf{u}}$ regions.

Table 7 Change in volume flux with the introduction of the greedy array layout, over transects of the array width and Inner Sound for a Spring cycle. Different transects are used for the ebb, flood and overall volume flux changes to best account for the impact of the array in each case. A negative change represents a decrease in flow through the channel when turbines are introduced.

Optimised Layout	Cycle	Volume flux change, $\Delta Q/Q_{\text{amb}}$ (%)	
		Array Width	Inner Sound
C.4 (24 turbines, 3D)	Ebb	-3.33	-0.47
	Flood	-3.59	-0.17
	24 hours	-3.66	-0.57
C.5 (24 turbines, 1.5D)	Ebb	-4.19	-0.55
	Flood	-3.31	-0.20
	24 hours	-3.86	-0.61
C.7 (48 turbines, 1.5D)	Ebb	-7.72	-1.17
	Flood	-6.76	-0.32
	24 hours	-7.39	-1.00

672 6 Discussion

673 6.1 On the turbine representation and the consideration 674 of local and global blockage

675 The general array micro-siting pattern returned by the optimisation
 676 approaches (SLSQP and greedy alike) sees turbines positioned within high
 677 power density regions (Fig. 11) and otherwise spread to maintain separation
 678 whilst avoiding wake interaction. The latter agrees with results reported pre-
 679 viously by Stansby and Stallard (2016) that emphasise wake avoidance within
 680 the optimisation process.

681 Under operational conditions below u_{rated} , variation in wake representa-
 682 tion can compromise optimisation, as key velocity deficit areas may not be
 683 captured accurately. If wake width is underestimated in the analytical model,

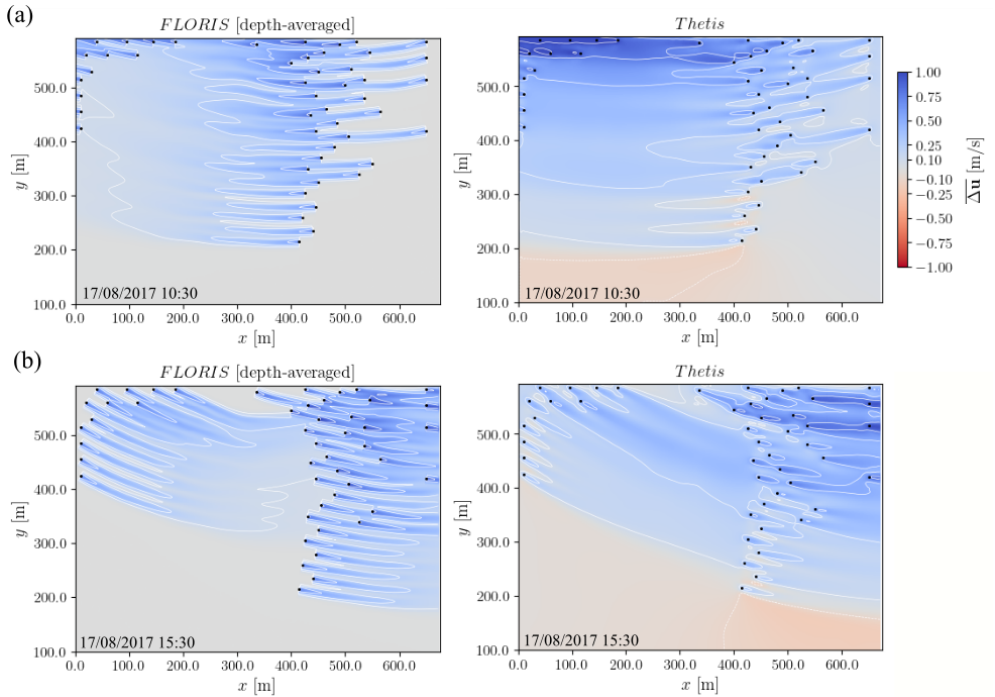


Fig. 14 Wake velocity deficit $\overline{\Delta u}$ predicted by *FLORIS* (left) and *Thetis* for Case C.7 over (a) ebb and (b) flood conditions. Coordinates are based in m from the bottom left corner of the allocated tidal array area located at (491770, 6501730) m based on a UTM30N projection.

684 some of the turbines may become partially immersed in upstream wakes when
 685 evaluated by the hydrodynamic model. This highlights the significance of cali-
 686 brating the wake model parameters as per Section 3.2. Additional parameters
 687 can be considered to improve accuracy, such as varying the turbulent intensi-
 688 ty, \mathcal{I} , as a function of \mathbf{u} , for better agreement against data when applied to
 689 non-idealised cases of complex bathymetry (as in Fig. 14). These parameters
 690 were assumed constant in this study for simplicity, but should be calibrated as
 691 they are subject to inflow conditions and varying turbulence levels. The inclu-
 692 sion of local blockage effects, which has been shown to be possible through
 693 ad-hoc corrections in analytical approaches such as *FLORIS* (Branlard and
 694 Meyer Forsting, 2020), could also benefit optimisation in high-density, confined
 695 scenarios of rectilinear flow.

696 In the case study within the Pentland Firth we consider a turbine array
 697 subject to a rating curve, varying flow directionality and practical clear-
 698 ance constraints. Firstly, we note the necessity of the ambient flow model to
 699 correctly capture the flow magnitude and direction over the tidal array deploy-
 700 ment area as discussed in Section 4.3. Otherwise, even minor departures from
 701 the actual flow direction will lead to a suboptimal array design (see Fig. 7).
 702 These deviations are typically attributed to under-resolved spatial features as

703 discussed by Mackie et al. (2021a). We observe that the varying flow direction
704 over ebb and flood tides renders blockage challenging to exploit. Consider-
705 ing the flow velocity across the array, we also note the persistent exceedance
706 of the rated velocity, u_{rated} , across spatial and temporal scales. As velocities
707 exceed u_{rated} , wake effects become locally constrained. These conditions, com-
708 pounded by non-rectilinear flow, make the “duct effect” difficult to exploit
709 and thus less influential on power production. This is more noticeable during
710 spring tides as $|\mathbf{u}| > u_{\text{rated}}$ over a longer fraction of the tidal cycle, and optimal
711 siting of turbines becomes less beneficial in terms of power output (Fig. 12).
712 These observations are informed by current practices, where turbines are pro-
713 posed to be deployed in channels where peak flow speeds comfortably exceed
714 u_{rated} . This is due to alternative objectives, such as maintaining a competitive
715 capacity factor over the device lifetime.

716 Furthermore, minimum turbine spacing may be forced to exceed $3D$ (Ouro
717 et al., 2019) in the in-stream direction (the value used in this study as a
718 typical separation constraint). The minimum spacing of $1.5D$ that enables
719 closer packing of turbines laterally, as in Ouro et al. (2019), can be challeng-
720 ing to accommodate due to O&M practicalities, complex terrain constraints
721 and non-rectilinear oscillatory flow. These considerations can restrict turbine
722 placement and reduce to an extent the positive influence of local blockage.
723 On that, Nishino and Willden (2013) analytically found that with increasing
724 turbine density in a partial tidal fence, optimal local blockage will increase
725 for both low and high global blockage cases. The benefit of exploiting block-
726 age effects was demonstrated numerically in Funke et al. (2014) where an
727 adjoint-optimisation approach promoted positioning of idealised turbines (i.e.
728 not subject to a u_{rated}) to form highly dense fence-like structures. It must be
729 remarked that in that case, the resistance introduced by individual turbines
730 was exaggerated (as the focus was instead on demonstrating the adjoint opti-
731 misation methodology), amplifying the benefits of local blockage. However,
732 within the steady-state flow through an idealised channel, and whilst consid-
733 ering more practical representations of turbine resistance and turbine density
734 ($3D$), the adjoint optimisation in *Thetis* delivers minimal gains over our greedy
735 or *FLORIS*’s SLSQP-based approaches.

736 A feasible placement of turbines within a channel such as the Pentland
737 Firth will be highly dependent on a number of factors including the bathymetry
738 gradient, bedrock hardness, turbulence loading and a variety of installation
739 and maintenance challenges. The initial turbine density for the Pentland Firth
740 case study was based on an initial separation of $5D$. If the density is increased
741 so that maximum initial separation is reduced to $3D$ (whilst increasing the
742 number of turbines in the initial array), local blockage effects become more
743 prominent, as indicated by the increase in power density around devices in Fig.
744 13. Nevertheless, quantification of the blockage effects by monitoring fluxes and
745 the power density changes over the array (Table 7) suggests that this remains
746 a low blockage case.

6.2 On the characterisation of array hydrodynamics

Our optimisation approach relies on the use of analytical wake models that typically assume steady-state conditions. The practice of wake superposition itself introduces a mass and momentum deficit that is not compensated without additional corrections (e.g. Branlard and Meyer Forsting (2020)); these necessitate the assessment of *FLORIS* derived layouts within hydrodynamics models (Fig. 14). On the other hand, the hydrodynamics model (in this case *Thetis*) does not capture horizontal flow structures below the mesh-size scale which means that many unsteady and quasi-steady flow phenomena are not considered in our analysis. In particular, turbulent mixing occurring at smaller scales is not modelled, which has been shown to influence wake evolution, as also recognised by the wind energy community (Singh et al., 2014). Overall, this represents an outstanding research area involving complex multi-scale flow modelling. Another phenomenon of relevance which is not captured in our simulations is *dynamic wake meandering*. As turbine wakes interact with the larger tidal-channel turbulent structures, such as near-wall high- and low-speed streaks, near-wake vortices start breaking down giving way to the generation of a cascade of turbulent scales. Additionally, the wake experiences lateral and vertical displacements caused by the larger-scales leading to their significant lateral expansion. These effects are not encapsulated within hydrodynamic models unless the model spatial and temporal resolution is increased and/or combined with more robust turbulence models that capture these effects while avoiding excessive dissipation in the solution. Inherently, all 2-D models are limited in their ability to capture dispersion effects due to the assumed uniform vertical velocity. These considerations may have implications for the final prediction of the wake deficits and therefore also affect the optimal array layout solution. 3-D shallow-water models on the other hand, can improve the representation of such scales as shown by Stansby (2003) through the addition of a horizontal mixing length scale which alters the velocity profile over the water column, resulting in greater vertical shear; however, further research is required in order to quantify their impact on wake dynamics.

Regarding the global array wake, experimental studies on turbulent wakes downstream of a two-dimensional porous obstruction (Zong and Nepf, 2012) show that the steady wake region increases with increasing porosity whereas the unsteady von Kármán vortex street may be delayed until well beyond the steady wake region. Given the low turbine density, as demonstrated by the global array volume flux (see Table 7), the array's equivalent porosity is small, thus we argue that no further quasi-steady effects are likely to be present in the array-scale wake region. Turbine-scale unsteadiness in the individual turbine's near wake region may be accounted for by a locally modified eddy-viscosity.

An alternative approach for the local and global hydrodynamics may be undertaken using higher-fidelity models such as those that utilise three-dimensional Reynolds-averaged Navier–Stokes (RANS) (Abolghasemi et al., 2016; Deskos et al., 2017) or large-eddy simulation (LES) methods (Churchfield et al., 2013; Ouro and Nishino, 2021) which inherently allow for greater

insight and accuracy in the near-wake region by allowing both horizontal and vertical wake dispersion through scale-resolving simulations. Such simulations emphasise how wake avoidance is not only critical for maximum exploitation of the channel potential, but also in reducing turbulence onto downstream turbines which may compromise the devices' lifetime due to fatigue (Thiébaud et al., 2020). Nevertheless, 2-D models are currently the standard option for regional assessments (Coles et al., 2020) and help counteract the computational cost within an optimisation framework. As demonstrated in Section 3.2, and in particular Fig. 4, it would be entirely possible to apply the same methodology to a 3-D higher-fidelity either coastal ocean or turbulence-resolving model to acquire greater consistency with measured data.

6.3 On the potential applications for large tidal array optimisation

Whilst a number of optimisation approaches have been proposed for the micro-siting of tidal turbines, these have been limited to idealised setups, or limited control parameters in terms of turbine placement. Some of the more sophisticated methods (e.g. Funke et al. (2016); Culley et al. (2016)) that consider blockage effects remain computationally and memory intensive. Taking our practical example of the Pentland Firth, an earlier approach required 24–48 hours on a 64-core supercomputer for a steady state simulation (Funke et al., 2014). Though pioneering, practical constraints including rated turbines, transient tidal flows and realistic bathymetry were not considered in early studies despite having an influence on the interactions between devices and the resource. Similarly, optimisation methods that estimate the objective function gradient iteratively (e.g. SLSQP), quickly become computationally demanding due to the quadratic complexity ($\mathcal{O}(n^2)$) of the optimisation algorithm. In the optimisation problem presented by the practical case (C.3), SLSQP becomes significantly more costly as the domain size and turbine number N_t increase. Given a tendency to converge to local minima it becomes distinctively ineffective for complex domains in the absence of a reliable gradient calculation strategy. The customised greedy approach developed here overcomes these computational constraints and offers a route to also incorporate additional features. These may include cabling constraints (Culley et al., 2016), seabed gradient restrictions, several turbine options and other factors such as wake steering which are considered in the optimisation of offshore wind farm operation (Deskos et al., 2020). However, greedy optimisation strategies possess shortcomings (Bang-Jensen et al., 2004), and whilst they can deliver a locally optimal solution in reasonable time, they must be applied and interpreted with their limitations in mind.

Adjoint-based and greedy methods could be combined in a cyclic manner for optimisation in larger domains whereby a greedy approach acts as a precursor that delivers an initial design to improve upon through adjoint-optimisation. This will sequentially seek to exploit hydrodynamics effects by exploring the parameter space through localised turbine displacements starting

836 from a decent design that should result in the requirement for less optimisation
837 iterations than a pure adjoint-based approach. It may also mitigate the issue
838 of getting stuck in a sub-optimal local optima. Alternatively, given the compu-
839 tational efficiency of the customised greedy optimisation, opportunities can be
840 explored to optimise for $N_s \subset N$ in fractions of the turbine deployment area
841 at a time. Turbines introduced can then be included in forward hydrodynam-
842 ics simulations to account for hydrodynamic impact and blockage effects when
843 designing the rest of the array. This approach could avoid the sudden introduc-
844 tion of substantial array impacts (Fig. 14) by incremental addition of turbines
845 in the array within an optimisation iteration. Extensions can also be made
846 towards multi-objective optimisation that balances cost against environmental
847 impacts (e.g. sediment transport or implications for benthic species habitats).
848 This could follow recent work on environmentally constrained optimisation by
849 [du Feu et al. \(2019\)](#).

850 7 Conclusions

851 A novel optimisation method was demonstrated by retrofitting an analyti-
852 cal wake superposition model, in this case *FLORIS*, for use with a coastal
853 hydrodynamics model, *Thetis*. The method is motivated upon reflection on the
854 bottlenecks observed in existing array optimisation approaches, which depend-
855 ing on acceptable computational costs may be constrained to (a) simplified
856 flow geometries, (b) steady-state flow conditions and (c) idealised turbine rep-
857 resentations. The work is driven by the complexity of the array micro-siting
858 problem, where an effective optimisation method should be able to deal with
859 complex flows caused by local bathymetric features and regional coastline,
860 the transient tidal flows over spring neap cycles, and the technical specifica-
861 tions and performance characteristics of the turbine technology that is to be
862 deployed. Once a hydrodynamic model delivers the spatially and temporally
863 varying flow information over a prospective development area, application of
864 a custom greedy placement algorithm within an analytical wake superposition
865 model allows for rapid optimisation.

866 The methodology was applied to three cases of increasing complexity (in
867 terms of geometry, oscillatory flow, and array turbine number) and was able
868 to demonstrate its potential and highlight multiple considerations emerging as
869 we progress from idealised to practical settings. For a simple steady-state rect-
870 angular channel, turbines were arranged in a longitudinally staggered fashion
871 across the domain, utilising the full width of the domain whilst maintaining
872 separation constraints, consistently with alternative optimisation strategies
873 (e.g. SLSQP and adjoint-based optimisation). The headland case demonstrated
874 the capacity to deal with more complicated flows and emphasised the trend
875 of turbines being positioned in areas of higher power density, whilst avoiding
876 wake effects from upstream turbines during ebb and flood flows. The opti-
877 misation scenario of 24 turbines in a confined region within the Pentland

878 Firth demonstrated the ineffectiveness of staggered arrangements for non-
 879 rectilinear oscillatory flows, and the computationally efficient application of
 880 this methodology for complex geometries and flow dynamics. It was found that
 881 the resultant method yielded an overall improvement in power output in the
 882 order of 12% for 3D minimum spacing and up to almost 16% when reduced
 883 to 1.5D.

884 Finally, it was observed that flow asymmetry in conjunction with min-
 885 imum distance requirements may render the exploitation of local blockage
 886 effects rather challenging. Case studies using 24 and then 48 turbines respec-
 887 tively within the Meygen site at the Pentland Firth indicated low levels of
 888 global blockage. However, as the number of turbines doubles to 48 in the latter
 889 case, blockage effects start to become more noticeable. Given the extensions
 890 expected as tidal arrays expand, it is proposed that the optimisation approach
 891 presented can be operated iteratively enabling the hydrodynamic model to
 892 account for array-scale blockage as the size of the array is extended.

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