

1 **Turbulence structure and the development of secondary outer-bank flow**
2 **cells at multiple discharges in a meander bend**

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22 **Abstract**

23 The erosion of the outer-banks of meander bends is mediated by the form
24 roughness of the bank topography, which has been shown to affect near bank
25 three-dimension flow structures and shear stresses. As the scales of bank
26 roughness is known to vary vertically from bank toe to bank edge variations in
27 flow discharge are likely to driver changes in near-bank flow velocities and
28 turbulent structures in the near-bank zone. However, to date field observations
29 of three-dimensional flow in the near-bank zone across multiple discharges
30 remain limited. Here, we report observations of three-dimensional flow
31 characteristics in the near-bank zone of a meander bend at a range of discharge
32 levels; low ($9 \text{ m}^3 \text{ s}^{-1}$), mid- ($23 \text{ m}^3 \text{ s}^{-1}$) and bankfull ($90 \text{ m}^3 \text{ s}^{-1}$). Our results
33 reveal that the relationship between the presence of a secondary outer-bank
34 flow cell (SOC) and magnitudes of turbulent kinetic energy (TKE) and shear
35 stresses vary with discharge. At high flows, the presence of a SOC is related to
36 increased TKE, whilst at mid-flow discharges SOCs are associated with reduced
37 TKE compared to transects where SOCs are absent. At low flows there is no
38 observed pattern between TKE and the presence/absence of a SOC. We attribute
39 these patterns to the role that failed material at the bank toe plays in deflecting
40 and modulating near-bank flows. This work highlights how important detailed
41 flow data across multiple discharges is in terms of understanding flow through
42 meander bends and thus the impact these flow processes will have on longer
43 term bend morphodynamics.

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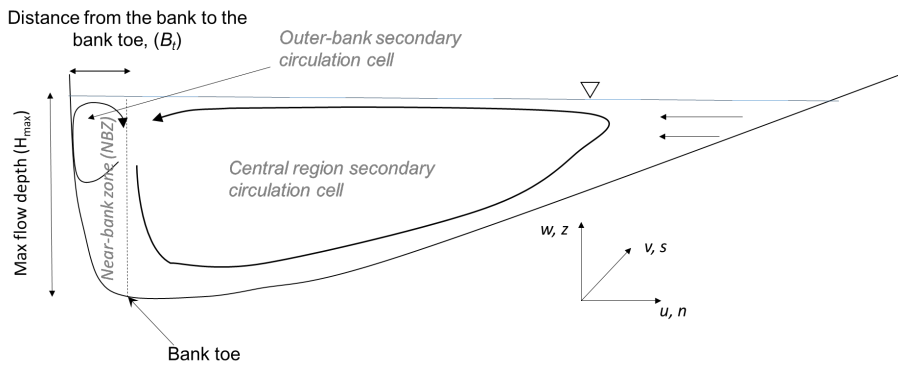
45 **Keywords**

46 Bank erosion, flow structures, near-bank flow, turbulence

47 **1. Introduction**

48 Given that flows in natural channels are fully turbulent, it is expected that outer-
49 bank erosion rates are also controlled to a large extent by turbulent stresses
50 (Abad and Garcia, 2009a; Engel and Rhoads, 2017; Jamieson et al., 2010). The
51 removal and remobilisation of bank sediment resulting from this interplay
52 ultimately drives channel migration and results in the generation of classical
53 meandering patterns across many lowland alluvial rivers (Brice, 1974; Parker et
54 al., 2011; Rinaldi and Darby, 2007). The erosion of the outer-banks of meander
55 bends is mediated by the form roughness of the bank topography, which has
56 been shown to affect near bank three-dimension flow structures and shear
57 stresses (Hackney et al., 2015; Kean and Smith, 2006a, 2006b; Konsoer et al.,
58 2016). As the scales of bank roughness is known to vary vertically from bank toe
59 to bank edge (Konsoer et al., 2017), as flow levels vary across a hydrograph,
60 the scales of roughness that actively impact on the near-bank flow velocities will
61 change. There is, therefore, a dependence upon flow discharge with respect to
62 the modulation of near-bank flow velocities and turbulent structures in the near-
63 bank zone (NBZ: here defined as the region of flow from the bank edge to the
64 bank toe; Figure 1) resulting from bank roughness that is not yet fully
65 appreciated. Whilst previous research has highlighted the complex nature of
66 outer-bank turbulence (Blanckaert, 2009; Blanckaert et al., 2012, 2013; Engel
67 and Rhoads, 2012, 2016, 2017), field observations of three-dimensional flow in
68 the NBZ remain limited.

69



70

71 **Figure 1:** Definition sketch of the idealised flow components in a meander bend. Co-ordinate definitions are provided
 72 where v , u and w represent primary, transverse and vertical flow components of the mean cross section whilst s , n and z ,
 73 represents the conversion of these respective components onto a curvilinear grid.

74

75 The development of secondary flow in meandering channels is a result of the
 76 curvature-induced imbalance between centrifugal forces operating of the flow as
 77 it passes through a curved channel, and the cross-stream water surface slope
 78 which generate a pressure gradient directed towards the inner bank (Blanckaert
 79 et al., 2013; Rozovskii, 1957). Although in larger channels the presence of
 80 secondary flow is absent due to the large channel widths which subdue the
 81 cross-stream water surface slopes (Hackney et al., 2017; McLelland et al., 1990;
 82 Parsons et al., 2007), in smaller channels secondary flow cells, have been well
 83 described (Bathurst et al., 1977; Blanckaert et al., 2013; Markham and Thorne,
 84 1992; Thorne et al., 1985; Thorne and Hey, 1979; Thorne and Furbish, 1995).
 85 Where flow impinges on the outer bank, bank roughness may induce a reversal
 86 of the velocity gradients, generating a secondary outer-bank cell (SOC; Figure 1)
 87 which rotates with a counter-helicity to the secondary flow cell in the channel
 88 centre. However, direct field-based quantification of the velocity profiles and
 89 turbulent structures close to outer-bends are limited in the literature. Previous
 90 field studies have assessed velocity and turbulence characteristics in the NBZ at
 91 singular discharges (Anwar, 1986; Engel and Rhoads, 2017) or has been limited
 92 to regions away from the NBZ (e.g. Sukhodolov, 2012). These studies show that

93 turbulent kinetic energy is relatively high in the NBZ as the high-velocity core is
94 displaced towards the bank toe. However, given the few studies which assess
95 flow fields at a range of discharges, our understanding of the development of
96 SOCs and their impact on turbulent kinetic energy as flow discharge varies is
97 lacking.

98

99 Understanding is further complicated by conflicting evidence from numerical and
100 physical experimental studies. Work by Blanckaert and Graf (2001) and
101 Blanckaert et al. (2012) suggests that the intensity of turbulence, turbulent
102 kinetic energy and the magnitude of Reynolds shear stresses at the outer bank
103 are relatively low compared with the centre of the channel. These low levels of
104 turbulence have been attributed to the development of weak SOCs (Engel and
105 Rhoads, 2017). Conversely, further work has demonstrated that increases in
106 outer bank turbulence relative to the central channel flow are observed when
107 mobile beds are present (Abad and Garcia, 2009a,b; Jamieson et al., 2010).
108 Indeed, Sukhodolov (2012) demonstrates increased turbulent kinetic energy
109 magnitudes with the presence of a SOC, in contradiction to the findings of
110 Blanckaert et al. (2012). However, many of these studies were only undertaken
111 at one flow discharge and changes in these relationships with flow discharge is
112 a notable gap in understanding.

113

114 Here, we report and detail the high-resolution three-dimensional flow fields in
115 the NBZ captured over a range of discharges (at $6 \text{ m}^3 \text{ s}^{-1}$, $23 \text{ m}^3 \text{ s}^{-1}$ and $90 \text{ m}^3 \text{ s}^{-1}$)
116 from a lowland meandering river in the United Kingdom. These datasets allow
117 us to examine the role that time-averaged and turbulent near-bank flow
118 structures play in driving turbulent kinetic energy and shear stresses in the NBZ

119 as discharge level varies. We use this data to critically assess how the presence
120 and development of secondary outer-bank flow cells impact the turbulent
121 stresses directed towards the outer-bank of the channel under varying flow
122 conditions and assess the implications of these changes for longer-term bend
123 morphodynamics.

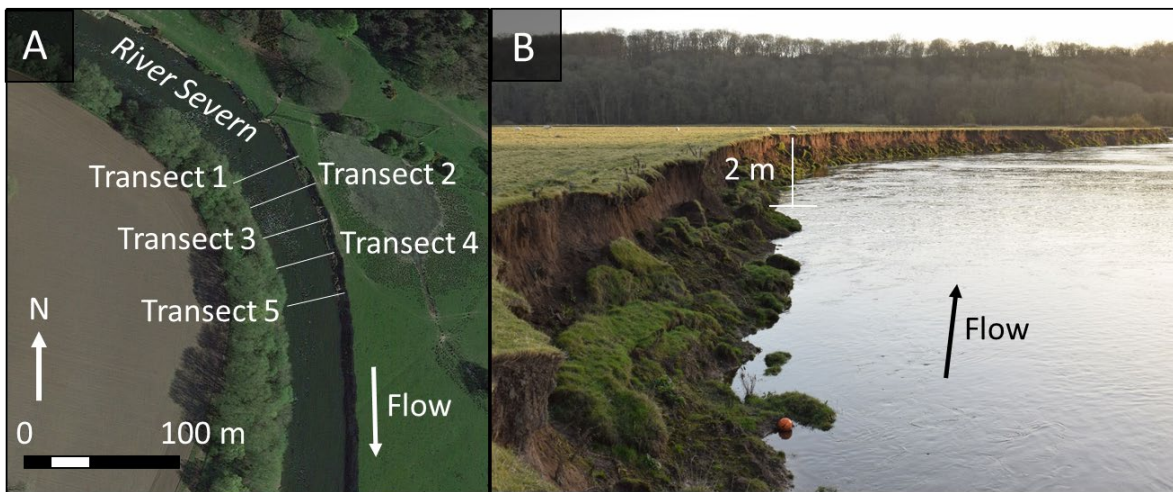
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125 **2. Study site and methods**

126 Field measurements of the three-dimensional flow structure in the near bank
127 zone were collected from a meandering reach of the River Severn, UK (Figure 2).
128 Here, the River Severn displays alternating vertical sandy cut-banks and despite
129 displaying classic meandering sequences incised into poorly indurated terrace
130 gravels (Carling, 1991; Dury, 1983). The study reach has a bankfull width of 45
131 m and a bankfull depth of 4.3 m. Observations of flow were made at three flow
132 discharges representing low ($Q = 6 \text{ m}^3 \text{ s}^{-1}$; August 2018), mid-flow ($Q = 23 \text{ m}^3$
133 s^{-1} ; January 2017) and bankfull flow conditions ($Q = 90 \text{ m}^3 \text{ s}^{-1}$; March 2017).

134 Detailed three-dimensional, time-mean flow velocity fields were collected using a
135 Sontek M9 Acoustic Doppler Current Profiler (ADCP). A series of flow-
136 perpendicular cross-sections were recorded and recaptured during each survey
137 period (Figure 2). Each transect was surveyed at least four times in order to
138 resolve the time-averaged flow field (Szupiany et al., 2007). The ADCP was
139 coupled with a RTK dGPS used to provide real-time positional corrections. A
140 standard moving-platform setup was used and data was collected with multiple
141 acoustic frequencies (1 MHz and 3 MHz) to best account for changing water level
142 depths and speeds across the section. As such, measured cell sizes ranged from
143 0.06 m (1 MHz readings) to 0.1 m (3 MHz readings). The ADCP data were

144 subsequently processed in RiverSurveyor Live and the Velocity Mapping Toolbox
145 (VMT; Parsons et al., 2013). The resultant mean transects were then rotated
146 using the method of Rozovskii (1957) that has been shown to capture well
147 details of the primary and secondary flow fields in a range of complex channel
148 planforms (Hackney et al., 2015; Lane et al., 2000; Rhoads and Kenworthy,
149 1995; Szupiany et al., 2009).



150

151 **Figure 2: A)** Location of the five transects covered in the study on the River Severn at Leighton,
152 Shropshire, UK. **B)** Image of the outer bank of the reach. Note the blocks of failed material at the
153 toe of the bank which measure $\sim 1 \text{ m} \times 0.5 \text{ m} \times 0.3 \text{ m}$ (L x W x D). Image taken in January 2017,
154 when discharge was $23 \text{ m}^3 \text{ s}^{-1}$. Water depths are 2 m and bank heights are $\sim 2 \text{ m}$ above the
155 water's surface.

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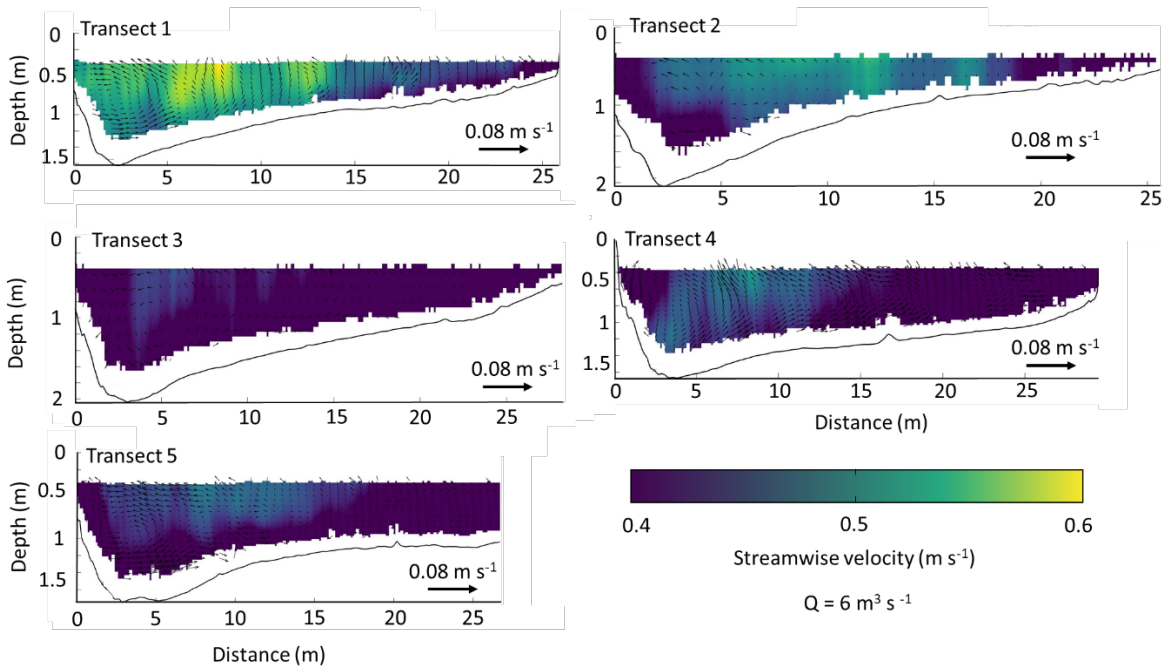
157 **3. Results**

158 **3.1. Time-averaged flow**

159 Time-averaged flow fields at each of the five transects, across the three flow
160 discharges, reveal that a core of higher-velocity is observed in the middle of the
161 channel away from the outer bank (Figures 3, 4 and 5). During low flow ($Q = 6$
162 $\text{m}^3 \text{ s}^{-1}$; Figure 3), depth-averaged velocities range from $0.34 - 0.49 \text{ m}^3 \text{ s}^{-1}$ (Table

163 1). Fastest flows are observed in transects one and two, with higher velocities
 164 ($\sim 0.6 \text{ m}^3 \text{ s}^{-1}$) being observed near the outer bank and in the channel centre in
 165 Transect 1 (Figure 3). Secondary outer-bank cells (SOCs) are observed
 166 Transects 1, 3 and 4 at low flow levels (Figure 3).

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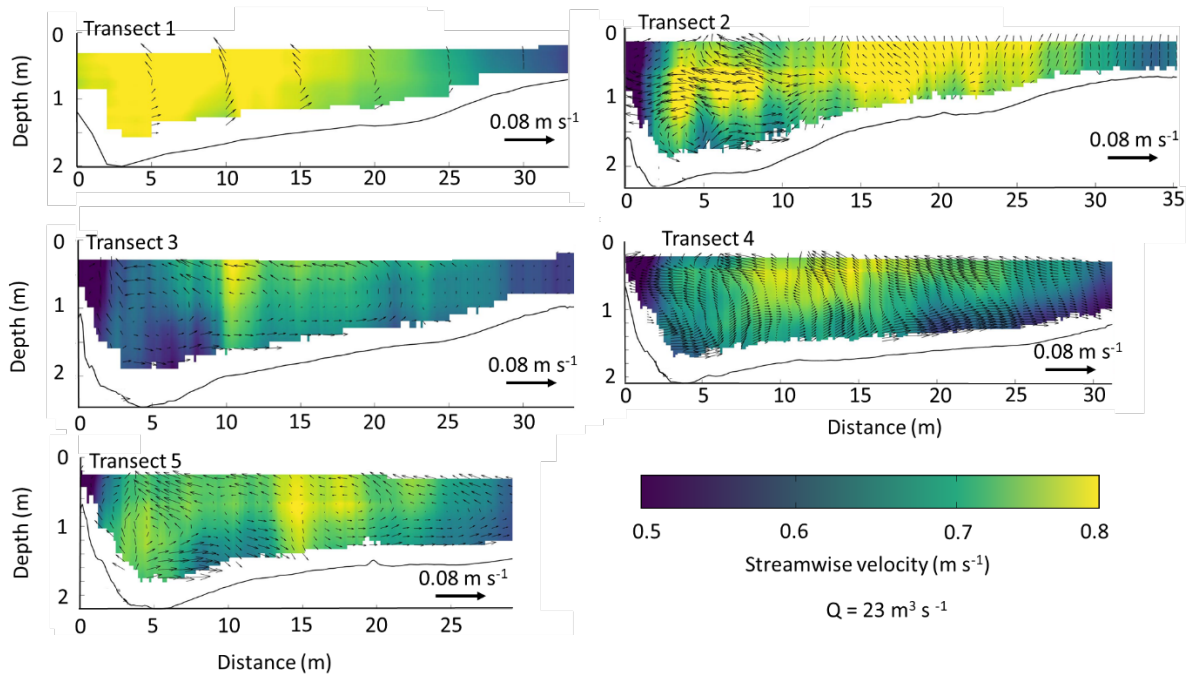


168

169 **Figure 3:** Streamwise flow velocities (m s^{-1}) with secondary flow vectors for each of the five
 170 transects surveyed at low flows ($Q = 6 \text{ m}^3 \text{ s}^{-1}$) in August 2018.

171

172 At mid-flow discharge levels ($Q = 23 \text{ m}^3 \text{ s}^{-1}$; Figure 4 and Table 1) depth-
 173 average velocities range from $0.64 - 0.76 \text{ m}^3 \text{ s}^{-1}$ (Table 1). Flows of 0.7 m s^{-1}
 174 and greater are observed in all five transects, with high velocity cores being
 175 observed in Transects 2 - 5. In Transect 1, velocities of 0.7 m s^{-1} are observed
 176 all the way up to the outer bank (Figure 4). SOCs are observed in Transects 2 -
 177 5, with Transect 1 displaying no SOC at this flow discharge (Figure 4).

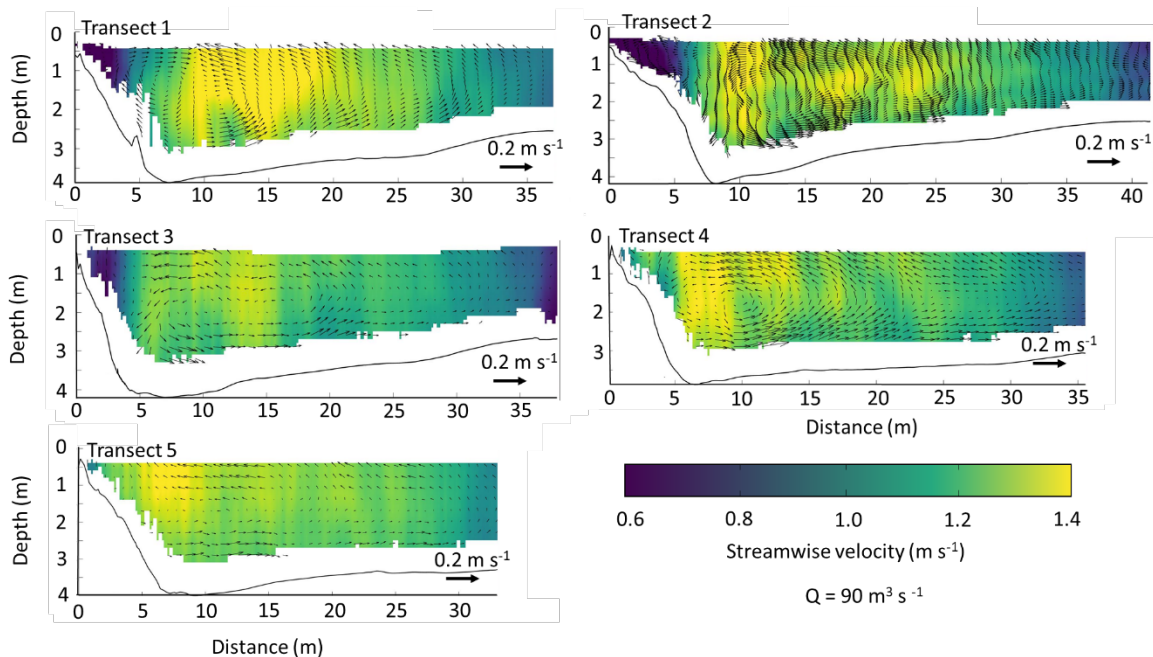


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179 **Figure 4:** Streamwise flow velocities (m s^{-1}) with secondary flow vectors for each of the five
 180 transects surveyed at median flows ($Q = 23 \text{ m}^3 \text{ s}^{-1}$) in January 2017.

181

182 At bank-full flows ($Q = 90 \text{ m}^3 \text{ s}^{-1}$; Figure 5 and Table 1) depth-average velocities
 183 range from $1.08 - 1.21 \text{ m}^3 \text{ s}^{-1}$, more than double those observed during low flow
 184 measurements (Table 1). Flows of 1.2 m s^{-1} and greater are observed in all five
 185 transects. Zones of slower flow near the outer bank are observed in Transects 1,
 186 2 and 3, with high velocities extending to the outer bank in Transects 4 and 5.
 187 SOC's are observed in Transects 1, 3 and 4. In Transects 2 and 5 secondary flow
 188 persists up to the outer bank but appears to be one continuous secondary flow
 189 cell across the whole transect in both cases (Figure 5). In Transect 1, the SOC
 190 extends out to the bank toe; approx. 8 m from the bank edge (Figure 5). In
 191 Transect 3 the SOC extends ~ 5 m out from the bank edge, whilst in Transect 4
 192 the SOC is reduced in width, and only extends ~ 2 m out from the bank edge
 193 (Figure 5).



195

196 **Figure 5:** Streamwise flow velocities (m s^{-1}) with secondary flow vectors for each of the five
 197 transects surveyed at bankfull flows ($Q = 90 \text{ m}^3 \text{ s}^{-1}$) in March 2017.

198 Focussing in closer to the NBZ (see Figure 1 for definition), Figure 6 shows how
 199 the flow characteristics with distance from the outer bank. As can be seen,
 200 primary flow velocities increase in all transects and at all discharges with
 201 distance from the outer bank (velocity profiles in Figure 6 are plotted at $0.25B_t$,
 202 $0.5B_t$, $0.75B_t$ and $1B_t$). Fully developed logarithmic flow profiles are only
 203 observed at the highest discharges and at the greatest distance from the bank
 204 edge, suggesting that the interaction between the bank and the generation of
 205 turbulent flow structures disrupts the development of velocity profiles in the
 206 NBZ. Primary flow velocities are found to be greatest in the NBZ at higher
 207 discharges. Transect 1 displays higher primary velocities in the NBZ than the
 208 other transects at low and mid-flow discharges, suggesting that the channel
 209 curvature impinges the core of high velocity upon the outer-bank at this location
 210 at these flow discharges. At higher flows, it appears the high-velocity core

211 impinges upon the outer-bank further round the meander bend, with the highest
 212 velocities in the NBZ under high flow conditions being observed in Transects 4
 213 and 5 (Figure 6).

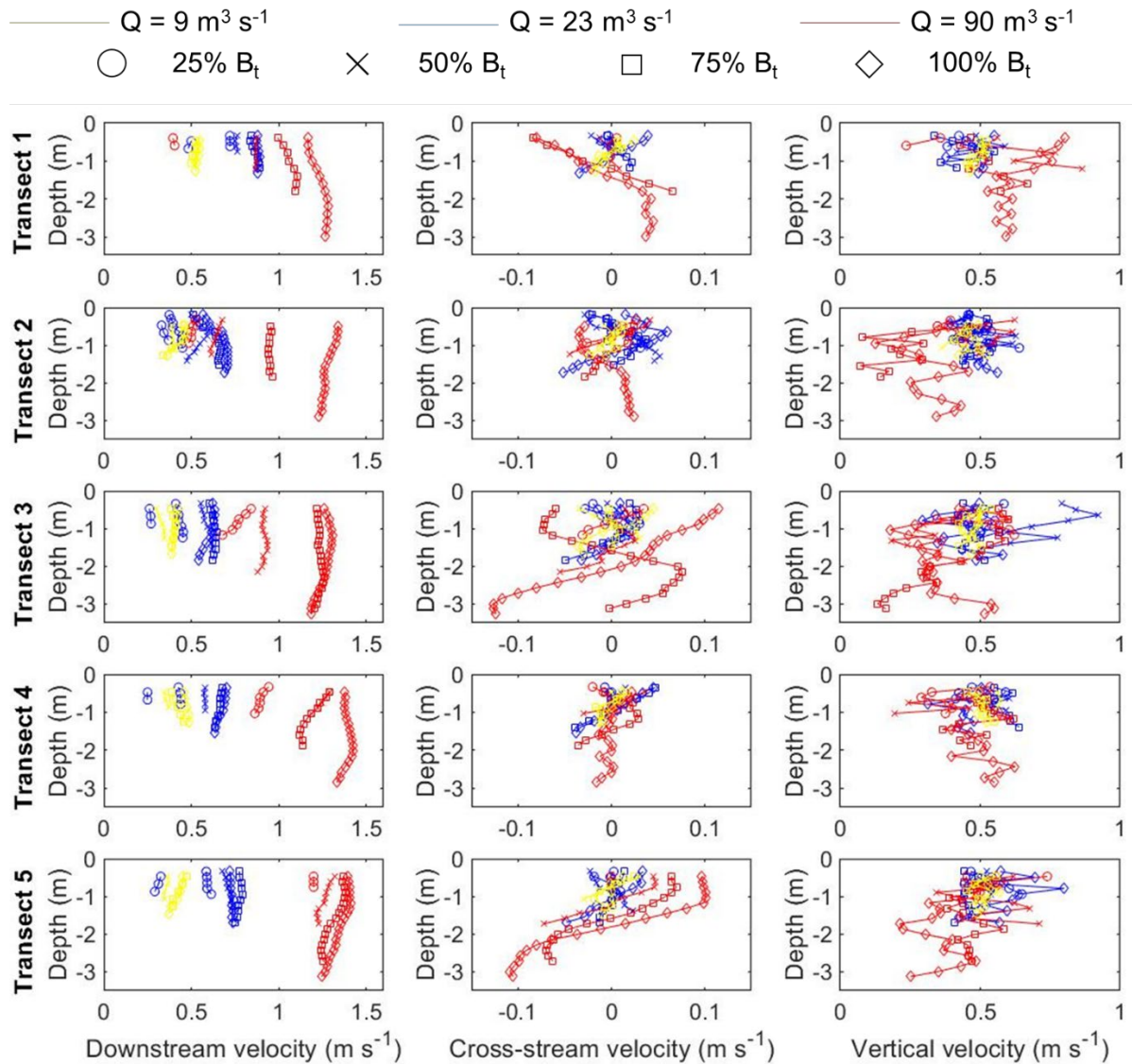
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TRANSECT	Q (M ³ S ⁻¹)	DEPTH-AVERAGED VELOCITY (M S ⁻¹)	MEAN FLOW DEPTH (M)	MAX FLOW DEPTH (M)
1	6	0.49	0.96	1.63
	23	0.76	1.41	2.01
	90	1.14	3.06	3.87
2	6	0.43	1.02	2.05
	23	0.72	1.41	2.30
	90	1.10	2.96	4.18
3	6	0.34	1.24	2.04
	23	0.64	1.72	2.45
	90	1.08	3.33	4.20
4	6	0.35	1.20	1.67
	23	0.66	1.66	2.10
	90	1.17	3.22	3.88
5	6	0.39	1.30	1.45
	23	0.70	1.71	2.20
	90	1.21	3.24	4.00

215 **Table 1:** Flow characteristics of the five transects at the three discharges measured.

216

217 Secondary flows are more pronounced at higher discharges in the NBZ, and are
 218 positioned further around the meander bend (Figure 6). Transitions from
 219 negative (where flow is directed away from the outer bank) to positive (where
 220 flow is directed towards the outer bank), for example observed between $0.75B_t$
 221 and $1B_t$ at high discharges ($Q = 90 \text{ m}^3 \text{ s}^{-1}$) in Transects 3, indicate that the SOC
 222 is located between these two profiles. Similar transitions can be observed at
 223 mid-flow discharges ($Q = 23 \text{ m}^3 \text{ s}^{-1}$) in Transect 1 between $0.5B_t$ and $0.75B_t$
 224 (Figure 6).



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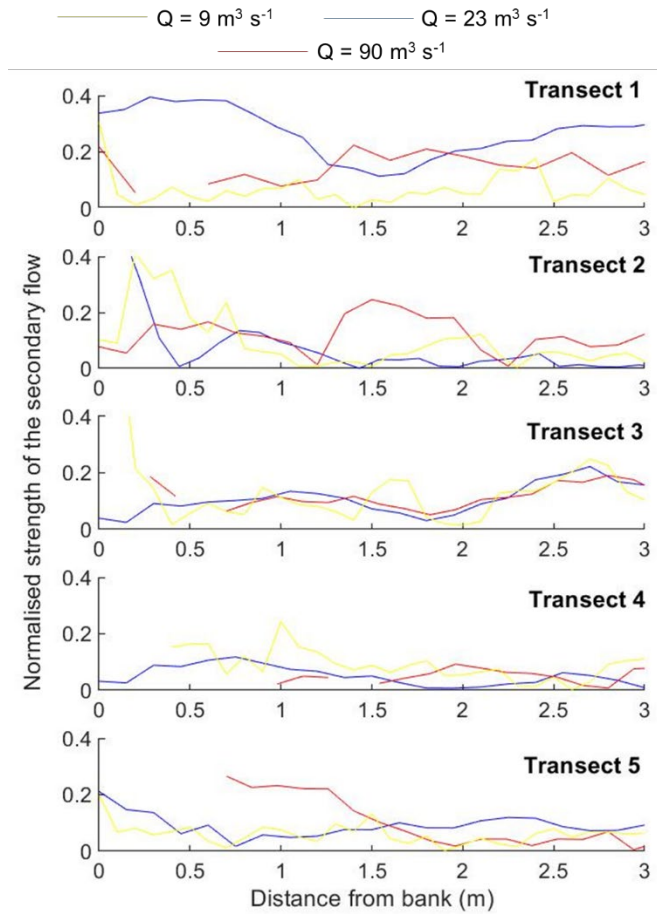
Figure 6: Near-bank downstream, cross-stream and vertical flow velocities (m s^{-1}) at the five survey transects (see Figure 1 for locations) for low flows (yellow lines, $Q = 9 \text{ m}^3 \text{ s}^{-1}$), mid-flow (blue lines, $Q = 23 \text{ m}^3 \text{ s}^{-1}$) and bank full flows (red lines, $Q = 90 \text{ m}^3 \text{ s}^{-1}$). Profiles are extracted at a range of distances from the bank to the bank toe (circles = 25% distance to bank toe, crosses = 50% distance to bank toe, squares = 75% distance to bank toes, diamonds = 100% distance to bank toe), where the bank toe is defined as the maximum channel depth around the outer bend.

3.2 Turbulence and Reynolds shear stresses

234 To assess the role that the SOC plays in driving turbulence and shear stresses
235 on the near-bank and bank-toe region we define the dimensionless strength of
236 the secondary flow component relative to the primary flow velocity, U_{s^*} ,
237 following Blanckaert (2009) such that:

$$238 \quad U_{s^*} = \sqrt{\langle (v_n - u_n)^2 \rangle} / U \quad (1)$$

239 Where $(v_n - u_n)$ is the depth-averaged transverse velocity (m s^{-1}) component of
240 the curvature-induced secondary flow and U is the depth-averaged velocity (m s^{-1}).
241 As shown in Figure 7 secondary flow components are strong in the NBZ. At
242 low flows ($Q = 9 \text{ m}^3 \text{ s}^{-1}$) secondary flow strength is high in Transects 1 – 3 ($U_{s^*} =$
243 $0.3 - 0.4$), suggesting that although discharges are low, there are strong
244 secondary flow patterns in the near bank zone, consistent with the SOC
245 observed in these transects at this discharge (see Figure 3). As U_{s^*} drops
246 towards the centre of the channel at this lower discharge, it is likely that the
247 SOC is relatively stronger than the central secondary flow cell. During mid-flow
248 conditions ($Q = 23 \text{ m}^3 \text{ s}^{-1}$), high secondary flow strength ($U_{s^*} = 0.4$) is observed
249 in Transect 1 despite no SOC being observed in the velocity fields (see Figure 4).
250 In the remaining four transects at these mid-flow discharges, secondary flow
251 strength remains relatively consistent ($U_{s^*} = 0.1 - 0.2$) often increasing towards
252 the centre of the channel (e.g. in Transects 1 and 3) suggesting that the SOC is
253 weaker than the central secondary cell at these discharges. At high discharge (Q
254 $= 90 \text{ m}^3 \text{ s}^{-1}$) U_{s^*} is relatively consistent across the region (~ 0.2) with some
255 localised variation in Transect 2 and higher values (~ 0.3) observed near bank in
256 Transect 5 (Figure 6). This suggests a relative balance between the SOC and the
257 central secondary flow cell at this discharge.



258

259 **Figure 7:** Near-bank distribution of the normalised strength of the secondary flow for the five
 260 surveyed transects at $Q = 9 \text{ m}^3 \text{ s}^{-1}$ (yellow), $Q = 23 \text{ m}^3 \text{ s}^{-1}$ (blue) and $Q = 90 \text{ m}^3 \text{ s}^{-1}$ (red).

261

262 Instantaneous velocities u_i ($i = s, n, z$) from each transect were Reynolds
 263 decomposed into mean \bar{u}_i ($i = s, n, z$) and turbulent u'_i ($i = s, n, z$) components. The
 264 turbulent kinetic energy, k , is thus calculated:

265
$$k = \frac{1}{2} (\overline{u_s'^2} + \overline{u_n'^2} + \overline{u_z'^2})$$

266 and Reynolds shear stresses, τ , are calculated as:

267
$$\tau_{i,j} = \overline{u'_i u'_j} \quad i, j = s, n, z$$

268 Where i, j are index variables indicating the direction component of the velocities
269 (following Engel and Rhoads, 2017). Below, the discussion focusses on the
270 components s and n of the Reynolds shear stresses.

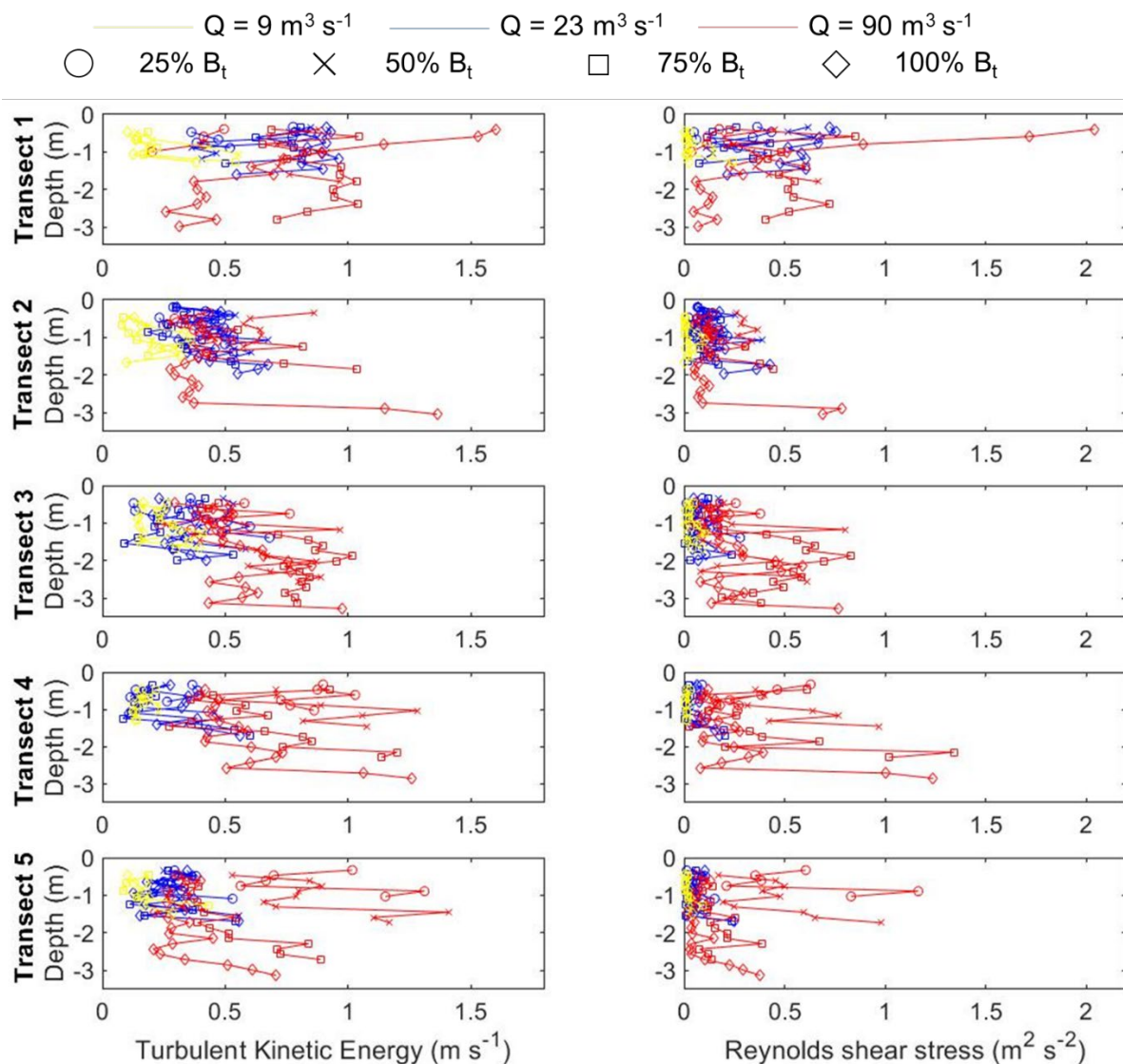
271 Turbulent kinetic energy, k , is greater at higher discharges in all transects
272 (Figure 8). Magnitudes of k vary with distance from the bank. At low discharges
273 ($Q = 9 \text{ m}^3 \text{ s}^{-1}$) magnitudes of k decrease as the distance to the bank toe
274 increases (Figure 8). In Transect 1, depth-averaged k decreases from 0.4 m s^{-1}
275 at $0.25B_t$ to 0.19 m s^{-1} at $1B_t$. At Transect 2, depth-averaged k decreases from
276 0.25 m s^{-1} at $0.25B_t$ to 0.15 m s^{-1} at $0.7B_t$, although rises again at $1B_t$ to 0.23 .
277 At Transect 3, depth-averaged k increases from 0.16 at $0.25B_t$ to 0.29 at $0.75B_t$
278 before decreasing to 0.19 at $1B_t$. At Transect 4, depth-averaged k increases
279 from 0.12 at $0.25B_t$ to 0.18 at $0.5B_t$ before decreasing to 0.15 at $1B_t$. At Transect
280 5, depth-averaged k decreases from 0.28 at $0.25B_t$ to 0.18 at $1B_t$.

281 At mid-flow ($Q = 23 \text{ m}^3 \text{ s}^{-1}$) in Transect 1, depth-averaged k increases from 0.66
282 m s^{-1} at $0.25B_t$ to 0.85 m s^{-1} at $1B_t$ (Figure 8). At Transect 2, depth-averaged k
283 remains consistent at 0.44 m s^{-1} , only varying by 0.05 m s^{-1} between $0.25B_t$ and
284 $1B_t$. At Transect 3, depth-averaged k decreases from 0.47 at $0.25B_t$ to 0.3 at
285 $1B_t$. At Transects 4 and 5, depth-averaged k remain consistently at around 0.3
286 $\text{m s}^{-1} \pm 0.05 \text{ m s}^{-1}$ at all distances from the bank toe.

287 At high discharges ($Q = 90 \text{ m}^3 \text{ s}^{-1}$) the highest values of k are observed, with
288 some locations seeing 0.9 m s^{-1} (Figure 8). In Transect 1, depth-averaged k
289 increases from 0.38 m s^{-1} at $0.25B_t$ to 0.88 m s^{-1} at $0.75B_t$ before decreasing to
290 0.7 m s^{-1} at $1B_t$ (Figure 8). At Transect 2, depth-averaged k increases at 0.35 m
291 s^{-1} at $0.25B_t$ to 0.47 at $1B_t$. At Transect 3, depth-averaged k increases from 0.46
292 at $0.25B_t$ to 0.6 at $1B_t$. At Transects 4, depth-averaged k decreases from 0.86 m

293 s^{-1} at $0.25B_t$ to 0.58 at $1B_t$. Similarly, at Transect 5, depth-averaged k decreases
 294 from a peak value of 0.9 m s^{-1} at $0.25B_t$ to 0.35 m s^{-1} at $1B_t$ (Figure 8).

295



296

297 **Figure 8:** Near-bank turbulent kinetic energy (m s^{-1}) and Reynolds shear stresses ($\text{m}^2 \text{s}^{-2}$) at the
 298 five survey transects (see Figure 1 for locations) for low flows (yellow lines, $Q = 9 \text{ m}^3 \text{ s}^{-1}$), mid-
 299 flow (blue lines, $Q = 23 \text{ m}^3 \text{ s}^{-1}$) and bank full flows (red lines, $Q = 90 \text{ m}^3 \text{ s}^{-1}$). Profiles are
 300 extracted at a range of distances from the bank to the bank toe (circles = 25% distance to bank
 301 toe, crosses = 50% distance to bank toe, squares = 75% distance to bank toes, diamonds =
 302 100% distance to bank toe), where the bank toe is defined as the maximum channel depth around
 303 the outer bend.

304 Reynolds shear stresses, τ , are greater at higher discharges in all transects
305 (Figure 8). At low discharges ($Q = 9 \text{ m}^3 \text{ s}^{-1}$) magnitudes of τ increase as the
306 distance to the bank toe increases (Figure 8), but decrease around the meander
307 bend, with shear stresses in Transect 1 being relatively greater (depth-average τ
308 $= 0.06 \text{ m}^2 \text{ s}^{-2}$) than those observed in Transect 5 (depth-average $\tau = 0.03 \text{ m}^2 \text{ s}^{-2}$).
309 All five transects show peak τ values close to the bed, with τ increasing with
310 depth. In Transect 1, peak τ values ($\tau = 0.24 \text{ m}^2 \text{ s}^{-2}$) are observed at $0.75B_t$ at
311 the bed. Higher up in the vertical profile, τ is consistent at $0.02 - 0.03 \text{ m}^2 \text{ s}^{-2}$ at
312 all distances from the bank. At the bank toe ($1B_t$) τ is consistently at $0.01 \text{ m}^2 \text{ s}^{-2}$,
313 displaying a marked reduction in magnitudes from those observed at $0.75B_t$.

314 At mid-flow discharges ($Q = 23 \text{ m}^3 \text{ s}^{-1}$) magnitudes of τ again increase as the
315 distance to the bank toe increases (Figure 8), but decrease around the meander
316 bend, with shear stresses in Transect 1 being greater (depth-average $\tau =$
317 $0.38 \text{ m}^2 \text{ s}^{-2}$) than those observed in Transect 5 (depth-average $\tau = 0.07 \text{ m}^2 \text{ s}^{-2}$).
318 Transects 2 - 5 show peak τ values close to the bed, with τ increasing with depth
319 (Figure 8). However, in Transect 1 peak τ are observed close to the water's
320 surface, and decrease towards the bed. In Transect 1, peak τ values ($\tau = 0.75$
321 $\text{ m}^2 \text{ s}^{-2}$) are observed at $0.1B_t$ at the water's surface, with values decreasing to
322 $0.22 \text{ m}^2 \text{ s}^{-2}$ at the bed. In transects 3-5 τ ranges from $0.05 - 0.2 \text{ m}^2 \text{ s}^{-2}$
323 consistently with higher values present at the bed and at greater distances
324 from the river bank.

325 At high discharges ($Q = 90 \text{ m}^3 \text{ s}^{-1}$) magnitudes of τ are more consistent
326 throughout the five transects, with no discernible decrease around the meander
327 bend (Figure 8) observable. Values are also higher than those recorded at low
328 and mid-flows. Values of τ at all transects increase towards the bed. τ is greatest

329 at $0.75B_t$ in Transect 1 (depth-averaged $\tau = 0.5 \text{ m}^2 \text{ s}^{-2}$), although maximum τ
330 values at Transect 1 are observed at $1B_t$ at the water's surface ($\tau = 1.6 \text{ m}^2 \text{ s}^{-2}$).
331 In Transect 5 the highest τ values are observed at $0.25B_t$ and $0.5B_t$ (depth-
332 averaged $\tau = 0.59 \text{ m}^2 \text{ s}^{-2}$ and $0.47 \text{ m}^2 \text{ s}^{-2}$, respectively), with values further
333 away from the bank considerably smaller ($0.75B_t \tau = 0.15 \text{ m}^2 \text{ s}^{-2}$ and $1B_t \tau =$
334 $0.09 \text{ m}^2 \text{ s}^{-2}$).

335

336 **Discussion**

337 Our results demonstrate that the patterns and magnitudes of turbulent flows,
338 Reynold shear stresses and the development of secondary outer-bank cells
339 (SOCs) change as discharge levels vary in a meander bend. We observe that the
340 magnitudes of all quantities increase as flow discharge increases. However,
341 spatial variations between locations around the meander bend are subdued as
342 flow discharges increase, resulting in more uniform flow dynamics throughout
343 the reach. At low flows the most upstream bend transects, Transects 1 and 2,
344 show a high relative strength of the secondary flow components (Figure 7) when
345 compared the three downstream transects. However, as discharges increase, the
346 relative strength of the near-bank secondary flow strength become closer in
347 magnitude. In fact at the highest discharges the furthest downstream transect,
348 Transect 5, displays the greatest normalised secondary flow strength (Figure 7).

349 This pattern is also reflected in the location of the high velocity flow core
350 (Figures 3, 4 and 5). Engel and Rhoads (2017) propose a conceptual model of
351 the velocity distribution (henceforth termed the ER17 model), and related
352 turbulence and shear stresses, through a meander bend based on results from a
353 single flow discharge. In the ER17 model the high velocity core progressively

354 moves towards the outer-bank and the bank toe through the meander. Our
355 results support this model for high discharges, where increased primary
356 velocities are observed close to the bank toe at the downstream extent of the
357 meander bend (Figure 6). However, at mid- and lower flow discharges, the
358 fastest flows near the outer bank were observed at the upstream extent of the
359 meander bend (Figure 6), where the highest secondary flow strength was also
360 observed (Figure 7). Given the ER17 model was also developed on data collected
361 at mid-flow conditions (Engel and Rhoads, 2017), this discrepancy is likely due
362 to the higher curvature of the meander bend in which data underpinning the
363 ER17 model was collected. Our results suggest for lower curvature bends the
364 ER17 model may only be appropriate at higher discharges, and that at low and
365 mid-flow levels where the high velocity cells tends to be shifted away from the
366 outer bank through the bend and the SOC strength is suppressed (Figure 6), the
367 ER17 model breaks down.

368 The deflection of the higher velocity flow cell at low and mid-flows may be a
369 result of the presence of failed material at the bank toe (Hackney et al., 2015;
370 Parker et al., 2011; Wood et al., 2008; Figure 1B). The ER17 model depicts this
371 deflection in its final stage, showing the high velocity core moving back towards
372 the channel centre. It is possible that for our low and mid-flow data, slump
373 blocks at the bank toe are a primary control on the flow in the NBZ, whilst at
374 high discharges their influence is subsumed by the high flow levels, and their
375 submergence alters the direction of their influence, deflecting flow up and onto
376 the outer bank (Hackney et al., 2015).

377 We also observe that when SOCs are present at high discharges (in Transects 1,
378 3 and 4), TKE values are also typically higher than in transects where no SOCs
379 are observed (Figure 8). This pattern is reversed at mid-flows where the

380 presence of SOCs (in Transects 2, 3, 4 and 5) are associated with lower values
381 of TKE than in Transect 1 where no SOC is present (Figure 8). At low flows TKE
382 values are consistent throughout each transect regardless of the presence of
383 SOCs. This again shows a clear and critical dependence on discharge levels. Prior
384 work has demonstrated that the presence of SOCs can increase turbulence levels
385 (Sukhodolov, 2012), whilst experimental work (Blanckaert et al., 2012) suggests
386 that SOCs are associated with reduced TKE. Both Sukhodolov (2012) and
387 Blanckaert et al. (2012) report observations in bends at a single discharge. In
388 comparison, our results across a range of discharges suggest that the
389 relationship between TKE and the presence of SOCs is discharge dependent, and
390 thus more nuanced than single-discharge studies have previously reported. This
391 has a range of implications for bank erosion processes through relative controls
392 on the distributions of the greatest magnitudes of outer bank shear.

393

394 **Conclusion**

395 This paper reports observations of three-dimensional flow characteristics in the
396 near-bank zone of a meander bend at a range of discharge levels; low ($9 \text{ m}^3 \text{ s}^{-1}$)
397 1), mid- ($23 \text{ m}^3 \text{ s}^{-1}$) and bankfull ($90 \text{ m}^3 \text{ s}^{-1}$). Through the application of high-
398 resolution ADCP data primary and secondary flow structures were described and
399 characteristics of turbulent kinetic energy (TKE) and Reynolds shear stresses
400 were decomposed. Our results reveal that the relationship between the presence
401 of a secondary outer-bank flow cell (SOC) and magnitudes of TKE and shear
402 stresses vary with discharge. At high flows, the presence of a SOC is related to
403 increased TKE, whilst at mid-flow discharges SOCs are associated with reduced
404 TKE compared to transects where SOCs are absent. At low flows there is no

405 observed pattern between TKE and the presence/absence of a SOC. We attribute
406 these patterns to the role that failed material at the bank toe plays in deflecting
407 and modulating near-bank flows. This work highlights how important detailed
408 flow data across multiple discharges is in terms of understanding flow through
409 meander bends and thus the impact these flow processes will have on longer
410 term bend morphodynamics.

411

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418 **Data availability statement**

419 All data will be made available on request to the corresponding author.

420

421 **Conflict of interest disclosure**

422 The authors declare no conflicts of interest.

423

424

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