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1 **Impact of stress regime change on the permeability of a naturally fractured carbonate**
2 **buildup (Latemar, The Dolomites, Northern Italy)**

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29 **Abstract**

30 Changing stress regimes control fracture network geometry and influence porosity and
31 permeability in carbonate reservoirs. We investigate the impact of stress-regime change on
32 fracture network permeability utilizing outcrop data analysis and a displacement-based linear
33 elastic finite element method. The model is based on fracture networks, specifically, fracture
34 sub-structures. The Latemar, predominantly affected by subsidence deformation and Alpine
35 compression, is taken as an outcrop analogue for isolated (Mesozoic) carbonate buildups with
36 fracture-dominated permeability. We apply a novel strategy involving two compressive
37 boundary loading conditions, constrained by the NW-SE and N-S stress directions in the study
38 area. Stress-dependent heterogeneous apertures and effective permeability were computed by:
39 (i) using the local stress state within the fracture sub-structure and (ii) running a single-phase
40 flow analysis considering the fracture apertures in each fracture sub-structure. Our results show
41 that the impact of the modelled far-field stresses at: (i) subsidence deformation from the NW-
42 SE, and (ii) Alpine deformation from N-S, increased the overall fracture aperture and
43 permeability. In each case, increasing permeability is associated with open fractures parallel to
44 the orientation of the loading stages and with fracture densities. The anisotropy of permeability
45 is increased by the density and connectedness of the fracture network and affected by of shear
46 dilation. The two far-field stresses simultaneously acting within the selected fracture sub-
47 structure at a different magnitude and orientation do not necessarily cancel out each other in
48 the mechanical deformation modelling. These stresses effect the overall aperture and
49 permeability distributions. These effects, which may be ignored in simpler stress-dependent
50 permeability, can result in significant inaccuracies in permeability estimation.

51 **1. Introduction**

52 Naturally fractured reservoirs accommodate a significant share of the world's hydrocarbon
53 resources, especially in carbonates that contain between 50 – 60% of oil and gas reserves
54 worldwide ([Garland et al., 2012](#)). These reservoirs also play an essential role in the transition
55 to a low carbon energy future, especially in producing low- and high-enthalpy geothermal heat
56 or sequestering CO₂ ([McNamara et al., 2015](#)). However, these potentials have not been fully
57 explored partly because of the challenge in predicting and quantifying the contribution of fluid
58 flow through fractures in naturally occurring complex fracture networks ([Berkowitz, 2002](#);
59 [Narr et al., 2006](#)). The challenge is primarily due to the sub-seismic heterogeneous
60 characteristics of fractures, partially studied from core data or image logs ([Laubach, 2003](#)), and

61 the lack of an understanding of the structural arrangement of the fracture networks and their 3-
62 D distribution and geometrical attributes. Given that core data or image log (capturing small-
63 scale or local information around a well) lacks information on the spatial arrangement
64 (Bourbiaux et al. 2005), outcrop analogues are often used for explicit descriptions of fracture
65 distributions, including length, orientation, spacing and aperture (Agosta et al., 2010; Bonnet
66 et al., 2001; Hooker et al., 2013; Igbokwe et al., 2018, 2020; Wilson et al., 2011). Integrated
67 and detailed outcrop studies provide constraints for understanding most fracture geometrical
68 parameters because analogues have adequate resolution over a varied length scale of $10^{-1} - 10^4$
69 m, even in the 3-D (Rotevatn et al., 2009; Igbokwe et al., 2020). Also, outcrop analogues can
70 be extrapolated to reservoir scale fracture models based on geomechanical relations as long as
71 the data are accurately captured and corrected for sampling artefacts (Bisdom et al., 2016; Narr
72 et al., 2006).

73 In many natural reservoirs, the fractures act as the principal pathways for fluid or gas flow,
74 particularly in low matrix permeability rocks. Fracture aperture is one of the main factors
75 controlling fracture flow, as the aperture delineates fracture porosity and permeability (Bisdom
76 et al., 2016; Hooker et al., 2013). Minor variations in aperture have enormous implications on
77 rock flow and transport properties (de Dreuzy et al., 2002; Matthäi, 2003; Matthäi & Belayneh,
78 2004), and obtaining a realistic aperture prediction from outcropping geometries is challenging.
79 At depth, in-situ stresses partly control aperture, which influences permeability by either
80 increasing or decreasing their order of magnitude (Lei et al., 2015, 2017; Zoback, 2007), but
81 pressure relief during exhumation and weathering effects dissolve occluded fractures and
82 change the aperture. Thus, the outcropping aperture cannot be considered representative,
83 except when these apertures are generated from veins that have not been reactivated during
84 exhumation (e.g., Hooker et al., 2013, 2014). Having said that, preserved veins covering a large
85 outcrop scale equivalent to a reservoir scale are rarely found in nature.

86 As an alternative, fracture aperture is modelled as a function of stress (shear stress-induced
87 dilations), using hydromechanical coupling based on linear elastic fracture mechanics (Bisdom
88 et al., 2017; Lei et al., 2015; Min et al., 2004). These models require the local stress state,
89 typically derived from Finite Element (FE) models with explicit fracture representations
90 (Bisdom et al., 2016; Lei et al., 2017). The local state of stress can change the hydraulic
91 properties and fluid pressure and vice versa in a rock domain. For stress-induced changes in
92 hydraulic properties, permeability can be several orders of magnitude and irreversible under
93 perturbations resulting from various natural activities. These activities can cause stress
94 redistribution, such as in geothermal energy and oil/gas reservoir production, where injections

95 and extractions of fluids demand meaningful change in effective stresses underground (Min et
96 al., 2004). Thus, investigating the impact of stress change on permeability becomes
97 fundamental in understanding the reservoirs' overall aperture distribution and flow pattern.

98 The effect of stress on the permeability of fractured rocks has been widely investigated using
99 synthetic fracture networks and 2-D fracture network models. For example, the level of
100 differential stress considerably impacted the magnitude and direction of rock mass permeability
101 when the stress effects on the 2-D permeability tensor of three sampled natural fracture
102 networks were analyzed (Zhang & Sanderson, 1996). Min et al. (2004) observed significant
103 stress-induced flow enhancement along with connected shear fractures in the study of stress-
104 dependency of rock mass permeability with the effects of non-linear joint normal deformation
105 and shear dilation. While Bisdorn et al. (2016) investigated the influence of in-situ stress on
106 the permeability of an outcrop-based fracture system, Lei et al. (2014) analyzed the stress effect
107 on the validity of synthetic fracture networks for representing a naturally fractured rock in
108 terms of geomechanical and hydraulic properties.

109 These previous studies mainly considered the tectonic stress perturbation based on a single
110 stress regime that differently scrutinizes the magnitude and orientation of principal stresses on
111 a rock body. However, past studies did not necessarily consider the different tectonic episodes
112 that build up different stress regimes, and which may change in geological time. In nature, the
113 stress in a rock body can exist in two forms (i) the far-field stress impacting the rock from
114 outside the body and (ii) the local state of stress domiciled inside the rock body. The far-field
115 stress is of tectonic origin, or related to lateral pressure changes affected by the lateral thickness
116 and density variations (Pascal & Cloetingh, 2009). At the same time, the local state of stress is
117 characterized by rotations and changes in principal stress orientations, magnitude, and so on,
118 all taking place inside the rock body. The local state of stress changes is linked to the impact
119 of far-field stresses and geometry, distribution and density of fracture network characteristics.
120 Therefore, investigating stress perturbation and its effects on the permeability of rock bodies
121 need a comprehensive approach that will capture the impact of the magnitude and orientation
122 of the far-field stresses and the associated changes in the local state of stress in different tectonic
123 episodes.

124 The difficulty is primarily representing: (i) the stress-regime changes over time, (ii) the
125 complex fracture system geometry, having various orientations and lengths, and (iii) the
126 complex mechanical deformation mechanism, influencing the interactions between individual
127 fractures in a FE mechanical model. Until now, a focused study dealing with the superposition
128 of different magnitude and direction of the far-field stresses, representing the major geological

129 tectonic episodes in a given area, has not been well-represented in several FE mechanical
130 models. As such, the overall impact of stress-regime change on permeability remains poorly
131 understood.

132 In this study specifically, we investigate the influence of the superposition of two orthogonal
133 far-field stresses, derived from significant tectonic episodes, on the permeability of the
134 carbonate rocks with different fracture distributions. Natural fracture geometry with multiple
135 fracture sets and intersections from an outcrop analogue at the Latemar carbonate platform was
136 utilized. The two major tectonic events that occurred at Latemar are associated with
137 subsidence-related deformation in Late Triassic and Early Jurassic times, shortly after the
138 fractures were formed, and later Alpine compression during the Neogene. Although new
139 fractures may have formed during these tectonic episodes, we assumed that new fractures did
140 not form and/or grow in the model used in this study. Instead, already developed fractures were
141 either opened and/or closed during these tectonic events. This enables us to study stress regime
142 change effects on permeability, focusing on those issues related to the complexity of multiple
143 fractures. The development of the Latemar fractures and their driving factors, including their
144 geometries, connectivities etc., is beyond this paper's scope and has been discussed in detail in
145 [Igbokwe et al. \(2022\)](#). Our goal is to draw general conclusions about the overall impact of
146 stress regime change on permeability in complex fractured systems representing an analogue
147 of the subsurface reservoirs.

148 The main objectives of this paper are to (i) analyze the selected fracture network
149 characteristics of the study area, to (ii) evaluate the impact of stress regime change, considering
150 the overall tectonic episodes, to (iii) compute the stress-induced fracture aperture in a FE
151 model, and to (iv) evaluate the changes in the effective permeability and permeability
152 anisotropy. The implication of the central assumptions and the impact of the changing stress
153 orientation and magnitude on effective permeability are discussed.

154 **2. Geological background and study area**

155 The Latemar carbonate platform is one of the pre-volcanic Middle Triassic isolated carbonate
156 platforms ([Goldhammer & Harris, 1989](#); [Preto et al., 2011](#)) located in the southwestern part of
157 the Dolomite Mountain belt (Northern Italy; Fig. 1), and neighboured by the Catinaccio and
158 the Agnello platforms in the north and south, respectively (Fig. 1a). The outcrops of the
159 Latemar are built predominantly by the Sciliar (Schlern) Formation (Fig. 1b), underlying the
160 Contrin Formation, which is a regionally important carbonate bank in the Dolomite Mountain

161 belt (Gaetani et al., 1981; Jacquemyn et al., 2014). Between the Late Anisian and Late Ladinian
162 (Middle Triassic), the Latemar platform formed on topographic highs. Their buildups were
163 separated by basinal areas where siliceous, basinal carbonates were deposited (Fig. 1; Bosellini,
164 1984).

165 The Latemar has a maximum altitude of 2850 m, with high peaks exposing the platform
166 margin, the slope and the interior (Fig. 1c, d). While the preserved portions of the platform
167 margin consist of reefal boundstones, microbial crust, and marine cement-facies, the slope is
168 characterized by massive breccia flows, including coarse and matrix-poor materials derived
169 from the platform margin or platform interior (Egenhoff et al., 1999; Emmerich et al., 2005;
170 Goldhammer & Harris, 1989; Harris, 1994; Marangon et al., 2011). The platform interior is
171 arranged in dm's-to m's-scale shallowing upward cycles (Christ et al., 2012; Goldhammer et
172 al., 1990), consisting of up to 750 m successions of subtidal and peritidal carbonate lagoonal
173 deposits. The Latemar platform was partly dolomitized as a consequence of fluid mobilization
174 triggered by the activity of the Predazzo Volcanic-Plutonic Complex in the Middle Triassic. A
175 recent review on the dolomitization and diagenesis of the Latemar platform is given by Mueller
176 et al. (2021).

177 The considered outcrop in the presented study area is a large fractured pavement in the
178 platform interior (Fig. 1c, d). The studied pavement (*ca.* 7.5×10^2 m²) consists of limestone
179 and dolostone rock bodies, showing a gentle dip of fewer than 5° towards the north. Most
180 structural features observed on the fractured pavements are fractures, veins and stylolites, some
181 of which are weathered.

182 2.1. Tectonic setting

183 The Latemar forms part of the Southern Alps, part of the Mesozoic Adriatic plate,
184 predominantly thrusting southward during the Alpine collision (e.g., Boro et al., 2013;
185 Doglioni, 1988). Deposition in the Latemar began on a structural high (horst) generated by
186 extensional tectonics, breaking up the widespread regional carbonate bank (Contrin Fm.).
187 Subsequently, subsidence deformation and extensional synsedimentary tectonics controlled the
188 geometry of the platform, leading to faulting. Several fractures and/or faults crosscut the
189 Latemar platform, formed in conjugate pairs and are related to Middle Triassic subsidence
190 deformation. Besides, Preto et al. (2011) documented the ENE-WSW and WNW-ESE faults
191 linked to the Triassic trans-tensional regime as the oldest fault direction in the Latemar. The
192 outcome is a platform with a horseshoe shape and intraplatform basins (Fig. 1a, c; Preto et al.,

193 2011). Dextral strike-slip reactivation, observed along magmatic dikes, reflects Neogene
194 Alpine compressional tectonics in the Latemar. Before the Neogene compressional tectonics,
195 a regional magmatic-tectonic event in the Late Ladinian to Early Carnian triggered intense
196 magmatic activity. These generated the intrusion of the Predazzo Volcanic-Plutonic Complex
197 and the Mt. Monzoni, which are associated with temporarily halting carbonate deposition at
198 Latemar (Bellieni et al., 2010; Bosellini, 1984; Bosellini et al., 2003).

199 In the Latemar platform, two principal far-field stresses did affect the carbonate deposits,
200 representing the two main phases of deformation, namely: NW-SSE (Middle Triassic
201 extensional (subsidence) tectonics) and the N-S (Alpine compressional tectonics) (Boro et al.,
202 2013; Hardebol et al., 2015; Jacquemyn et al., 2015; Preto et al., 2011).

203 3. Methodology

204 The methodology applied includes a three phase workflow with (i) drone image acquisition
205 and outcrop interpretation and digitization (Fig. 2a), (ii) meshing and geomechanical finite
206 element aperture modelling and calculations (Fig. 2b), and (iii) fluid-flow modelling and
207 effective permeability calculations for uncoupled hydromechanical conditions.

208 3.1. Outcrop acquisition and digitization

209 Fracture network patterns from the outcropping carbonate rocks in the sub-horizontal
210 pavements of the flat-topped Latemar interior platform were acquired using drone imagery (DJI
211 Phantom 4®). The acquired images were processed using Agisoft Metashape® and converted
212 into georeferenced digital outcrop models using photogrammetry.

213 The observed fractures were interpreted and digitized using ArcGIS 10.5™ software, where
214 fractures were traced and digitized with a polyline interpretation tool. Structural data such as
215 length, orientation and spacing were computed for each polyline. The high quality of the drone
216 images allows interpretations of thousands of potential fractures. In total, more than 2000
217 fractures were documented.

218 Although the drone image covers most areas with a relatively high resolution, these data
219 sets still include a limited number of sampling and truncation artefacts. Additionally, drone
220 images were used to guide detailed fieldwork on the ground to document minute structural
221 details that were only in part visible in drone images. Aside from the sub-horizontal outcrops,
222 bedding perpendicular outcrop stations, ranging from a few m's to 10's meters, were also

223 studied in the nearby Valsorda Valley. This provides information that enabled the
224 documentation of the paleostress fields affecting the entire buildup.

225 Within the 2-D digitized fracture network of the sub-horizontal outcrop, five representative
226 fracture network areas, referred to as fracture sub-structures (FSS), were selected and used to
227 model stress heterogeneity and permeability. The dimensions of the individual FSS are
228 approximately 2 x 2 m. For each FSS, the fracture length distributions and frequencies were
229 fetched from the digitized fractures and plotted in a histogram chart. The FSS areas are located
230 in the same structural domain, but they display fracture networks with different spatial
231 distributions. Further, fracture networks were commonly sub-vertical, splaying, curving,
232 intersecting and tipping adjacent to other fractures. Locally, the fractures are orthogonal and/or
233 in a conjugate pattern with a small conjugate angle. Slight modifications and/or extrapolations
234 of the fracture's original pattern were implemented to maintain the fracture topological
235 connectivity.

236 **3.2. Stress and permeability modelling**

237 The acquired 2-D fracture network was simplified and meshed to model the impact of stress
238 regime change and fracture topology on the permeability of the fractured carbonate rock, using
239 selected FSS (FSS 1 through FSS 5) in the manner described below.

240 First, the selected digitized fracture geometry was converted to 2-D 'element geometry' and
241 meshed into a triangular mesh using GiD¹ (a geometric modelling and mesh generation
242 software for pre-and post-processing). The GiD converts the vector images into meshes using
243 triangular elements, and fractures were specified as interface elements (Fig. 2b; Phase 2.1). A
244 complete geometrical topology was ensured by defining additional interface elements not
245 corresponding to the fractures. This provides an accurate representation of fracture connectivity
246 and topology, which is essential, especially when the matrix of the fractured rock is close
247 impermeable (Hardebol et al., 2015; Sanderson & Nixon, 2015). Further, the mesh was refined
248 to avoid singularities issues to capture complex geometries where high variations were
249 expected, especially around the fracture tips and corners. Finally, these meshes were read
250 directly into the FE simulator to analyze the stress distribution in a complex fracture network.

251 Second, 2-D linear-elastic mechanical finite element analyses were conducted using
252 KRATOS open-source geomechanical software. The 2-D linear-elastic mechanical modelling

¹ GiD is a universal, adaptive and user-friendly **pre** and **post processor** for numerical simulations in science and engineering.
<https://www.gidhome.com/> (Accessed May 24, 2022)

253 was achieved by applying loads and/or displacement boundary conditions to the models (Fig.
254 [2b](#); Phase 2.2). The applied boundary conditions approximate the impact of the two tectonic
255 episodes (far-field stresses) within the study area that affected the complex fracture network
256 shortly after the fractures were formed. These tectonic episodes are the subsidence-related
257 deformation, having a sub-horizontal NW-SE shortening direction and the later Alpine
258 deformation with a N-S compression direction. We assume their maximum magnitudes as 50
259 and 160 MPa for subsidence-related- and Alpine deformations, respectively. A 2-D plane strain
260 linear elastic isotropic matrix was assumed for the presented models. The 2-D plane strain was
261 used in order to treat all modelled fracture planes equally, especially on the z -axis.

262 The stress distributions were analyzed using continuum triangular elements and interface
263 elements, corresponding to the rock matrix and fractures. The effects of the far-field stresses
264 on the overall model and the relative displacement of the deformed and undeformed element
265 geometries were stimulated. Given that all FSS are sub-horizontal, the origin of the x and y -
266 axis is placed at the centre of the plane. In other words, a homogenous Dirichlet boundary
267 condition was applied at the centre of the FSS, i.e., assuming the centre to be fixed in either x -
268 or y -axis so that translation and rotation are prevented. After the specified Dirichlet boundary,
269 a sequential Neumann boundary (applied loading) condition followed. The Neumann boundary
270 application utilized the direction and magnitude of the two major tectonic (stress) episodes in
271 the study area, on each selected FSS. The loading of the first tectonic episode, subsidence
272 deformation, represented as first stage loading, was applied from the NW-SE direction and
273 increased in discrete steps until a maximum magnitude of 50 MPa was reached. After that, the
274 loading of the second tectonic stage, Alpine compression and known as second stage loading,
275 was added (or applied) from the N-S direction, while the compressive loading corresponding
276 to the first stage was active and/or maintained at 50 MPa. Analogous to stage 1 loading, the
277 load values were increased in discrete steps until a maximum value of 160 MPa was attained
278 (Fig. [2b](#); Phase 2.2.1). The applied boundary loading is analogous to in-situ stresses, which are
279 feasible magnitudes for modelling compressive settings ([Heidbach et al., 2018](#)). The essential
280 model parameters are listed in Table 1. The modelled local state stresses and slips were used
281 to calculate the local stress-induced aperture and their distributions, even as individual fractures
282 open differently.

283 Third, the permeability was calculated by conducting a single-phase flow simulation in two
284 perpendicular directions, using a computational homogenization ([Leonhart et al., 2017](#)) for an
285 unperturbed and a perturbed fracture state. The permeability tensors of the deformed carbonates
286 were obtained. We simulated (i) uncoupled hydromechanical modelling scenarios were

287 simulated, (ii) applied fluid pressure as a surface pressure to the sidewall of the FSS, (iii) we
288 run simulations using linear FE solver, and (iv) post-processed outputs the use in flow
289 simulations.

290 **3.3. Stress-dependent aperture**

291 Stress-dependent apertures describe the opening of fracture planes, as compressive loads are
292 applied within a fracture system. In the geomechanical FE model (Fig. 2b; Phase 2.3), stress-
293 dependent apertures were calculated from the shear stresses. As represented in the model, the
294 shear stresses resulting from the impact of the two primary loading conditions and changes in
295 apertures were documented within each loading scenario.

296 In the model, the interface elements were inserted in sections, which are not considered as
297 fractures. However, these interface elements are closed, behaving like the rock matrix, and
298 were inserted into the simulation purely for computational reasons. Interface elements
299 corresponding to the fractures obtained from the digitizing procedure were allowed to only
300 open and/or close. The initial aperture size of these interface elements was assumed to be zero.
301 The interfaces corresponding to the rock matrix were filtered out of the aperture dataset because
302 the 'aperture' values corresponding to these interfaces are always zero. Since we did not allow
303 new fractures to form, the interface elements were not activated, thus maintaining zero aperture
304 values.

305 **4. Finite Element Analysis (FEA)**

306 A displacement-based linear elastic finite element method (FEM) was used to solve linear
307 poroelasticity equations over the domain of interest. In this section, the fractures and the rock
308 matrix are considered in the uncoupled hydromechanical numerical modelling. The fractures
309 are regarded as having zero-thickness interface elements embedded into the rock space, as
310 shown in figure 3a.

311 The balanced equation for the porous medium according to the general Biot's theory can be
312 written

$$313 \quad \nabla \cdot \boldsymbol{\sigma} + \rho \mathbf{b} - \rho \ddot{\mathbf{u}} = 0 \quad (1)$$

314 where $\boldsymbol{\sigma}$ is the total stress tensor, \mathbf{b} is the body force, $\ddot{\mathbf{u}}$ is the acceleration vector of the solid
315 and ρ is the density of the porous medium.

316 The saturated single-phase flow in both the fractures and rock matrix obeys Darcy's law and
 317 can be calculated based on a mass conservation equation:

$$318 \quad \frac{\partial}{\partial t}(\phi^\tau \rho_w) + \nabla \cdot (\rho_w \dot{\mathbf{w}}_m) = \rho_w q_s$$

319 where τ is a sign expressing the different media in the model domain, for example, $\tau = m$
 320 expressing a matrix and $\tau = f$ expressing a fracture, where ϕ^m is the porosity of the rock matrix,
 321 and ϕ^f is the porosity of a fracture, ρ_w is the density of the fluid, q_s is the source term, $\varepsilon_{vol} = \nabla \cdot$
 322 \mathbf{u} denotes the velocity of the solid matrix and $\dot{\mathbf{w}}_m$ is the Darcy velocity vector of the pore fluid,
 323 which can be written as:

$$324 \quad \dot{\mathbf{w}}_m = \frac{1}{\mu} \mathbf{k}^\tau \cdot \nabla p \quad (3)$$

326 where \mathbf{k}_τ is the intrinsic permeability tensor for rock matrix and fracture, which is replaced by
 327 a scalar value k for the isotropic medium, and μ is the fluid viscosity. According to the cubic
 328 law, the intrinsic fracture permeability can be calculated as

$$329 \quad k^f = \frac{w^2}{12} \quad (4)$$

332 where w is the fracture aperture.

333 4.1. Governing equations

334 A two-dimensional porous media domain Ω bounded by the boundary Γ is necessarily
 335 considered to obtain the governing equations. As depicted in figure 3b, a fluid-filled
 336 geomechanical discontinuity Γ_d is contained in the porous medium.

337 The essential boundary conditions are imposed on the external boundary as the prescribed
 338 primary variables:

$$339 \quad \begin{aligned} \mathbf{u} &= \mathbf{u}^- & \text{on } \Gamma_u \\ p &= \bar{p} & \text{on } \Gamma_p \end{aligned} \quad (5)$$

341 and the natural boundary conditions are imposed on the external boundary as the prescribed
 342 traction and volume flux:

$$343 \quad \begin{aligned} \boldsymbol{\sigma} \cdot \mathbf{n}_\Gamma &= \bar{\mathbf{t}} & \text{on } \Gamma_t \\ \dot{\mathbf{W}} \cdot \mathbf{n}_\Gamma &= \bar{q} & \text{on } \Gamma_q \end{aligned} \quad (6)$$

345

346 where $\bar{\mathbf{t}}$ is the prescribed traction applied on the boundary Γ_t ; \bar{q} is the prescribed flux imposed
 347 on the permeable boundary Γ_q , and \mathbf{n}_Γ is the unit outward normal vector to the external
 348 boundary.

349 4.2. Discretization model

350 This section presents the computational framework of the numerical modelling and the method
 351 to estimate the effective permeability of fractured rock medium. It is essential to note that
 352 stress-dependent permeability of a fractured rock domain is governed by dilatation of the
 353 fractures. In this model, the coupling problem includes two parts: the hydraulic solver, which
 354 solves the fluid pressure during the fluid process, and the mechanical solver, which calculates
 355 the deformation of the rock matrix and evaluates the fracture open and closure.

356 4.2.1. Fluid discretization

357 The fluid flows through the same 'conduit systems' in the fluid solver, which can be regarded
 358 as matrix conduits and fracture conduits. As shown in figure 4, the interface element is a 6-
 359 node quadrangle. The fluid flows between the mid-plane nodes of the interface element (see
 360 node 5 and node 6 in Fig. 4a), which can be seen as the flow fracture (conduit). The degree of
 361 freedom (dof) of fluid pressure is added to node 5 and node 6.

362 The equations for fluid flow through the control volume of a node are derived by formal
 363 integration methods, such as the finite volume method (FVM), expressed as follows:

$$364 \int_V \frac{\partial \rho_w \phi^\tau}{\partial t} dV + \int_A \mathbf{n} \cdot (\rho_w \mathbf{w}_m) = \int_V \rho_w q_s dV \quad (7)$$

365 In this model, the 2D fracture segment can further be simplified as a 1D line called fracture
 366 conduit, denoted with $fp(i,j)$, which is associated with two end nodes i and j , as shown in figure
 367 4b. Each node represents a half volume of the fracture ($wl/2$). If the fluid pressures are known
 368 in the end nodes, the flow rate in the fracture can be evaluated by the difference equation:

$$369 Q_{fij} = K_{ij}^f (p_i - p_j) \quad (8)$$

370 where K_{ij}^f is an equivalent conductance coefficient of the fracture conduit, which can be
 371 calculated by:

$$372 K_{ij}^f = \frac{w^3}{12\mu l_{ij}} \quad (9)$$

373 For an effective simulation of fluid flow in a porous medium, the porous medium is
 374 reconstructed using an equivalent fracture network graph. The volume of the porous medium
 375 is discretized and represented by nodes. The equivalent hydraulic parameters of the fractures
 376 are derived based on the unstructured triangular mesh.

377 For the derivation of the parameters of fractures, forming a triangular mesh, the
 378 circumcenter o of the triangle is selected as its division point (Fig. 4c). The line linking the
 379 circumcenter and the mid-point of each edge is perpendicular to the edge, and these
 380 perpendicular bisectors divide the triangle into three parts. The equivalent conductance
 381 coefficient of the matrix conduit $mp(i,j)$ is derived from unstructured triangular elements, as
 382 shown in figure 4c. The volume of each fracture node is half the volume of the triangle, which
 383 is composed of the two fracture nodes (node i and j) and the circumcenter (node O).

384 In the porous medium, when the fluid is transferred (between node i and j), the fluid flows
 385 between domain $ogif$ and domain $oejf$ through their common face of . Thus, the flow rate Q_{ij} in
 386 the fracture $mp(i,j)$ is equal to the flow rate

387 Q_{of} .

$$388 \quad Q_{mij} = K_{ijm}(p_i - p_j) \quad (10)$$

$$389 \quad Q_{of} = -\frac{k}{\mu} \int_{of} \nabla p \cdot \mathbf{n}_{of} dl = \frac{kl_{of}}{\mu l_{ij}} (p_i - p_j) \quad (11)$$

390 Thus, the equivalent conductance coefficient K_{ij}^m can be calculated by:

$$391 \quad K_{ij}^m = \frac{kl_{of}}{\mu l_{ij}} \quad (12)$$

392 The equivalent conductance coefficient of flow fracture is the sum of the fracture
 393 conductance coefficient K^f and matrix conduit conductance coefficient K^m , which can be
 394 expressed as:

$$395 \quad K_c = K^f + K^m \quad (13)$$

396 For each node i , the governing equation for single-phase saturated flow in the rock matrix
 397 and fractures are expressed as:

$$398 \quad \frac{\partial \rho_w \phi^\tau V_i^\tau}{\partial t} + \rho_w \sum_{j=1}^{n_i} K_{ij}^\tau (P_i - P_j) = \rho_w Q_{s_i} \quad (14)$$

399 4.2.2. Mechanical discretization

400 In the mechanical solver, the fluid pressure on the fracture surface is governed by

$$401 \quad p_{\Gamma d^-} = p = p_{\Gamma d^+} \quad (15)$$

402 Thus, the pressure on node 1 and node 3 is equal to that on node 5, and the pressure on node 2
403 and node 4 is equal to that on node 6 (Fig. 4a).

404 The weak form of the equilibrium equation for the porous fractured medium is given as:

$$405 \quad \int_{\Omega} (\boldsymbol{\sigma}' - \alpha \mathbf{m} P) : \nabla \delta \mathbf{u} \, d\Omega + \int_{\Gamma_d} [[\delta \mathbf{u}]] (\mathbf{t}_d - P \mathbf{n}_{\Gamma_d}) \, d\Gamma_d = \int_{\Gamma_t} \delta \mathbf{u} \cdot \bar{\mathbf{t}} \, d\Gamma_t + \int_{\Omega} \delta \mathbf{u} \cdot \mathbf{b} \, d\Omega \quad (16)$$

408

409 The discrete displacement field and the pore pressure field in the rock matrix are approximated
410 by:

$$411 \quad \mathbf{u} = \mathbf{N}_u \mathbf{U} \quad (17)$$

412

$$413 \quad p = \mathbf{N}_p \mathbf{P}$$

414 where \mathbf{U} and \mathbf{P} are the nodal displacement and pressure; \mathbf{N}_u is the matrix of shape functions,
415 and \mathbf{N}_p is the row vector of shape functions. It is assumed that the same shape functions adopted
416 for both \mathbf{u} and p are reasonable.

$$417 \quad \int_{\Omega} (\boldsymbol{\sigma}' - \alpha \mathbf{m} P) : \nabla \delta \mathbf{u} \, d\Omega + \int_{\Gamma_d} [[\delta \mathbf{u}]] (\mathbf{t}_d - P \mathbf{n}_{\Gamma_d}) \, d\Gamma_d = \int_{\Gamma_t} \delta \mathbf{u} \cdot \bar{\mathbf{t}} \, d\Gamma_t + \int_{\Omega} \delta \mathbf{u} \cdot \mathbf{b} \, d\Omega \quad (18)$$

418 The displacement jump across the fracture and the pressure field in the fracture can be
419 expressed as:

$$420 \quad [[\mathbf{u}]] = \mathbf{R} \mathbf{N}_{int} \mathbf{U}_{int}$$

$$421 \quad p_f = \mathbf{N}_f \mathbf{P}_f$$

$$422 \quad (19)$$

423 where \mathbf{R} is the rotation matrix; \mathbf{N}_{int} is the matrix of shape functions in the interface element and
424 \mathbf{N}_f is the row vector of shape functions in the interface element.

425 According to the Eq.16, the discrete equation for the mechanical part is given as:

$$426 \quad \int_{\Omega} \mathbf{B}_u^T \cdot \boldsymbol{\sigma}' \, d\Omega - \mathbf{C}_{uw} \mathbf{P} + \int_{\Gamma_d} \mathbf{N}_{int}^T \mathbf{R}^T \mathbf{t}_d \, d\Gamma - \mathbf{C}_{int} \mathbf{P} = \int_{\Gamma_t} \mathbf{N}_u^T \bar{\mathbf{t}} \, d\Gamma + \int_{\Omega} \mathbf{N}_u^T \mathbf{b} \, d\Omega \quad (20)$$

427 where \mathbf{B}_u is the strain-displacement ($\boldsymbol{\varepsilon} - \mathbf{u}$) matrix and it can be expressed as:

$$428 \quad \boldsymbol{\varepsilon}_u = \mathbf{B}_u \mathbf{u} \quad (21)$$

429 the matrices \mathbf{C}_{uw} and \mathbf{C}_{int} are given by

$$\mathbf{C}^{uw} = \int_{\Omega} \alpha \mathbf{B}_u^T \mathbf{m} \mathbf{N}_p d\Omega \quad (22)$$

$$\mathbf{C}_{int} = \int_{\Gamma_d} \mathbf{N}^T \mathbf{t} \mathbf{R}^T \mathbf{m}_2 \mathbf{N}_f d\Gamma \quad (23)$$

433 Γ_d

434 where $\mathbf{m} = \{1 \ 1 \ 0\}^T$ and $\mathbf{m}_2 = \{1 \ 0\}^T$.

435 In this mechanical solver, the pressure acting on the fracture surface is regarded as an
436 external force. Thus, Eq.20 can be rewritten as:

$$\mathbf{K}\mathbf{U} = \mathbf{F}^{ext} \quad (24)$$

where \mathbf{K} is the stiffness matrix and \mathbf{F}^{ext} is the vector of external force:

$$\mathbf{K} = \mathbf{K}^{bulk} + \mathbf{K}^{int} \quad (25)$$

with

$$\mathbf{K}^{bulk} = \int_{\Omega} \mathbf{B}_u^T \mathbf{D} \mathbf{B}_u d\Omega \quad (26)$$

$$\mathbf{K}^{int} = \int_{\Gamma_d} \mathbf{N}_{int}^T \mathbf{R}^T \mathbf{T} \mathbf{R} \mathbf{N}_{int} d\Gamma \quad (27)$$

$$\mathbf{F}^{ext} = \int_{\Gamma_t} \mathbf{N}_u^T \bar{\mathbf{t}} d\Gamma + \int_{\Omega} \mathbf{N}_u^T \mathbf{b} d\Omega + \mathbf{C}_{uw} \mathbf{P} + \mathbf{C}_{int} \mathbf{P} \quad (28)$$

431 4.3. Effective permeability for fractured rock and aperture modelling

442 This section provides algorithms for the effective permeability k_{eff} of fractured rock. For a 2D
443 square domain with randomly distributed fractures, the effective permeability for different flow
444 directions k_{xx} and k_{yy} is different. k_{xx} is solved by applying pressure boundary conditions ($p_1 \neq$
445 p_2) at the left and right boundaries and zero fluid flow conditions ($q = 0$) at the top and bottom
446 boundaries, while applying pressure boundary conditions at the top and bottom boundaries and
447 zero fluid flow conditions at the right and left boundaries for k_{yy} . Having computed the fluid
448 flow q through the fractured rock domain through Equation.14, the effective permeability is
449 estimated as:

$$k_{eff} = \frac{\mu L}{(p_1 - p_2)W} \sum_{i=1}^m q_i \quad (29)$$

451 where m is the number of total points on the fluid injection boundary, L is the length of the
452 domain in the direction of fluid flow, and W is the width of the domain.

453 5. Numerical experiments on a single fracture impact on effective permeability

454 This experiment evaluates the behaviour of the effective permeability k_{xx} and k_{yy} under different
455 fracture aperture values for uncoupled hydromechanics processes. In essence, this experiment
456 aims to validate the consistency of the computed effective permeability values (results) with
457 varying aperture sizes in the models. For this reason, numerical experiments are simulated, in
458 this section in a selected rock domain with a single fracture, by choosing a range of intrinsic
459 matrix permeability and fracture apertures, such as 10^{-9} , 10^{-15} , and 10^{-21} m² and 0.001, 0.01,
460 0.1, 1, and 10 mm, respectively.

461 The matrix permeability values are drawn from Latemar mean platform permeabilities
462 estimated from forward-coupled modelling of the sedimentary and diagenetic evolution in
463 [Whitaker et al. \(2014\)](#). In contrast, the aperture sizes are arbitrarily chosen, reflecting a
464 logarithm range. The permeability k_{xx} corresponds to the permeability computed by applying a
465 pressure gradient in the horizontal direction. In contrast, the permeability k_{yy} corresponds to the
466 permeability calculated by applying a pressure gradient in the vertical direction. By
467 'uncoupled', we mean that fluid flow cannot cause the deformation of the rock matrix and does
468 not influence the opening and closure of fractures.

469 Constraining the simulation within the rock domain to a single fracture in the rock matrix,
470 [figure 5](#) shows the simulation of effective permeability under different inclination degrees for
471 a single fracture across the rock matrix, such as 0°, 45° and 90°.

472 It is observed that the existence of a single fracture has little influence on the effective
473 permeability of the rock domain when the permeability of the fracture is less than that of the
474 rock matrix. This means that the fracture aperture is small, i.e., when $w < \sqrt{12 k_m}$. Also, if
475 the fracture in the rock matrix is parallel to the x- and as well to the y-axis, k_{yy} and k_{xx} assume
476 constant values, the same as the intrinsic matrix permeability ([Fig. 5a – c](#)), respectively.

477 6. Results

478 The workflow presented in [figure 2](#) is applied to model the aperture and permeability
479 distributions through an outcropping network of fractures. Further, the two (quasi-static)
480 loading scenarios, representing the boundary loading conditions (or far-field stresses), are
481 highlighted. Extracting five selected outcrops (e.g., FSS1 – FSS5) from the fractured pavement,
482 outcrop models developed demonstrate in more detail the impact of the mechanical
483 deformation (loading), change in stress regimes and fracture geometry on modelled aperture

484 and effective permeability. Also, the flow anisotropy function in response to the loading
485 scenarios and orientation is evaluated.

486 **6.1. Field data and fracture network geometry analysis**

487 Field investigation of Latemar outcrops shows widespread brittle deformation features with
488 low strain barren fractures and veins as the dominant structures. These features are displayed
489 as mode I, conjugate hybrid fractures, and stylolites. By considering the prominent sub-vertical
490 outcrops exposed at the base of the Latemar (in the Valsorda Valley; Fig. 6) and sub-horizontal
491 (pavement) outcrops at the flat-topped Latemar buildup (Fig. 7), the arrangements, orientations
492 and the stress fields during the development of the fractures are documented.

493 In the Valsorda Valley (Fig. 6), carbonate outcrops are affected by minor reverse conjugate
494 faults dipping at low-angle ($< 30^\circ$) to bedding. These conjugate reverse faults strike between
495 *ca* 238° WSW-ENE and 250° SW-NE, accommodating low-angle SSE - and ENE dipping
496 fractures with a horizontal intersection. The movement of these faults and associated tectonic
497 stylolites correspond to NNW-SSE compression. On the other side, at the flat-topped Latemar,
498 on the sub-horizontal (pavement) outcrops, fractures also form conjugate patterns, exhibiting
499 dextral and sinistral displacements (Figs. 7 and 8). These fractures documented from the field
500 and drone images highlight the NNW-SSE, NE-SW and ENE-WSW orientations as the
501 dominant fracture set and record an approximately NE-SW compression (Fig. 8b). The
502 measured sub-vertical stylolites show the primary orientation of WNW-ESE and strike
503 perpendicular to the bisection planes of the observed conjugate systems (Figs. 6 and 7c).

504 Here, we focused on the sub-horizontal (pavement) outcrops, where figure 8b shows
505 embedded five representative FSS. Each FSS contains a sufficient number of fracture
506 heterogeneities to give a representative value of a property such as fracture densities, spacing,
507 connectivity etc. Geometrical analysis of the outcrops indicates that while average length
508 varied between 0.5 and 3.5 m, in all the selected FSS, fracture density (P20) is similar in FSS1,
509 FSS2 and FSS3 (10.5 m^{-2} , 21 m^{-2} , 13.3 m^{-2}), and dissimilar across FSS4 and FSS5 (62.3 m^{-2}
510 and 42.5 m^{-2}) (Fig. 8d, e). The average fracture spacing (in each FSS) for each fracture
511 orientation range between 0.7, 0.72 and 0.4 m for NNW-SSE, NE-SW, and ENE-WSW,
512 respectively (Fig. 8e). The dihedral angles between the different fracture orientations measure
513 between 18° and 60° .

514 The overall results show two deformation phases, falling in the reverse fault and strike-slip
515 stress regimes with sub-horizontal σ_1 striking approximately NW-SE to NNW-SSE and NE-

516 SW, respectively (Figs. 6 through 8). A later compressive deformation stage with a stress
517 regime showing largely N-S sub-horizontal σ_1 was also observed. These stress regimes and
518 orientations correlate well to the far-field sub-horizontal σ_1 observed for the Latemar
519 mountains during the Middle to Late Triassic and Neogene times, representing the subsidence
520 – and Alpine deformation stages, respectively (Boro et al., 2013, 2014; Hardebol et al., 2015;
521 Igbokwe et al., 2022). The orientation of these stress regimes supports the boundary loading
522 directions defined in the modelling work presented here. Also, during the modelling, we
523 assumed that all fractures, including stylolites, can be re-used as fluid-flow conduits. Although
524 stylolites tend to hinder fluid flow (Boersma et al., 2019), observations in figures 6 and 7 show
525 they can enhance fluid movement.

526 **6.2. Boundary loading conditions**

527 The boundary conditions defined by two stages of far-field stress loading scenarios are
528 presented in figure 9. The first and second loading scenarios, corresponding to the far-field
529 sub-horizontal σ_1 (stress), acted on the studied carbonates forming the Latemar buildup during
530 the subsidence and Alpine deformation stages, respectively. The first stage of loading, acting
531 from the NW-SE direction, gradually reaches a maximum loading condition at 50 MPa in Time
532 Step 1.0 (Time Steps here are analogues to quasi-static loading; Fig. 9a, c). Line a-b-c (blue
533 curve) represents this stage in figure 9c. The Line d-e-f (orange curve) depicts the second stage
534 loading, which acts from the N-S direction and gradually reaches a maximum value of 160
535 MPa in Time Step 2.0 (Fig. 9b, c). It is essential to note that the second loading stage was added
536 when the first stage loading reached the maximum magnitude of 50 MPa, and while the first
537 stage loading was still active, the addition of the second stage loading continued until a
538 maximum magnitude of 160 MPa was reached (Fig. 9c). Figure 9c shows the graph of the Load
539 (MPa) versus Time Steps, and the grey bar represents the transition zone between the two
540 loading scenarios.

541 **6.3. Stress orientation effects and aperture distributions**

542 The local distributions of stresses and the computed aperture distributions from local state of
543 stress are presented. The effects of changing stress regimes, from NW-SE and N-S directed
544 stress cases, for each of the FSS domains are shown in figure 10, demonstrating the
545 distributions of Von-Mises stress in all the FSS due to applied loading conditions. At the onset
546 of the subsidence stage, a uniform distribution of local stress state at Time Step 0.01 is

547 observed. This uniform distribution of local stress increases gradually until Time Step 1.0 is
548 reached. At Time Step 2.0 (Alpine deformation stage), the increasing loading in the N-S
549 direction shows the disperse of the uniform stress distribution with noticeable fluctuations in
550 magnitudes, pointing to the varied change in the local state of stress caused by the heterogeneity
551 of fractures (Fig. 10).

552 Figure 11 shows the displacement magnitude distribution when the loading conditions are
553 at Time Step 2.0 in each FSS. The observations in figures 10 and 11 document how changes of
554 the sub-horizontal far-field stresses and their magnitude cause varied deformation and/or
555 influence the individual fractured rock domain, the FSS. These changes, particularly in figure
556 11 are, however, quantified for each FSS, as ranging from 0 – 0.012 m (FSS1), 0 – 0.079 m
557 (FSS2), 0 – 0.080 m (FSS3), 0 – 0.034 m (FSS4) and 0 – 0.015 m (FSS5). It is observed that the
558 total displacement magnitude in the model increases with increased fracture density and
559 connectivity. For example, FSS2 and FSS3 show a slight increase in the displacement
560 magnitude due to increased fracture density and intersection (Figs. 8e and 11).

561 Depending on the magnitude and orientation of the loading scenarios, fractures open, close
562 and/or shear. Fracture apertures and their overall distributions against the number of fractures
563 for each FSS are shown in figure 12. The aperture values vary between 2 and 40 mm, showing
564 distinct heterogeneous apertures of variable sizes within each FSS (Fig. 13a). Quantifying the
565 changes in aperture as a function of increasing stress and changing stress orientations for each
566 FSS, figure 12b shows the general trend of aperture distribution. For instance, in each FSS, a
567 rapid increase in aperture value was observed as the first loading in the NW-SE direction
568 increased, and this value peaked at Time Step 1.0 (at 50 MPa). At the second loading stage in
569 the N-S direction, the trend of aperture values changes. For example, in this case, FSS 1, 4 and
570 5, the aperture values initially decrease rapidly until Time Step 1.3 is reached. After that,
571 aperture trends remained constant for FSS 4 and 5, whereas FSS 1, 2 and 3 aperture values
572 gradually increased until reaching their peak values at Time Step 2.0 (160 MPa; Fig. 13b).

573 **6.4. Effective permeability as a function of loading**

574 The effective permeability, in each of the FSS, is presented as a function of the NW-SE and N-
575 S loading scenarios. Fracture apertures mechanically generated were applied to the finite
576 element models, and by running the single-phase flow simulations, the effective permeabilities
577 for each loading condition (stress orientation) were obtained. For a $2 \times 10^{-15} \text{ m}^2$ matrix
578 permeability, the flow is calculated based on the fracture densities and orientations in all the

579 FSS. The flow paths are linked to the areas where there is a high number of fractures. Figure
580 13 shows a nearly homogenous long-term steady-state fluid pressure distribution and gradients
581 over the fractured rock domain (FSS) at the quasi-static loading of Time Step 2.0 in the x- and
582 y-directions. This result points to the steady-state condition of the pressure field, serving as the
583 base from which the effective permeabilities of all the selected FSS were computed.

584 In addition, using the parameters in Table 1 and considering the initial fracture aperture to be
585 zero, figure 14 show the computed effective permeabilities in both x and y directions (the red
586 curve = k_{xx} and the black curve = k_{yy}). Also, the components of the permeability tensor for each
587 loading condition are shown in Table 2.

588 The results from the first loading stage (Time Step 1.0; representing the subsidence
589 deformation stage from the NW-SE direction) show a gradual increase in permeability beyond
590 the matrix permeability within each FSS (Fig. 14). The increased permeability values
591 correspond to increased aperture values in figure 12b, pointing to the initial opening of most
592 fractures parallel to the NW-SE direction. On the other side, for the second loading stage (Time
593 Step 2.0), for which the loading is applied from the N-S while the first loading stage is active,
594 the permeability values are seen to increase in varying degrees. While the close fractures have
595 reduced aperture and permeability values, opened fractures exhibit increased permeability
596 values (Figs. 12b and 14). Further, in both loading scenarios, a general observation shows that
597 the permeability values are variable in each FSS, pointing to the random distribution of
598 fractures, varied densities (Fig. 8) and varied aperture values (Fig. 12a). For instance, in loading
599 stage 1 from the NW-SE orientation, the results in the permeability plots show both the vertical
600 and horizontal permeabilities increase gradually in FSS 1 through FSS 5 (Fig. 14).

601 Although FSS 1, FSS 2 and FSS 5 have their permeability values (in both x- and y-
602 direction) close to the matrix permeability, FSS 3 and FSS 4 only show permeability values
603 increased by 55 and 70 % in comparison to the matrix permeability (Table 2). But, with the
604 second loading scenario from the N-S orientation, the k_{xx} and k_{yy} maintained a relatively steady
605 permeability value with a slight increase in FSS 3 through FSS 5. This is unlike FSS 1 and FSS
606 2, which recorded a sharp increase (a jump) in both k_{xx} and k_{yy} .

607 Quantitatively, the average permeability for all the FSS is given as $2.75 \times 10^{-15} \text{ m}^2$ and $2.64 \times$
608 10^{-15} m^2 in the x- and y-direction for Time Step 1.0, and $2.77 \times 10^{-15} \text{ m}^2$ and $3.24 \times 10^{-15} \text{ m}^2$ in
609 the x- and y-direction for Time Step 2.0, respectively. When compared to the matrix
610 permeability, these average permeability values, k_{xx} and k_{yy} , increased by 37.5 and 32% for
611 Time Step 1.0 (at the maximum magnitude of 50 MPa in the subsidence stage) and 38.5 and

612 62% for Time Step 2.0 (at the maximum magnitude of 160 MPa during the Alpine deformation
613 stage).

614 **7. Interpretation and Discussion**

615 The results of the structural analysis and geomechanical (numerical) simulations show how
616 stress regime change can impact the permeability of a naturally fractured carbonate rock. In
617 addition, this process can be the first step toward using fractures (and faults) as a flow medium.
618 This is especially true for the upscaling and larger-scale numerical simulations that are
619 important for fluid flow in geothermal and hydrocarbon systems.

620 **7.1. Subsidence- and Alpine deformation: establishing a realistic loading condition for the** 621 **Latemar buildup**

622 Deformation in the carbonates of the Latemar buildup caused the development of different sets
623 of fracture networks, including veins. As documented before, these fracture networks formed
624 during syn-sedimentary extension tectonics in the Middle Triassic, characterized by wholesale
625 subsidence deformation and faulting tectonics (Boro et al., 2013; Preto et al., 2011). Other
626 studies (e.g., Goldhammer and Harris, 1989; Emmerich et al., 2005) suggest that the subsidence
627 deformation in Latemar may have continued until Late Triassic or Early Jurassic times.
628 Thereafter, the Latemar platform was affected by the Alpine deformation between the Late
629 Paleogene and Early Neogene times. These tectonic episodes significantly impacted the
630 hydraulic properties of fracture networks at the Latemar.

631 Given the complexities of the modelling work presented here, the studied fracture networks
632 were assumed to have formed earlier prior to the major tectonic episodes, as documented in
633 Preto et al. (2011). Although both tectonic events may have developed new fractures, the
634 modelled tectonic stresses in this study did not allow for further development of fractures but
635 for fractures to open, shear and/or close, consequent on the orientation of the stresses.

636 Our structural analysis results (Figs. 6 and 7) reveal the orientation of the principal stress
637 fields affecting the fractures at the subsidence and extension tectonic stage, which is roughly
638 NW-SE to NNW-SSE trending sub-horizontal σ_1 and a sub-vertical σ_3 . This compression is
639 believed to have taken place at a relatively shallow to an intermediate burial depth. Indirect
640 evidence for this notion comes from the understanding that, in the presence of sub-horizontal
641 tectonic stress, a sub-vertical position of σ_3 is compatible with low-angle reverse faults (or
642 structures), forming at shallow to intermediate burial depth (e.g., Fig. 6; Bisdorn et al., 2016;

643 Bertotti et al., 2020). This means that at the subsidence stage, the overburden stress is the
644 resultant far-field stress with an approximate NW-SE orientation and a magnitude of 50 MPa.
645 On the other side, the main Alpine deformation in the Dolomite Mountain Belt (including the
646 study area) involved maximum compressive stress (σ_1) orientated approximately N-S with a
647 magnitude estimated at 160 MPa (Peacock, 2009; Abbà et al., 2018). Because the observed
648 correlation between the fractures (and their networks) and stress fields are largely clear, the
649 magnitude of the two sub-horizontal σ_1 served as the input parameters for the numerical
650 mechanical modelling phases, i.e., the mechanical loads used during the mechanical FE model
651 setup. These mechanical loads captured the realistic compressive boundary condition,
652 reflecting the tectonic episodes of the study area, unlike previous studies (e.g., Zhang and
653 Sanderson, 1996) and are essential for the realistic computation of effective permeabilities,
654 even at the subsurface.

655 Figure 9c reveals that the deformation caused by the first stress field (NW-SE shortening
656 direction at subsidence stage) was still active, as the second stress field (N-S sub-horizontal
657 σ_1 ; Alpine deformation) was added. The gap (the grey bar Fig. 9c) between the two stresses in
658 our model signifies that some stresses in the horizontal dimension due to subsidence
659 deformation are present (active) at the Alpine deformation stage when they increase and
660 become more extensive in the N-S direction. This is in contrast with previous studies, for
661 example, Stephansson et al. (1991) and Yale (2003), where modelling results (or model setup)
662 utilize a homogenous stress regime that does not account for all other stress impacts around the
663 fracture network. In the cases where all the stress regime changes are considered, each stress
664 regime's effect remains independent from each other (Agheshlui et al., 2018; Bisdom et al.,
665 2017). That is, the impact of the first or second stress field, as the case may be, is not kept
666 active when the third or fourth stress field is implemented. Unique to our study, the orientation
667 and magnitude of all the stress regimes, reflecting the major tectonic event over a geological
668 period, are kept active and accounted for during the geomechanical modelling. Thus, making
669 the impact of stress irreversible in a given rock domain, without which it can pose significant
670 uncertainty in the computation of effective permeability at the subsurface reservoir scale level.
671 This argument follows the proponent of representing the realistic tectonic events affecting
672 fracture networks on the geomechanical models. Thus, proposing that modelling the impact of
673 different tectonic episodes (in our case, the subsidence- and Alpine deformation) on a given
674 rock domain (FSS, in the Latemar platform) over a geologic time (Triassic to Neogene) will
675 significantly reduce uncertainties in computing the apertures and permeabilities, even at the
676 reservoir scale.

677 Based on this, the loading conditions and the calculated permeabilities presented in [figures](#)
678 [9](#) and [14](#) may reflect close occurrences of what is obtainable at the subsurface of a natural
679 carbonate reservoir.

680 **7.2. The link between heterogeneous aperture, fracture geometry and the impact of stress** 681 **and effective permeability**

682 As a rule, the geomechanical models and flow simulations in fractured rocks have always
683 depended on: (i) the subsurface datasets, which are typically expensive, albeit with uncertainty
684 ranges ([Bourbiaux et al., 2002; 2005](#)), and (ii) stochastic datasets ([Khodaei et al., 2021;](#)
685 [Timothy & Meschke, 2016](#)), which are not realistic when considering the behavior of a fracture
686 network in a natural reservoir. In contrast to these studies, the presented investigation solely
687 uses the outcropping network geometry as input for geomechanical and flow models, not
688 considering the outcropping apertures. Instead, the geomechanical aperture sizes were
689 modelled in a representative outcrop fracture network using computational homogenization
690 (FE mechanical modelling) for unperturbed and perturbed fracture states. The natural fracture
691 system is perturbed by applying mechanical load, analogous to in-situ stress.

692 The combination of outcrop fracture geometries, mechanical loading and aperture
693 distribution results in models that are more representative of fractured reservoir permeability
694 compared to analogues studies that use apertures of exhumed barren fractures or assume a
695 constant aperture for the whole fracture network ([Bisdom et al., 2017; Makedonska et al.,](#)
696 [2016](#)). Like most fractured reservoir models, the aperture is assumed constant per fracture or
697 even per fracture set because generating a reservoir scale 3-D fracture model with mechanically
698 controlled heterogeneous aperture distributions can be a complex task ([Geiger and Matthäi,](#)
699 [2014](#)). However, studies by [Jonoud and Jackson \(2008\)](#) and [Cottureau et al. \(2010\)](#) have given
700 an upscaling alternative through arithmetic or harmonic averaging of the explicit fracture
701 permeability model calculated per fracture node. That is, the mechanically controlled
702 heterogeneous aperture, just like the mechanically generated aperture in [figure 12](#), can easily
703 be upscaled to serve a more representative reservoir scale fracture model.

704 Further, fracture networks, including intensely fractured zones, are believed to significantly
705 influence the effective permeability and fluid flow patterns in a naturally fractured reservoir,
706 particularly in tight carbonate reservoirs. These network areas show high porosities and
707 permeabilities relative to the surrounding host rock ([Bruhn et al., 2017; Matthäi, 2003](#)). It is a
708 widely held view that the extent to which the fracture networks impact flow lies in the fracture's

709 structural arrangement and geometry, such as fracture orientations, spacing, and length
710 (Bisdom et al., 2016; Hardebol et al., 2015; Olson, 2003; Scholz, 2010). However, these views
711 are limited to models that quantify aperture based on fracture length and spacing relations
712 (Olson, 2003; Scholz, 2010). The results from our models (Figs. 12 and 14) indicate that
713 aperture and effective permeability are not easily related to fracture geometrical parameters
714 such as length or spacing. This is especially true because the linear functions of fracture lengths
715 and spacing have little or no effect on the mechanically generated aperture distribution (Bisdom
716 et al., 2016), controlled by fracture orientation and the impact of stress.

717 Given these results, we suggest that the distribution of aperture and permeability (in the
718 presented models) is influenced by the impact and direction of the stresses, fracture orientation
719 and shear displacement (Figs. 12 and 14). For the stress-induced aperture, the mechanical load
720 opens individual fractures orthogonal to the direction of loading at different rates and, at the
721 same time, closes the fractures parallel to the loading direction (Heffer & Koutsabeloulis,
722 1995). Therefore, in a favorable orientation, that is, the direction with significant stress
723 components (of mechanical loading), which in our case is NW-SE for the subsidence
724 deformation, the results show a considerable increase in permeability as the load increases (Fig.
725 14) before reaching a peak at Time Step 1.0. The increased permeability is beyond the matrix
726 permeability within all the FSS. On the other side, permeability continues in an upward
727 trajectory with varying degrees of increased values (Fig. 14) in the direction of more significant
728 stress components (N-S), at the Alpine deformation stage, for all the FSS, until a maximum
729 loading value is reached.

730 The evolution of permeability values in x- and y-directions within the FSS indicates that
731 the values of the permeability distribution are variable. These variabilities are linked to the
732 varied fracture densities and aperture values (Fig. 8). In fractured reservoirs, apertures of
733 natural fractures are highly heterogeneous and can contribute to the induced anisotropy in the
734 permeability (Makedonska et al., 2016). In our case, for instance, FSS 1, FSS 3, and FSS 5
735 significantly differ in the evolution of the permeability values in the x- and y-directions and
736 distinctly show induced anisotropy in the permeability within their rock domains (Fig. 14).
737 This is demonstrated by the differences in the permeability values in x- and y-directions. The
738 higher the difference in the permeability values of the x-and y-direction, the more significant
739 the induced anisotropy. These differences are linked to the fracture connectivity and/or
740 distributions (Fig. 8c, d) within each FSS. Conversely, FSS 2 and FSS 4 show isotropy in
741 permeability, as the differences in the permeability values of x- and y- directions are relatively
742 small compared to the differences in FSS 1, FSS 3, and FSS 5. These behaviours can be

743 explained as follows; initially, during the subsidence stage, when the far-field stress (or
744 compressive loading) is in the NW-SE direction, the fractures parallel to this direction open
745 while the fractures orthogonal to the NW-SE direction close. The opening of the NW-SE-
746 directed parallel fractures increases the overall permeability in all FSS (Fig. 14). However, at
747 Time Step 1.0, the second set of loading scenarios is gradually applied from the N-S direction
748 at the onset of Alpine deformation. While the loading from the first stage is kept constant, the
749 overall permeability at this stage continues with varying degrees of the increase until Time
750 Step 2.0. This implies that the new loading opens the fractures parallel to the N-S direction and
751 closes the fracture orientation in the E-W direction (Fig. 14). Hence the second loading stage,
752 in principle, opens the fractures that were previously closed by the first loading stage. Meaning
753 that a significant number of fractures opened, resulting in larger permeability values. There is
754 also a documented sharp jump in permeability values for FSS 1, FSS 2 and FSS 3 at the
755 beginning of the second loading stage. This sharp increase in the permeabilities may imply that
756 fractures oriented slightly in the NE-SW and/or NW-SE directions (Fig. 8c) may have been
757 opened by the N-S loading condition.

758 In addition, fracture density and/or the number of fractures in each rock domain, FSS, may
759 have played a considerable role in the sharp spike in permeability values. For example, in FSS
760 1 and FSS 2, where the quick jump in permeability values is predominant, fracture density is
761 relatively low than in FSS 4 and FSS 5, with high fracture density and no visible sharp increase.
762 These explain the jump and the overall increase in permeability during the second loading
763 stage. Recent studies have linked the fracture density, the number of fractures and the stress
764 orientation with an overall increase in the permeability values (e.g., [Bisdom et al., 2016](#);
765 [Furtado et al., 2022](#)). Although some fractures may have been sheared and eventually opened
766 during loading, we believe that the shearing of these fractures has minimal or no impact on the
767 overall permeability of the rock domains.

768 One interesting observation in this research investigation is that the two far-field stresses
769 acting within a rock domain at a different magnitude and orientation at the same time (in our
770 case at Time Step 2.0) do not necessarily cancel out each other in the mechanical FE modelling
771 phases. But these stresses create new effects on the overall aperture and permeability
772 distributions. In our case, a general increase in the overall aperture and permeability values,
773 respectively (Figs. 12b and 14).

774 **7.3. Implications**

775 All modelled features are assumed as reactivated fractures that control the fluid flow in the
776 presented numerical workflow. Because of the effects of weathering and exhumation, the
777 distinction between open fractures, veins, stylolites and shear fractures was not made. Although
778 this assumption is widely used in reservoir modelling, complex features such as sub-vertical
779 (tectonic) stylolites or partly cemented fractures can inhibit or enhance fluid flow (Bruna et al.,
780 2019). Cemented fractures strongly constrain fluid flow, as cementation reduces the aperture
781 of a fracture plane (Gale et al., 2010; Olson, 2007).

782 Although our study permits investigating the impact of stress regime change in detail, the
783 results will broadly differ in complex and large-scale reservoirs since our model is orders of
784 magnitude smaller than a subsurface reservoir. The studied fracture networks have relatively
785 simple geometries on a small scale compared to what is usually observed in large-scale
786 reservoirs. However, Matthai and Nick (2009) and Bisdorn et al. (2016a) upscaled
787 heterogeneous aperture distribution to a single averaged aperture, which provided the same
788 permeability distribution. This means that averaging and upscaling the permeability of small-
789 scale fracture models can accurately describe permeability when most fracture contributes to
790 flow. Otherwise, the small-scale models may not be representative.

791 Another issue is ignoring the influence of the overburden stress on fracture aperture.
792 Overburden stresses result in lateral expansion of rock layers and, in addition to the horizontal
793 stresses, fracture apertures are strongly dependent on overburden stresses due to Poisson's
794 effect (Agheshlui et al., 2018). Therefore, conducting the numerical analysis in 2-D constrains
795 the effects of lateral expansion of fractured rocks domain, which can displace the rock body
796 and increase the fracture aperture. This means that the results presented in our model may only
797 provide indicative approximations.

798 Lastly, having a better knowledge of stress conditions and elastic parameters aids the
799 prediction of realistic aperture descriptions for reservoir models. Studies have noted the impact
800 of these parameters on modelled stress-induced aperture such that stress orientations and the
801 magnitude of differential stress largely influenced the resulting aperture distribution
802 (Agheshlui et al., 2018; Bisdorn et al., 2016a). The presented model assumed the magnitudes
803 of the two stress regimes following the calculated palaeostress in other field areas with the
804 same geodynamic conditions. The values of these magnitudes are debatable and subjective.
805 Therefore, changing the magnitude of these stress values can affect this study's overall aperture
806 and permeability distributions. Still, the workability of the model remains stable and can
807 compute any given matter.

808 8. CONCLUSIONS

809 A detailed workflow is presented that applies a displacement-based linear elastic finite element
810 method (FEM) to solve linear poroelasticity equations to model the impact of stress regime
811 change on the permeability of carbonate rock. By considering the tectonic episodes
812 (Subsidence- and Alpine deformations) and outcropping fracture network at the Latemar, the
813 stress-induced heterogeneous aperture distribution was generated in the selected Fracture Sub-
814 structure (FSS). The impact of stress regime change on flow was quantified in terms of varied
815 aperture and effective permeability. Although the studied fracture networks have relatively
816 simple geometries compared to what is ordinarily observed in reservoir scale, this permits
817 investigating the impact of individual stress magnitude and stress regime change on the
818 permeability.

819 First, a numerical experiment was simulated using a synthetic single-fracture in a rock
820 matrix domain to validate the consistency of the effective permeability values with the chosen
821 varying aperture sizes. By choosing a range of intrinsic matrix permeability and fracture
822 apertures, the results show that a single fracture has little influence on the effective permeability
823 of the rock domain when the permeability of the fracture is less than that of the rock matrix.

824 Second, we presented the structural analysis and interpretation of fracture network geometry
825 in the study area, considering the two major tectonic episodes, the subsidence – and Alpine
826 deformation. Their stresses were considered far-field stresses (compressive loading), which
827 informed the boundary conditions (two loading stages) in the geomechanical FE modelling.
828 The first stage of loading acts from the NW-SE direction, representing the subsidence
829 deformation and gradually reaches a maximum loading condition at 50 MPa in Time Step 1.0,
830 whereas the second loading stage, the Alpine deformation, was superimposed on the first and
831 gradually reached a maximum value of 160 MPa in Time Step 2.0.

832 It was shown that in the directions with less significant stress components (of mechanical
833 loading), NW-SE, the overall permeability increased gradually within all the FSS as the load
834 increased before reaching a peak at Time Step 1.0. In addition, permeability increase continues
835 with the upward trajectory in the direction with more significant stress components (N-S) for
836 all the FSS as the load increases. Fracture density, the number of fractures and the stress
837 orientation play a critical role in the overall increase in the permeability values.

838 Finally, we conclude that the two mechanical loadings simultaneously acting within a rock
839 domain at a different magnitude and orientation do not necessarily cancel out each other in the
840 mechanical FE modelling phases. However, these mechanical loads create new effects in the

841 overall aperture and permeability distributions, which can be complicated in large subsurface
842 reservoirs.

843 **Acknowledgements**

844 The Petroleum Technology Development Fund (PTDF) Abuja, Nigeria, is thanked for
845 sponsoring the doctoral fellowship of the first author (Grant number: OSS/PHD/IOA/843/16).
846 The authors are grateful to N. Gründken and S. Schurr for their assistance during the fieldwork.
847 Many thanks to J. Dikachi-Igbokwe for her helpful comments, which helped improve this
848 paper.

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1095 **Declaration of interests**

1096 The authors declare that they have no known competing financial interests or personal
1097 relationships that could have appeared to influence the work reported in this paper.

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1101 **Author Contribution Statement**

1102 The first author named is the lead and corresponding author. All other authors are listed
1103 according to their contributions. We describe contributions to the paper using Contribution
1104 Roles Taxonomy (CRediT)² as follows:

1105 **Onyedika Anthony Igbokwe:** Conceptualization, Methodology, Software, Formal analysis,
1106 Investigation, Data curation, Writing- Original draft preparation, Visualization, Project
1107 administration. **Jithender J. Timothy:** Data curation, Writing- Reviewing and Editing,
1108 Validation. **Ashwani Kumar:** Investigation, Software, Resources, Data curation. **Xiao**
1109 **Yan:** Software, Validation, Data curation, Formal analysis. **Mathias Mueller:** Resources, Data
1110 curation. **Alessandro Verdecchia:** Resources, Data curation. **Günther Meschke:** Supervision,
1111 Project administration. **Adrian Immenhauser:** Supervision, Writing- Reviewing and Editing,
1112 Validation, Project administration.

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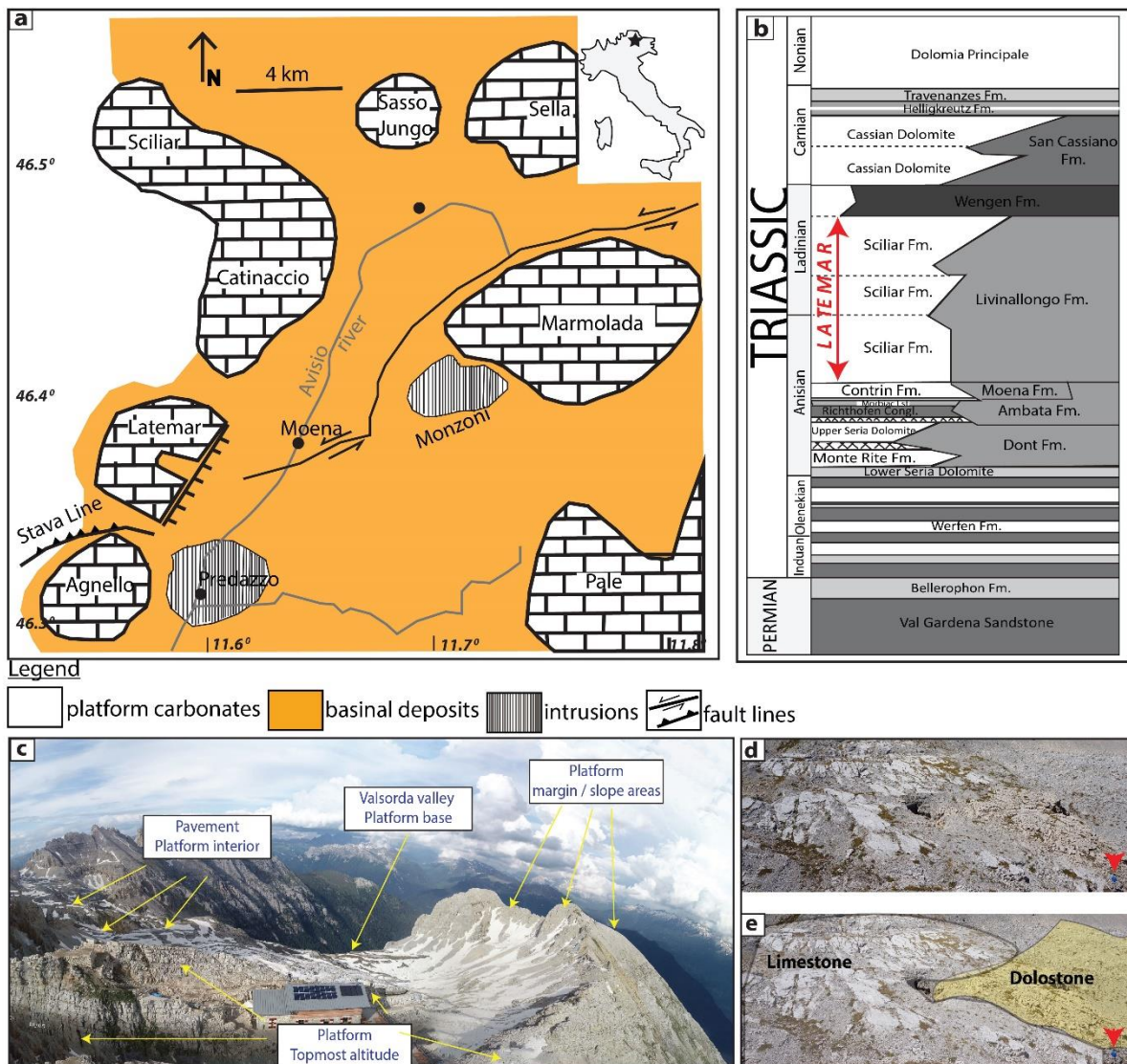
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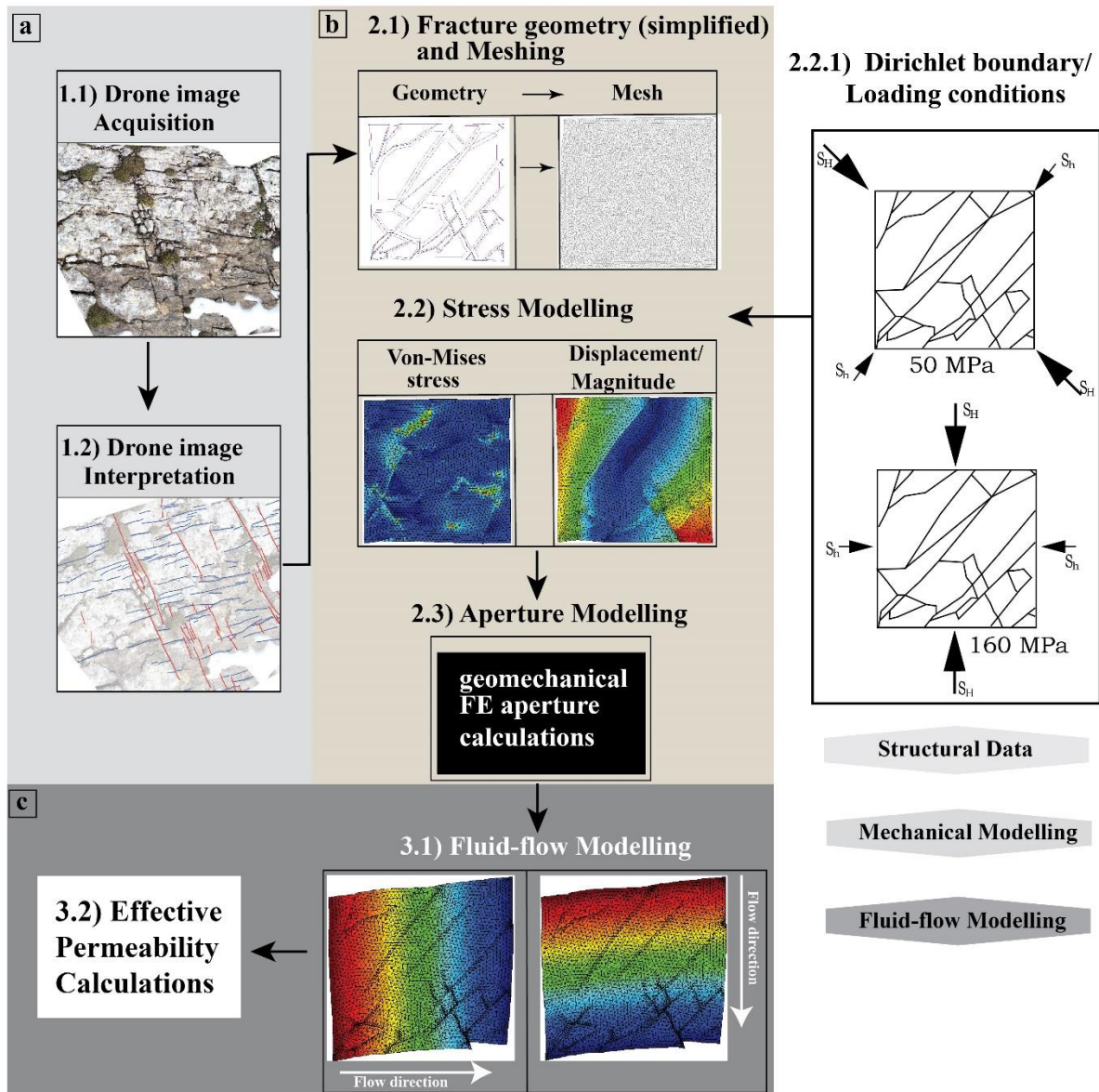
² Accessed from <https://www.elsevier.com/authors/policies-and-guidelines/credit-author-statement> (Sept. 20, 2022)



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1124 **Figure 1.** (A) Overview of the Latemar and the neighbouring Ladinian carbonate buildups (or
 1125 platforms) and Upper Ladinian intrusions of the Dolomites (modified after Jacquemyn et al.,
 1126 2014, 2015). (B) Simplified stratigraphic chart of the Dolomite Mountains, modified after
 1127 Jacquemyn et al., 2014. (D) Drone image showing the general overview of the horse-shoe shape
 1128 of the Latemar buildups. The geometry and topology of the outcrops pointed with arrows have
 1129 been presented in Igbokwe et al. (2022). (E) Drone images acquired from the outcrop pavement
 1130 at platform interior.

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1133 **Figure 2.** Workflow for obtaining flow-base effective permeability using acquired drone
 1134 images from outcropping fracture networks. A) (Phase 1.1) Drone imagery, photogrammetry
 1135 and (Phase 1.2) fracture interpretation and digitization (the illustration shown is a Fracture Sub-
 1136 Sample (FSS) with $\sim 2 \times 2$ m dimension). B) (Phase 2.1) Simplifying and converting the
 1137 interpreted fracture network geometry to element geometry and, then, meshing, (Phase 2.2)
 1138 local stress modelling, (Phase 2.3) geomechanical finite-element fracture-aperture modelling
 1139 and calculations. C) (Phase 3.1) Fluid-flow modelling, considering "uncoupled" conditions and
 1140 (Phase 3.2) effective permeability calculations.

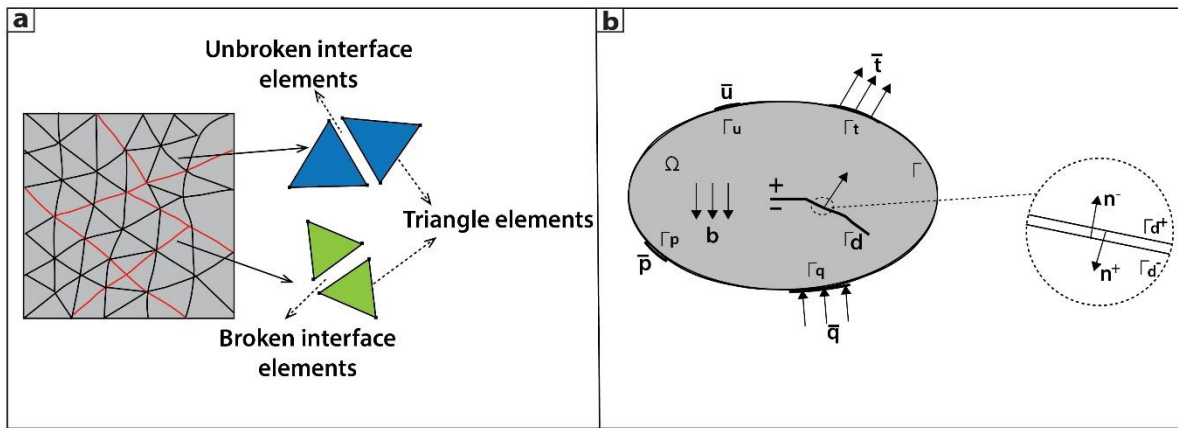
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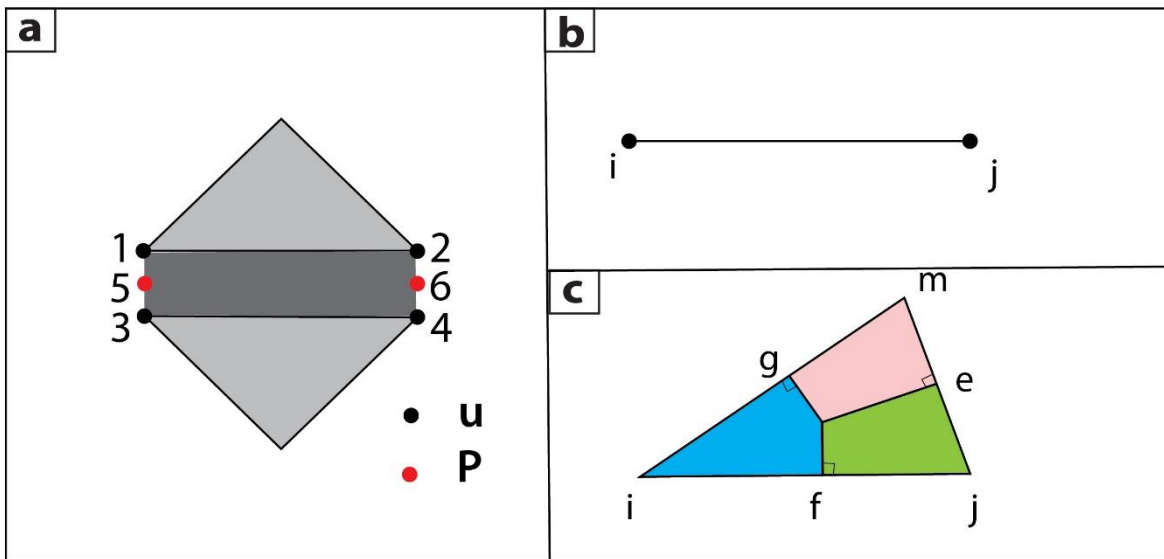
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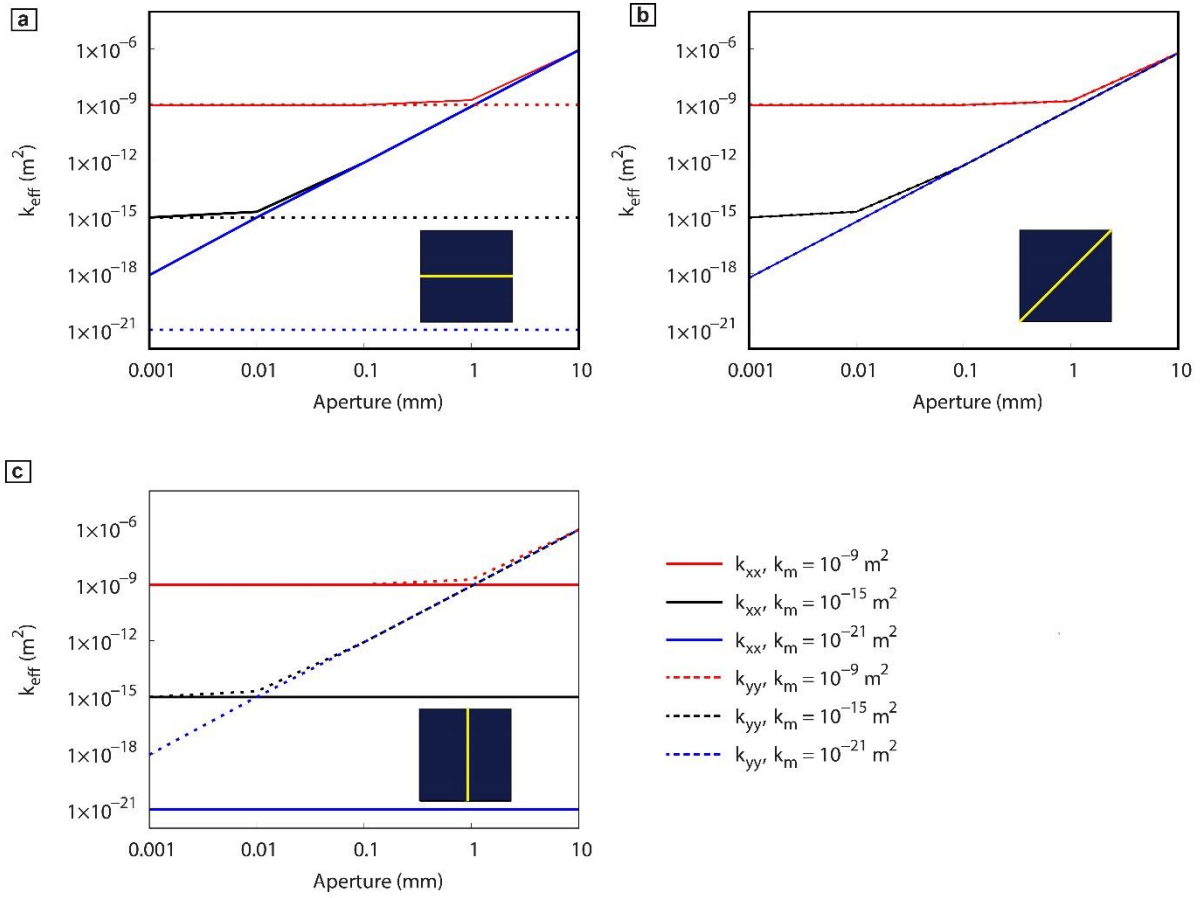
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1147 **Figure 3.** (A) Representation of a fractured rock using triangle elements and embedded
1148 zero-thickness interface elements in the solid model. (B) Boundary conditions of the porous
1149 medium Ω , including the geomechanical discontinuity Γ_d
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1152 **Figure 4.** The computational discretization model. (A) the thickness of the interface element
1153 with hydromechanical coupling. (B) Fracture (Pipe) equivalence of idealized 1-D fracture. (C)
1154 Fracture (Pipe) equivalence of 2-D porous medium
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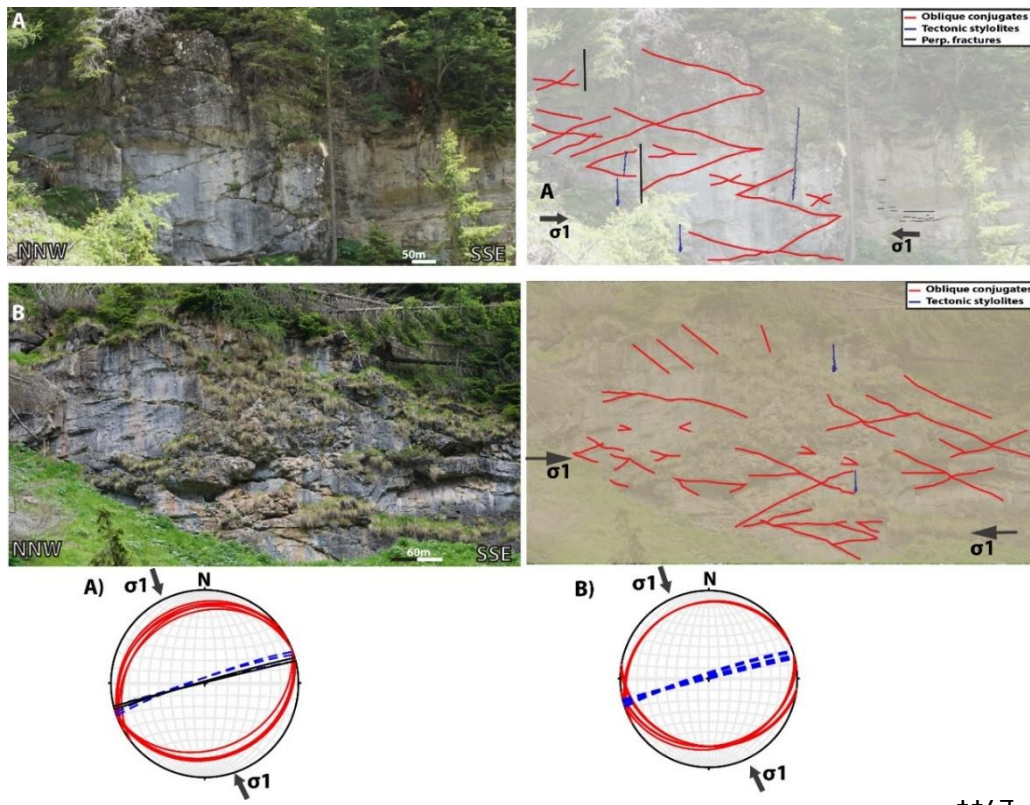


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1158 **Figure 5.** The effective permeability with different fracture aperture and rock matrix intrinsic

1159 permeability for single fracture (see embedded block) with different inclination degrees (a) 0° ,

1160 (b) 45° and (c) 90° .

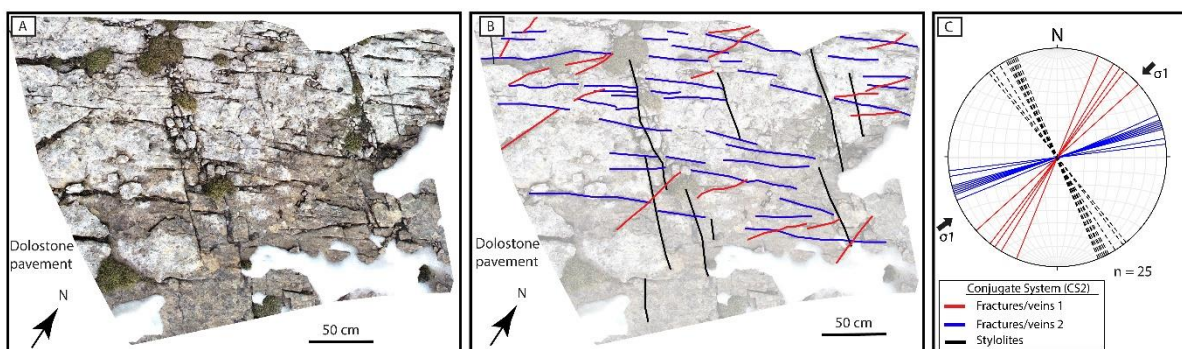


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1176 **Figure 6.** Sub-vertical outcrops exposed at the base of the Latemar. A and B) show reverse
 1177 conjugate fault with low-angle SSE - and ENE dipping fractures and a horizontal intersection.
 1178 The stereoplots show bedding-perpendicular stylolites and σ_1 striking approximately WSW-
 1179 ENE and NW-SE to NNW-SSE direction, respectively.

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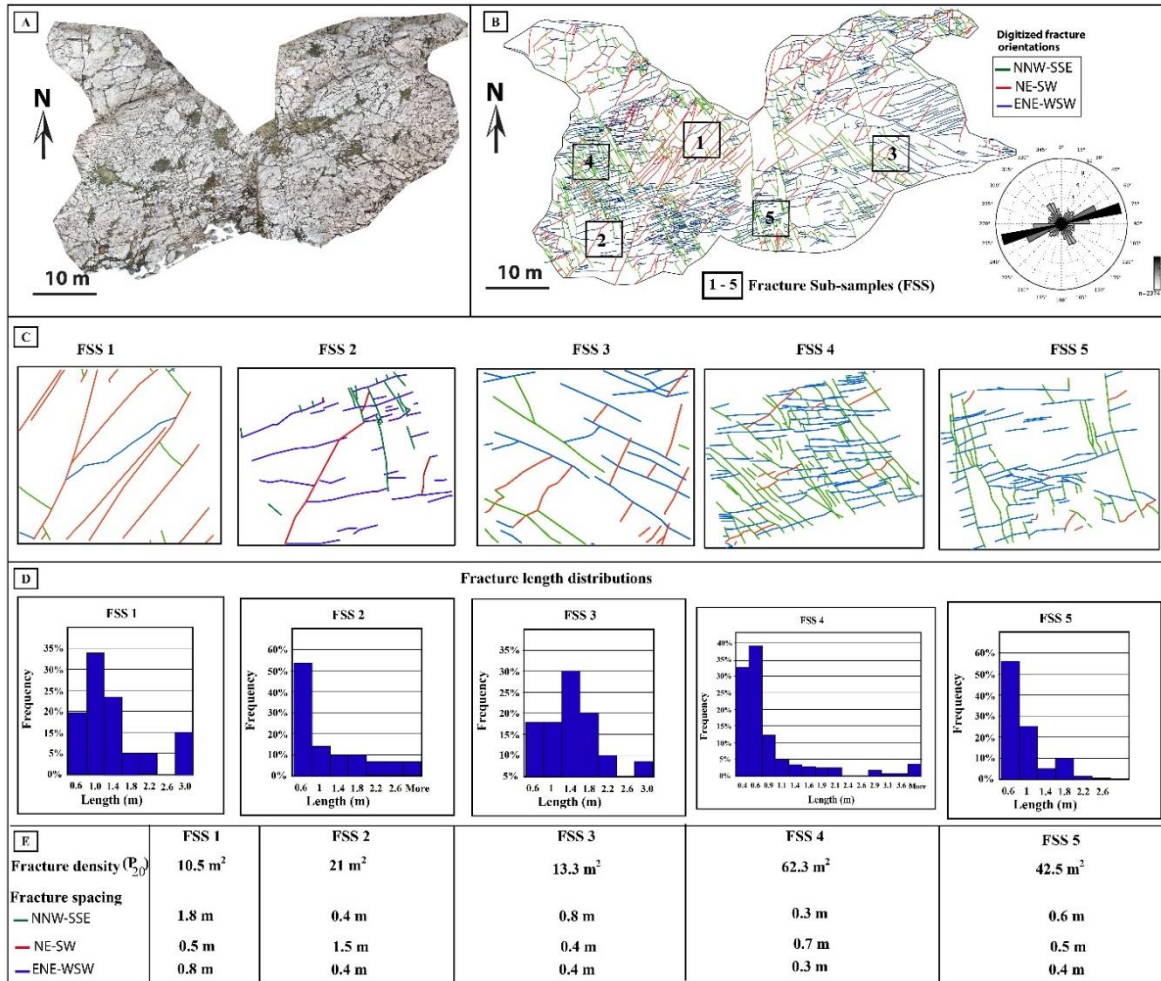
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1183 **Figure 7.** Sub-horizontal (pavement) outcrop exposed at the flat-topped Latemar. A and B)
 1184 show high-resolution 2D outcrop orthorectified photograph and digitized fracture map. C) The
 1185 stereoplots show σ_1 striking approximately NW-SE to NNW-SSE direction.

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1188 **Figure 8.** Original outcrop model with interpreted fractures, from where the five FSS were
 1189 extracted. A) High-resolution 2D outcrop orthorectified photograph. B) Digitized sub-
 1190 horizontal (pavement) in the Latmar with the position of the five outcrop windows, including
 1191 the stereoplot of more than 2000 fracture orientations. C) The five fracture models. D) Length
 1192 distribution of the five fracture models. E) Fracture geometry, including density and spacing
 1193 values of the five outcrop models.

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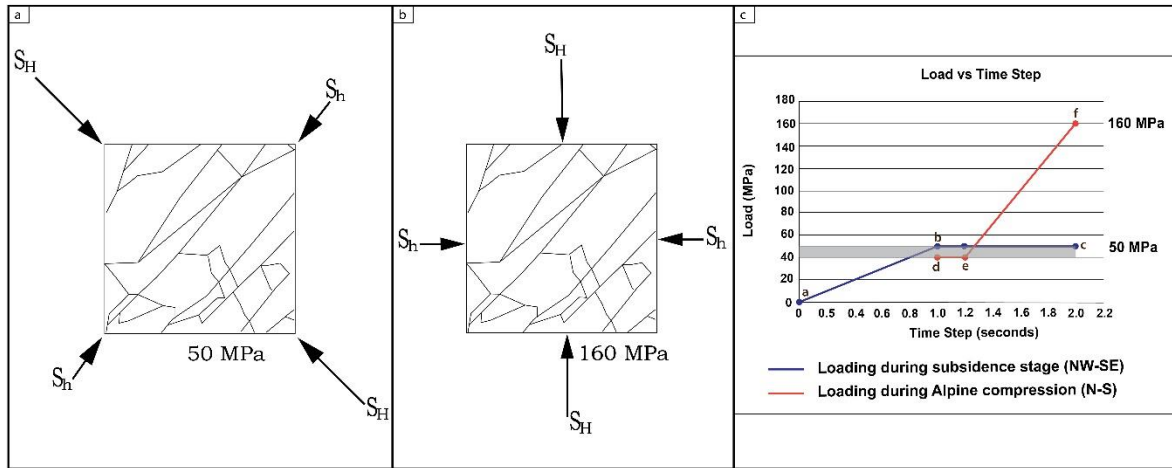
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1203 **Figure 9.** Quasi-static loading scenarios of the two tectonic stresses. A) The first stage loading
 1204 reflects the subsidence deformation stress from the NW-SE direction and has a maximum load
 1205 of 50 MPa. B) The second stage loading, representing the Alpine deformation stress from the
 1206 N-S direction, is applied (added), while the loading of 50 MPa is maintained until a maximum
 1207 of 160 MPa is reached. C) The plot of load against Time Steps. The first stage loading of 50
 1208 MPa corresponds to the line a-b-c (blue curve), whereas the second stage loading of 160 MPa
 1209 corresponds to the line d-e-f (orange curve). The grey bar indicates that some stresses during
 1210 the first loading were still active and became more significant during the second loading
 1211 scenario.

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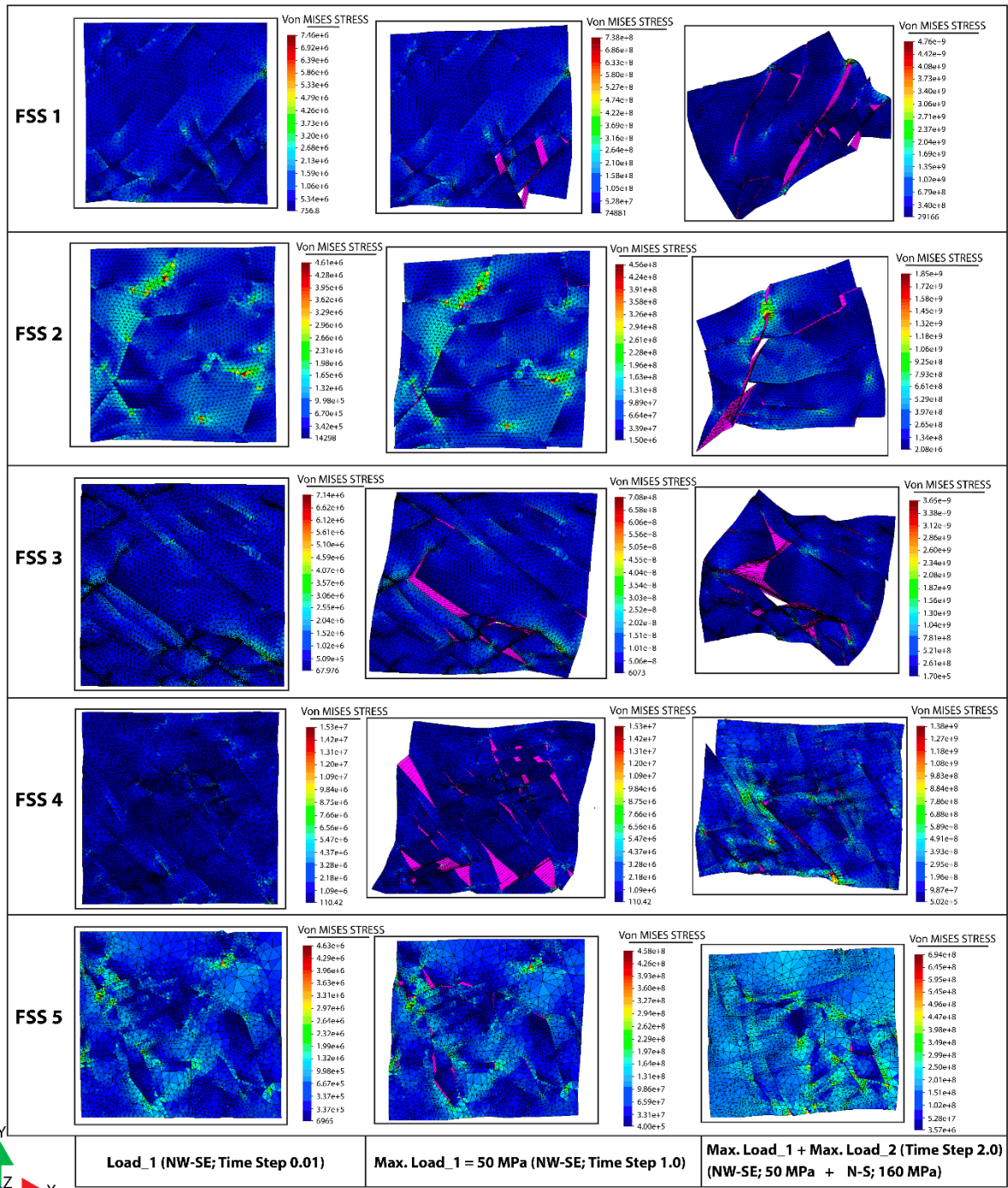
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1228 **Figure 10.** Von-Mises stress distribution (Pa) in the fractured rock domain (FSS 1 through FSS
 1229 5) under different loading conditions and orientations at Time Steps (0.01, 1.0, and 2.0). Note
 1230 that the deformation scale is x10. The stress colour spectra are the same for all figures.

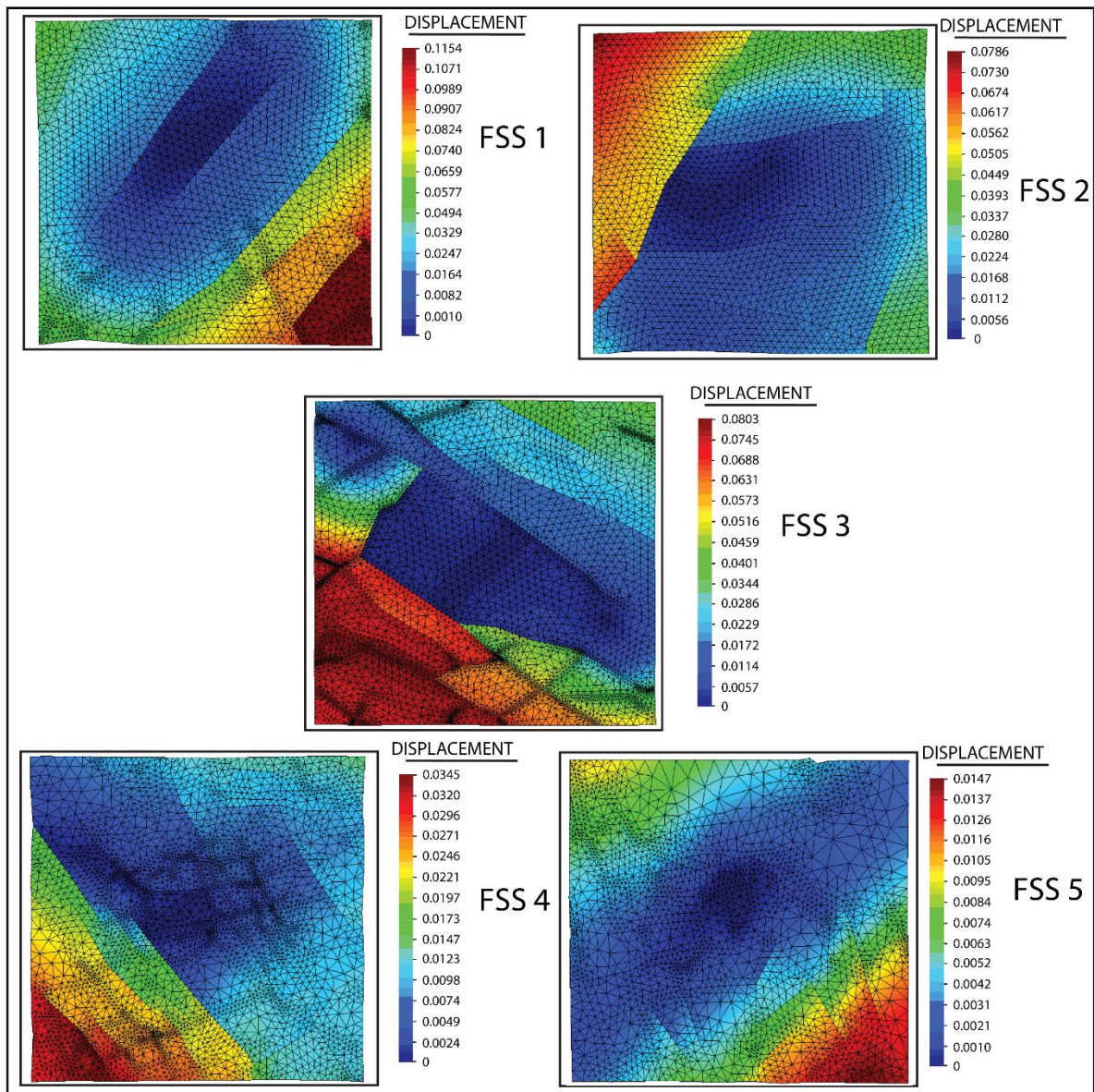
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1239 **Figure 11.** Distribution of actual displacement magnitude under the two tectonic stress
1240 conditions for FSS1 through FSS5, when the Time Step is 2. Units are in meters, and
1241 deformations are slightly magnified. Notice the varied displacements across the FSSs.

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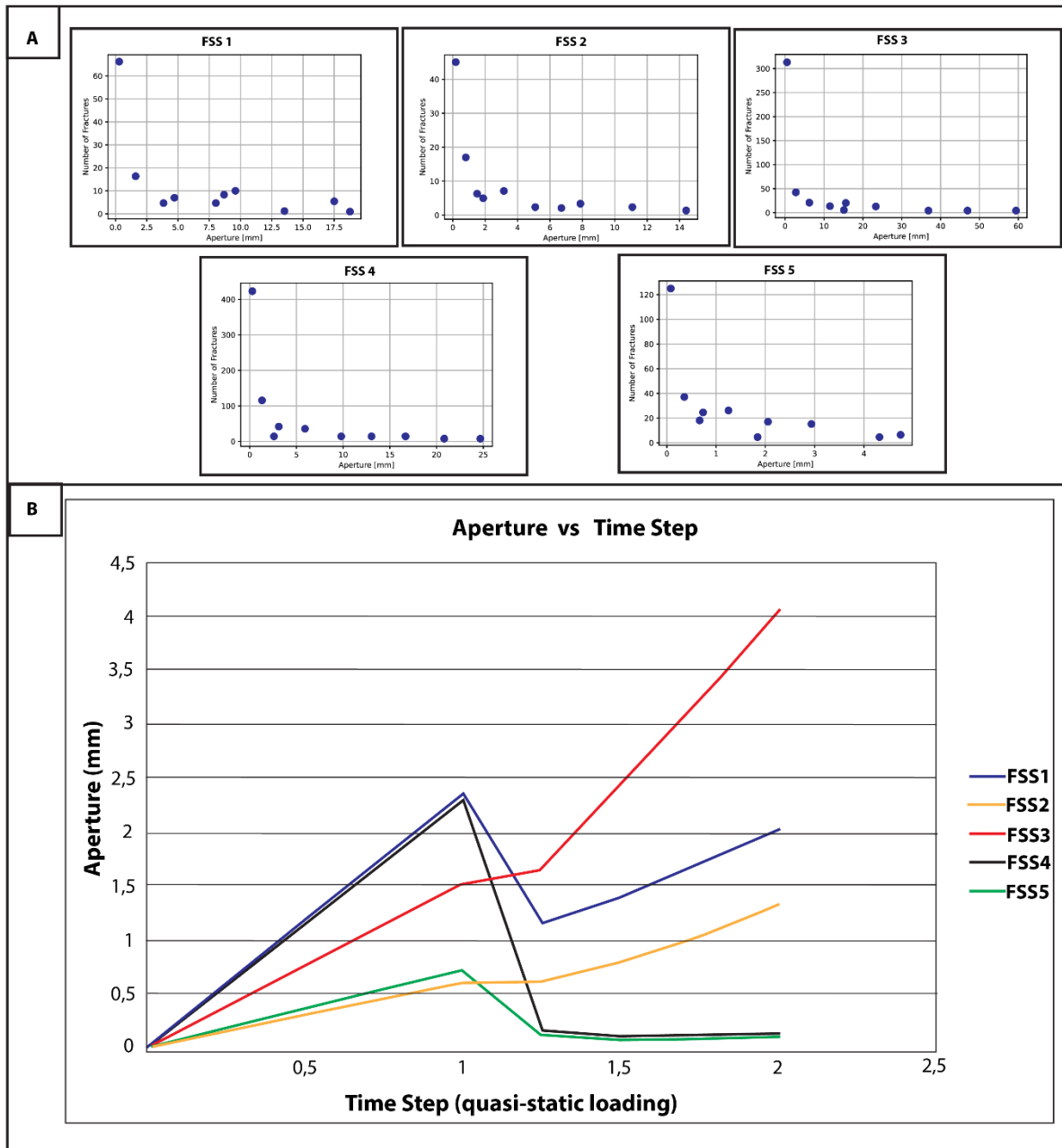
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1251 **Figure 12.** A) Distribution of fracture aperture under the two horizontal stress orientations. B) 1252 The relationship between aperture and mechanical loading conditions. Note that the Time is 1253 analogous to quasi-static loading conditions.

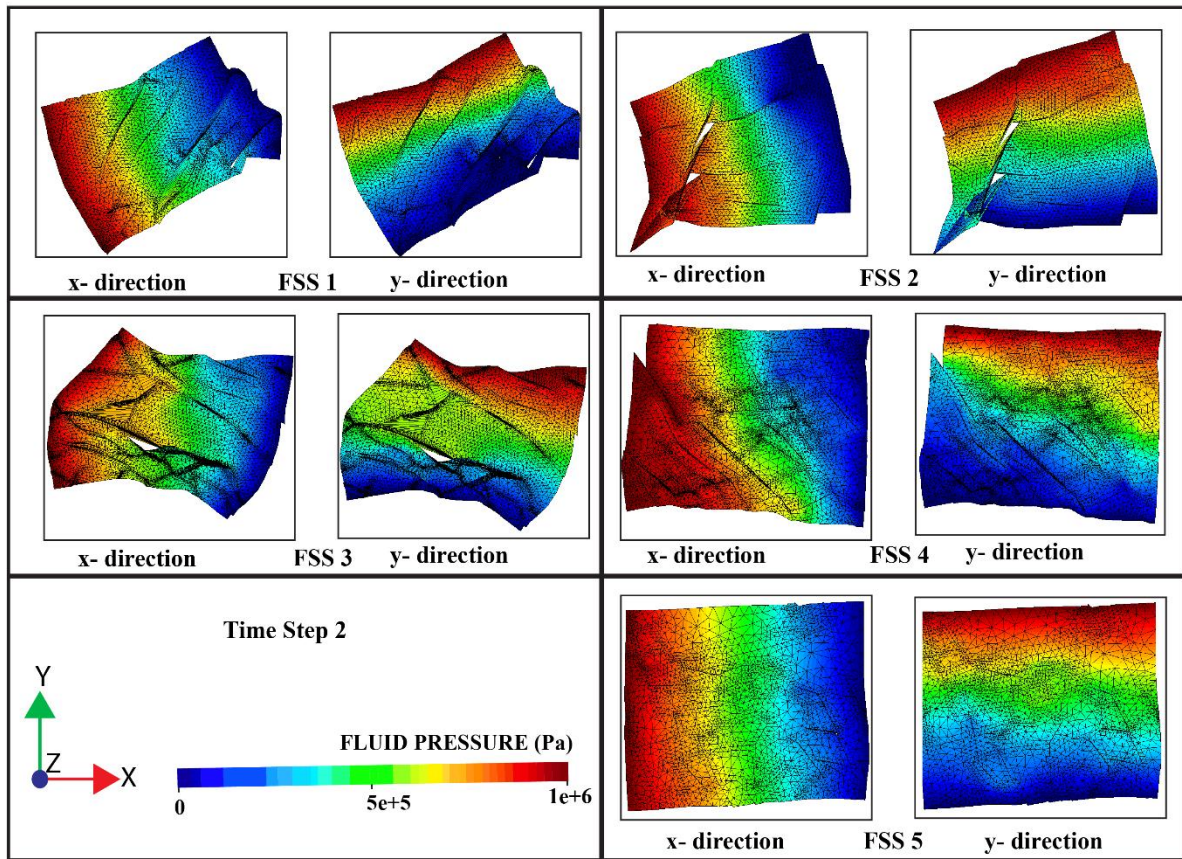
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1260 **Figure 13.** Contours of the fluid pressure gradient (Pa), obtained for a fluid pore pressure p at
1261 Time Step 2 (see Fig. 9 for the loading conditions), in the x- and y- directions. The matrix
1262 permeability is given as $2 \times 10^{-15} \text{ m}^2$

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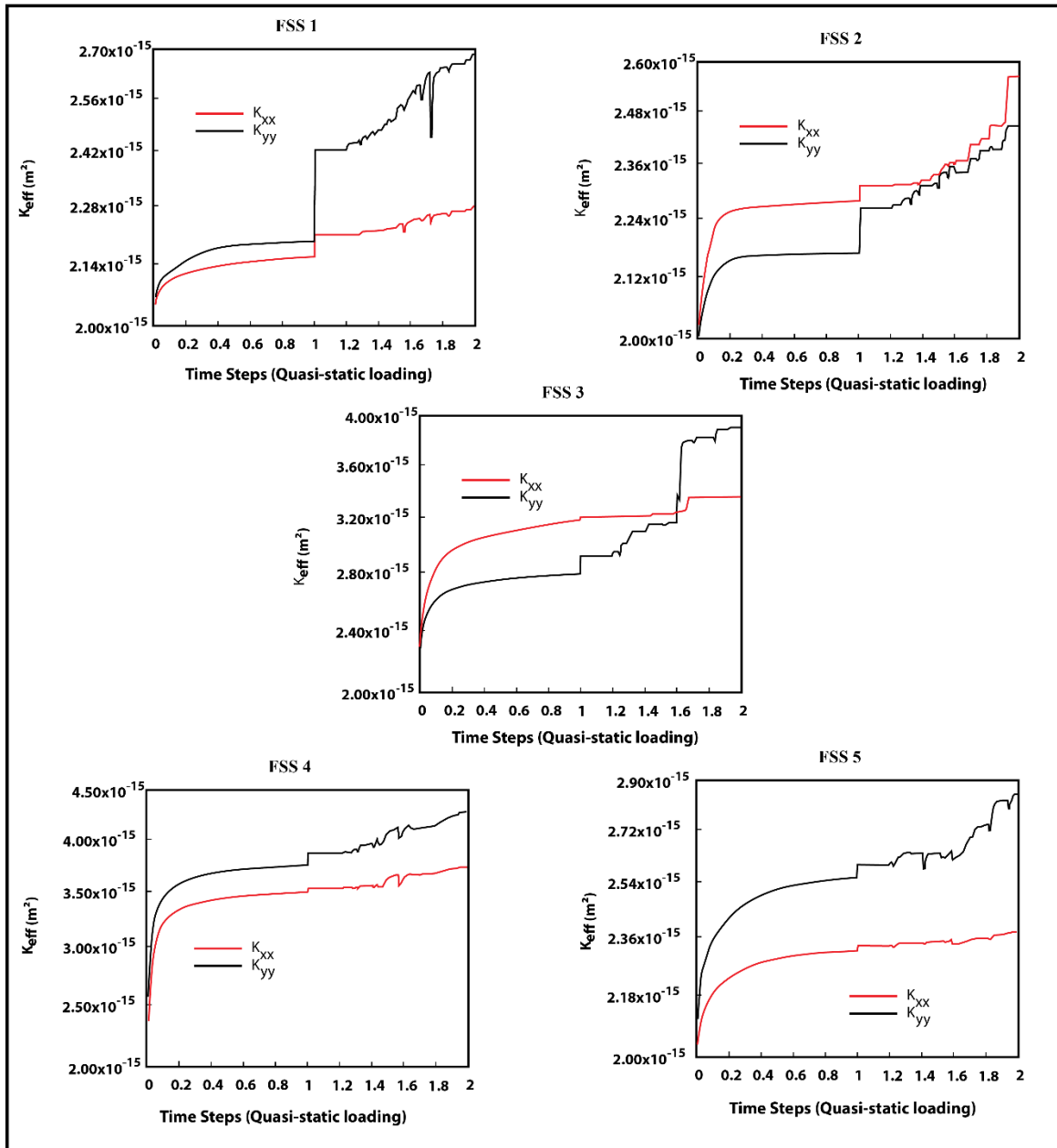
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1277 **Figure 14.** The Effective permeability values are plotted against the loading conditions for
 1278 FSS1 through FSS5. The FSS 1 through 3 show significant “jump in permeability values” at
 1279 Time Step 1 when the second quasi-static loading commences.

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1285 **Tables**

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1287 **Table 1.** Essential model parameters applied for stress calculations. Adapted from Bertotti et
 1288 al. (2017)

		Average data	Data range
Surface Tension (Jm^{-2})	Y	0.27	0.27
Young's Modulus (GPa)	E	25	15 – 45
Poisson's ratio	ν	0.3	0.25 – 0.3
Rock density (Kgm^{-3})	ρ	2200	2000 – 2700
Tectonic Stress (MPa)		10	0 – 35

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1292 **Table 2.** Effective permeabilities, including permeability Tensor obtained at different loading
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		Effective Permeability (K_{eff}) $K \times 10^{-15} \text{ (m}^2\text{)}$		
		Matrix permeability = 2.00		
	Time Steps	K_{xx}	K_{yy}	K_{xx}/K_{yy}
FSS 1	1	2.14	2.20	0.98
FSS 2	1	2.30	2.18	1.05
FSS 3	1	3.10	2.60	1.19
FSS 4	1	3.40	3.70	0.92
FSS 5	1	2.80	2.54	1.10
Average	1	2.75	2.64	1.04
FSS 1	2	2.28	2.70	0.84
FSS 2	2	2.42	2.56	0.95
FSS 3	2	3.28	3.90	0.84
FSS 4	2	3.50	4.20	0.83
FSS 5	2	2.36	2.84	0.83
Average	2	2.77	3.24	0.85