

1 **Physically based Probabilistic Rainfall Intensity-Duration (ID) Thresholds for**
2 **Runoff-Generated Debris Flows**

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21 **Key Points:**

- 22 • **Calibration of numerical models using multiple debris flow events reveals the**
23 **variation of catchment parameters.**
 - 24 • **Infiltration controls the shape of rainfall intensity-duration thresholds for runoff-**
25 **generated debris flows in carbonate-dominated catchments**
 - 26 • **Ensemble of thresholds can estimate the probability of debris flows for a given storm,**
27 **improving compliance with early warning systems**
- 28

29 **Abstract**

30 Runoff-generated debris flows are common hazards in mountainous regions, causing millions of
31 dollars lost and hundreds of casualties yearly. Early warning systems based on rainfall thresholds
32 have been implemented to reduce the impact of these hazards. These thresholds tend to be based
33 on short monitoring periods, which cannot fully capture the varying responses of catchments to
34 rainfall. As a result, the uncertainty of many thresholds is unknown, limiting their usefulness to
35 the general public. We propose a new modelling framework to derive probabilistic rainfall
36 intensity-duration (ID) thresholds from limited observations. We test this framework on a small
37 catchment in the Italian Dolomites to determine probabilistic thresholds for the occurrence of
38 debris flows. Instead of a widely used power-law function, our new rainfall thresholds are a
39 negative exponential function controlled by infiltration capacity. These probabilistic thresholds
40 can help improve early warning system performance by providing additional information to the
41 public.

42 **Plain Language Summary**

43 Debris flows, a very fast and mobile form of landslide, are a common hazard in mountainous areas.
44 To reduce casualties and the economic cost of these hazards it is important that at risk areas have
45 reliable early warning systems. The accuracy of an early warning system is strongly linked to the
46 length of time the area has been monitored. The longer the area has been monitored for, the more
47 debris flows are recorded resulting in more accurate warning systems. However, many debris flow
48 prone regions have limited records of past debris flow events resulting in unreliable warnings.
49 Here we present a new method for producing debris flow forecasts based on short monitoring
50 periods using a numerical model framework. We test this framework on a small simple catchment
51 in the Italian Dolomites with a multi-year record of debris flows. The framework produces many
52 possible scenarios allowing for the probability of a debris flow being triggered to be calculated.
53 These probabilities can be used to produce highly accurate classifications of past debris flow
54 events. A warning system based upon this framework can use these probabilities to provide
55 valuable information to the public enhancing compliance with the system.

56 **1 Introduction**

57 Debris flows, high-speed surges of poorly sorted sediment saturated with water, are one of
58 the most common hazards in mountainous regions (Dowling & Santi, 2014; Fan et al., 2019;
59 Hürlimann et al., 2019; Iverson, 1997). These flows regularly damage local infrastructure such as
60 roads, rail networks, and waterways (Alessio et al., 2021; Bainbridge et al., 2022; Horton et al.,
61 2019; Huang & Fan, 2013; Iverson et al., 2011). Globally between 1950 and 2011, 77,759
62 casualties were attributed to the direct effects of debris flows, with two events causing over half
63 of the recorded casualties (Dowling & Santi, 2014). The large number of casualties demonstrates
64 the need for accurate early warning systems and engineering measures in debris-flow-impacted
65 areas.

66 Critical to reducing the economic and human cost of debris flows is the issuing of timely
67 warnings to local communities. These warnings can help ensure homes and businesses are
68 evacuated and transportation routes are closed during times of high risk (Bainbridge et al., 2022;
69 Hürlimann et al., 2019). For effective warnings, an understanding of debris flow triggering
70 conditions is required. Debris flows typically occur in steep mountainous areas during intense
71 rainfall where they can evolve either from landslides or from surface runoff entraining sediment

72 within channels (Bennett et al., 2013; Iverson, 1997; Iverson & George, 2016; Kean et al., 2013;
73 Takahashi, 1981). The triggering conditions of landslide-generated debris flows are relatively well
74 understood and can be estimated through a slope stability analysis (Dietrich & Montgomery, 1998;
75 Guzzetti et al., 2008). Runoff-generated debris flows, on the other hand, are not well understood.
76 Debris flow prone areas have been monitored by a combination of rainfall gauges, passive seismic
77 monitoring, and video camera recording in order to identify their triggering conditions (Badoux et
78 al., 2009; Hürlimann et al., 2019; Kean et al., 2013). However, debris flow initiation is rarely
79 directly observed, and as a result, the exact timing and processes that triggered a particular debris
80 flow are not often known (Benda & Dunne, 1997; Hirschberg et al., 2021; Iverson, 1997; Neely &
81 DiBiase, 2023; Prancevic et al., 2014). Without such information, runoff-triggering debris flows
82 remain challenging to forecast.

83 An effective early warning system must provide sufficient warning so that communities
84 and businesses have time to organize and prepare (Badoux et al., 2009; Jakob et al., 2012; LeClerc
85 & Joslyn, 2015; Roulston & Smith, 2004). To produce the earliest warning possible most systems
86 rely upon rainfall forecasts (Bernard & Gregoretti, 2021; Berti et al., 2020; Hirschberg et al., 2021;
87 Hürlimann et al., 2019). These systems will issue warnings if the expected rainfall is likely to
88 exceed the intensity of previously recorded debris flow triggering storms (Cannon et al., 2001;
89 Coe et al., 2008; Hürlimann et al., 2019; Staley et al., 2013). The identified critical rainfall intensity
90 is often a function of rainfall duration and is called the rainfall intensity-duration (ID) threshold
91 (Guzzetti et al., 2008; Hirschberg et al., 2021; Iverson, 2000). These thresholds are derived from
92 statistical analysis of observed debris flow triggering rainfall events. Generally, these thresholds
93 are assumed to take the form of a power law function described as:

$$94 \quad I = \alpha D^{-\beta} \quad (1)$$

95 where I represents rainfall intensity, D is storm duration, and α and β are empirical constants (Berti
96 et al., 2020). For these warning systems, the accuracy of this threshold is critical to their success.
97 Poorly performing systems can generate mistrust within the local community through false alarms
98 or by failing to issue a warning of a debris flow potentially leading to casualties (Badoux et al.,
99 2009; LeClerc & Joslyn, 2015; Roulston & Smith, 2004). Determining accurate thresholds can
100 require a large number of recorded debris flows. For example, in the Illgraben catchment in the
101 Swiss Alps, twenty-five debris flow events are required to produce a threshold that accurately
102 classifies 70% of rainfall events (Hirschberg et al., 2021). Twenty-five debris flows are
103 significantly more than have been recorded in almost any other debris flow catchment (Hirschberg
104 et al., 2021; Hürlimann et al., 2019). Therefore, there is a need to produce accurate, testable rainfall
105 ID thresholds from a small number of observations.

106 Numerical models can be used to reduce the number of recorded debris flow events
107 required for accurate thresholds (Gregoretti & Dalla Fontana, 2008; Tang et al., 2019a). Model-
108 derived thresholds do not require long observational periods to be determined; instead, they can
109 be generated from a model calibrated on a single event. However, thresholds determined by this
110 methodology rarely consider temporal variability in the catchment. The rainfall intensity required
111 to trigger a debris flow has been shown to vary through time, primarily due to changes in the
112 infiltration capacity and sediment availability of the catchment (Guo et al., 2016; Hürlimann et al.,
113 2019; Raymond et al., 2020; Thomas et al., 2021). Considering the natural variability within the
114 numerical model allows for a number of possible scenarios to be generated and probabilistic debris
115 flow forecasts to be derived. Probabilistic forecasts have been shown to increase compliance with

116 warning messages and are more adaptable than traditional empirical thresholds (Beguería, 2006;
117 LeClerc & Joslyn, 2015; Roulston & Smith, 2004).

118 Here, we present a new modelling framework to derive probabilistic rainfall intensity-
119 duration threshold functions of a small catchment in the Italian Dolomites. With this framework,
120 we calibrate the SWEHR model developed by McGuire et al. (2016, 2017) on four debris flow
121 events to determine the range of possible parameter values. By sampling from this parameter space
122 using a Monte Carlo scheme, we can derive the rainfall ID threshold function of runoff-generated
123 debris flows. Finally, we use this constrained variation to generate an ensemble of one thousand
124 possible rainfall ID thresholds to produce probabilistic thresholds of runoff-generated debris flows.

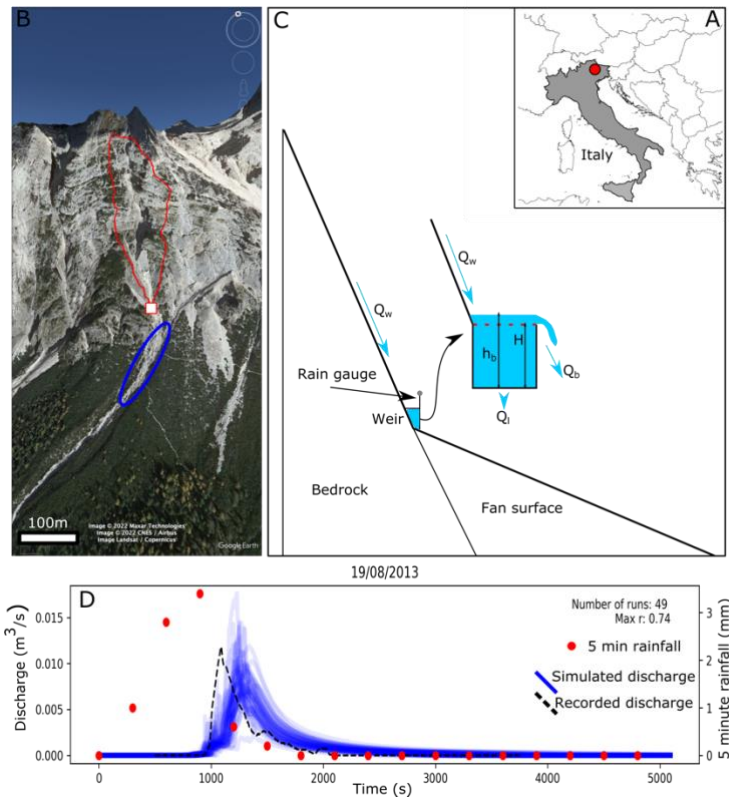
125 **2 Study area**

126 Our study site is located in the Venetian Dolomites in the north-eastern Italian Alps (Figure
127 1A), which are composed of a thick fractured dolomitized carbonate rock known as the Dolomia
128 Principale Formation (Berti et al., 2020; Berti & Simoni, 2005; Gregoretti et al., 2016, 2018;
129 Simoni et al., 2020). This lithology forms the steep, picturesque, rocky cliffs which make up the
130 majority of the headwater catchments in the region (Figure 1B). At the foot of these cliffs are post-
131 glacial scree slopes, which are dissected by debris flow channels. Debris flows here are primarily
132 triggered when runoff funneled in from the headwater cliffs mobilizes loose sediment accumulated
133 in the channel (Berti et al., 2020; Gregoretti et al., 2016).

134 This study focuses on a single catchment, Dimai, located two kilometers north of Cortina
135 d'Ampezzo in Boite Valley. The Dimai catchment is small (97,880m²) and consists of two main
136 units. The upper headwater part of the catchment (30,440m²) is formed of the steep (average slope
137 63°) western slope of the Pomagagnon massif, and its main channel runs through a forested scree
138 fan (Figure 1B) (Berti et al., 2020; Gregoretti et al., 2016). For this study, we focus on modelling
139 the runoff generation in the headwater catchment to determine its rainfall ID threshold function.
140 The relative simplicity and small size of the headwater catchment make it an excellent location for
141 our study. The lack of significant sediment transport reduces the computational load required for
142 each simulation, allowing a Monte Carlo simulation scheme to be performed. In addition, as the

143 catchment has been monitored and modelled in the past, many major hydrological model
 144 parameters have been constrained (Gregoretti et al., 2016; Berti et al., 2020).

145



146
 147 *Figure 1: The study area, monitoring setup, and an example of the field observation and model results. (A) The location of the*
 148 *study area. (B) The Dimai headwater catchment (red) shown in Google Earth satellite imagery superimposed on a DEM. Debris*
 149 *flow initiation site is outlined in blue, and the location of the monitoring site is indicated by the white square. (C) A diagram of the*
 150 *monitoring setup adapted from Berti et al., 2020. The monitoring station is located at the base of the headwater catchment and*
 151 *consists of a purpose-built weir, rain gauge, video cameras, and pressure gauges. (D) An example of a calibration exercise, the*
 152 *discharge record is shown as a black dashed line, precipitation is shown as red points, the simulated discharge is shown in blue.*
 153 *There are 49 simulation runs shown here, each with a Pearson's correlation coefficient (r) of greater than 0.5 and a maximum of*
 154 *0.74.*

155 In 2010, a monitoring station was set up in Dimai to observe debris flow initiation (white
 156 box in Figure 1B). This monitoring station used a weir, cameras, and a rain gauge to measure the
 157 triggering rainfall and resulting discharge and determine whether a debris flow was triggered.
 158 Figure 1C shows the basic setup of the monitoring station. See supplemental information and Berti
 159 et al., 2020 for more detail.

160 The monitoring station was active between 2010 and 2016 and recorded five debris flows,
 161 four of which we have the complete discharge and rainfall records required for calibration. All
 162 four debris flows occurred during August, highlighting summer convective storms as an important
 163 process in triggering debris flow in the area (Berti et al., 2020; Gregoretti et al., 2016). The
 164 recorded events are between 15 and 120 minutes long and show steep rising and falling limbs in
 165 their discharges, indicating rapid response times and low storage capacity in the catchment (Figure
 166 1D) (Berti et al., 2020). Peak flow depth typically ranges from 20 to 40 cm and can erode several
 167 tens of centimeters from the debris flow channel bed. For our study the catchment is represented

168 by one-meter resolution raster DEM derived from a lidar survey performed in October 2011
169 (Gregoretti et al., 2016).

170 **3 Methodology**

171 3.1 Numerical model

172 We simulate the runoff response of the catchment using the SWEHR model (McGuire et
173 al., 2016, 2017). The model is comprised of two main components: fluid flow and sediment
174 transport. Since sediment availability is minimal in the headwater catchment, we only focus on
175 calibrating the fluid flow governing equations. Rainfall is converted into runoff via infiltration and
176 interception models before being routed through the catchment using a set of conservation laws
177 with nonlinear shallow water equations (McGuire et al., 2016). Previous studies (Berti et al., 2020;
178 Gregoretti et al., 2016) demonstrated the importance of infiltration in Dimai for controlling the
179 magnitude of the generated runoff. This process is simulated in the model with the Green-Ampt
180 equation. In the model we use separate calibrated parameters for the saturated conductivity of
181 sediment cover and bedrock. We also calibrate Manning's roughness coefficient within the shallow
182 water equations.

183
184 The saturated conductivity of the bedrock and sediment controls the rate at which water
185 can infiltrate into the surface (m/s). At higher saturated conductivity values, the water can pass
186 more easily into the subsurface, resulting in a lower runoff volume. Manning's roughness
187 coefficient influences the velocity of the resulting surface runoff. We provide more detail on the
188 Green-Ampt equation in the supplemental information. For more detail on the other parameters of
189 the model, we direct the reader to McGuire et al. (2016).

190 3.2 Model calibration

191 We want to identify and constrain any variation within these parameters therefore, we
192 calibrate the model separately on each of the four recorded discharge events using a Monte Carlo
193 framework. As the interactions and dependencies between the parameters are not known, we use
194 this framework to identify a distribution of possible parameter values for each event. These
195 parameter distributions can then be combined to produce a parameter space representing the
196 catchment variation over the monitoring period.

197
198 The framework calibrates the model's parameter space by comparing the simulated and
199 recorded discharge of a known storm. The framework generates one hundred model runs for each
200 recorded storm, each with a unique parameter set. Using Pearson's correlation coefficient, the
201 parameter space is refined by eliminating parameter sets that produce simulations with a
202 coefficient of less than 0.5. The value of 0.5 is chosen as this represents a good correlation between
203 the simulation and recorded event while ensuring some variation in the parameter sets is retained.
204 The correlation coefficient does not capture the magnitude of the discharge, an essential control
205 on debris flow initiation (Gregoretti & Dalla Fontana, 2008; Prancevic et al., 2014; Tang et al.,
206 2019a). Therefore, the framework further refines the parameter space by discarding simulated
207 discharges with a peak of less than 50% of the maximum discharge of the recorded event. The
208 framework then iterates again with a refined parameter space set by a two-standard deviation range
209 of the retained parameter sets. The framework will continue to iterate and refine the parameter
210 space until each rainfall event has 50 retained parameter sets or for five iterations. These iteration

211 limits balance the need to capture the full distribution of the parameter space with computational
 212 time. The retained parameter sets for each recorded event are combined to produce the full
 213 parameter space for the catchment.

214 3.3 Hydrodynamic thresholds and Rainfall ID thresholds

215 Debris flows are triggered when runoff exceeds a critical threshold and rapidly entrains
 216 sediment (Gregoretti & Dalla Fontana, 2008; Lamb et al., 2008; Prancevic et al., 2014; Tang et al.,
 217 2019b). With our calibrated model and parameter space, we can identify this threshold and
 218 determine its relationship to rainfall intensity and duration. This methodology provides us with
 219 physically based rainfall ID thresholds that can be tested against known debris flow events (Tang
 220 et al., 2019).

221
 222 Following Tang et al., (2019a), we use two hydrodynamic metrics to calculate the rainfall
 223 ID threshold: Shields stress and dimensionless discharge. Shields stress is calculated as:

$$224 \tau_* = \frac{(\rho_w h S_f)}{(\rho_s - \rho_w) D_{50}} \quad (2)$$

225 where ρ_w is the density of water, ρ_s is the density of sediment, h is flow depth, D_{50} is the median
 226 grain size, and S_f is the friction slope $S_f = n_0^2 (uh^2 + vh^2) h^{-10/3}$, where n_0 is Manning's roughness
 227 coefficient and u and v are the velocity in the x and y directions, respectively (Tang et al., 2019a).
 228 Shields stress describes the excess stress that acts on grains in the channel pulling it downslope
 229 (Takahashi, 1981; Prancevic et al., 2014). The dimensionless discharge is defined as:

$$230 q_* = \frac{q}{\sqrt{\frac{\rho_s - \rho_w}{\rho_w g}}} \quad (3)$$

231 where q is the flow discharge leaving the outlet defined by the product of the water stage and
 232 velocity, and g is gravity. τ^* and q^* are both functions of the channel slope and, therefore, can be
 233 compared to other catchments (Gregoretti & Dalla Fontana, 2008; Tang et al., 2019a).

234
 235 As debris flows are not directly observed within our model domain, we assume the debris
 236 flow is triggered by the peak outlet τ^* or q^* . We derive a distribution of τ^* and q^* thresholds from
 237 the simulations retained from the model calibration. To derive the rainfall ID threshold of the
 238 catchment, we need to identify a wide range of rainfall events which can trigger a debris flow. We
 239 generate a matrix of 1000 rainfall events bounded by the historical rainfall record of the catchment.
 240 The matrix of rainfall events is equally spaced in log space for intensity and duration. The rainfall
 241 within these events is normally distributed through time and uniformly across the catchment.
 242 Gregoretti et al., (2016) showed that rainfall intensity does not vary significantly across Dimai due
 243 to its small size. The framework draws a unique parameter set for each rainfall event and collects
 244 the resulting simulated discharge. Finally, the framework calculates τ^* and q^* of each event. These
 245 are compared with the distribution of critical hydrodynamic values to derive the rainfall ID
 246 threshold.

247
 248 With the rainfall matrix and distribution of critical hydrodynamic values, we generate an
 249 ensemble of one thousand potential rainfall ID thresholds for the catchment. First, one hundred
 250 rainfall events are sampled from the rainfall matrix using a uniform distribution. Next, ten values
 251 from the critical hydrodynamic value distributions are chosen to determine possible rainfall ID
 252 thresholds. Using an interpolation algorithm (matplotlib.pyplot.tricontour (Hunter, 2007)), a phase
 253 space of the hydrodynamic metrics is generated for each sample of rainfall events. The

254 interpolation algorithm then draws a contour of the sampled τ^* or q^* values through this space.
 255 Finally, the rainfall ID threshold is determined by fitting a negative exponential function to this
 256 contour. This shape is chosen based on visual interpretation. In order to test the accuracy of the
 257 generated thresholds, we use the F1 accuracy score comparing the predictions of the threshold with
 258 the recorded debris flow events (see supplemental information for more details).

259 **4 Results**

260 4.1 Model Calibration

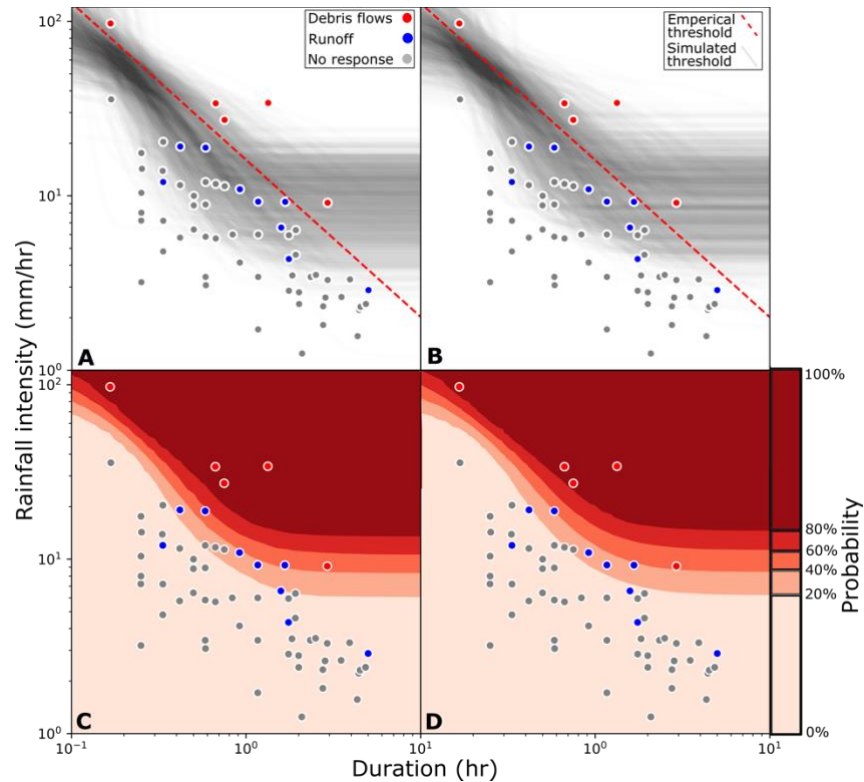
261 With our Monte Carlo framework, we successfully calibrated the model for all four debris
 262 flow events (Figure S1). After running the framework on each rainfall event, 174 calibrated
 263 parameter sets were collected and combined to produce a single parameter space. Other than the
 264 calibration of the 24th August 2013 debris flow event, the framework retained a minimum of 49
 265 parameter sets. The calibration of the debris flow event on 24th August 2013 ended after five
 266 iterations resulting in a low number (11) of retained parameter sets. The resulting parameter space
 267 is relatively tight, while saturated conductivity in other locations can vary by several orders of
 268 magnitude, our parameter space is limited to a single order (Table S1). From the retained
 269 simulations we find that both the Shield's stress and dimensionless discharge critical values for
 270 debris flow initiation are log-normally distributed. However, the dimensionless discharge values
 271 have a larger standard deviation than the Shields stress values (5.31 v.s. 3.03) (Figure S2).

272 4.2 Rainfall Intensity-Duration (ID) Thresholds

273 From our τ^* and q^* phase spaces, we can see that the hydrodynamics of generated runoff
 274 are controlled by rainfall duration and intensity (Figure S3). Very short rainfall events generate no
 275 runoff, while low-intensity events only produce runoff during long-duration events. Contours of
 276 τ^* and q^* close to the minimum duration of runoff generation are steep and then rapidly flatten out
 277 to a critical intensity (Figure S3). This relationship is best described as a negative exponential
 278 rather than a power law (Figure 2). Our rainfall ID thresholds take the form:

$$279 \quad I = ae^{-bD} + c \quad (4)$$

280 where I is the rainfall intensity required to generate the runoff hydrodynamic metric as a function
 281 of rainfall duration D . a , b , and c are empirical parameters. Parameter a determines where the
 282 threshold passes through the y-axis. The b parameter controls the rate at which the rainfall intensity
 283 changes as a function of the rainfall event duration. And c controls the minimum intensity required
 284 to exceed the threshold and is strongly correlated with the critical τ^* and q^* chosen to generate the
 285 threshold. τ^* consistently produces rainfall ID thresholds with higher intensities than q^* , resulting
 286 in a levelling off of the function at a higher point on the y-axis (Figures 2 and S2).

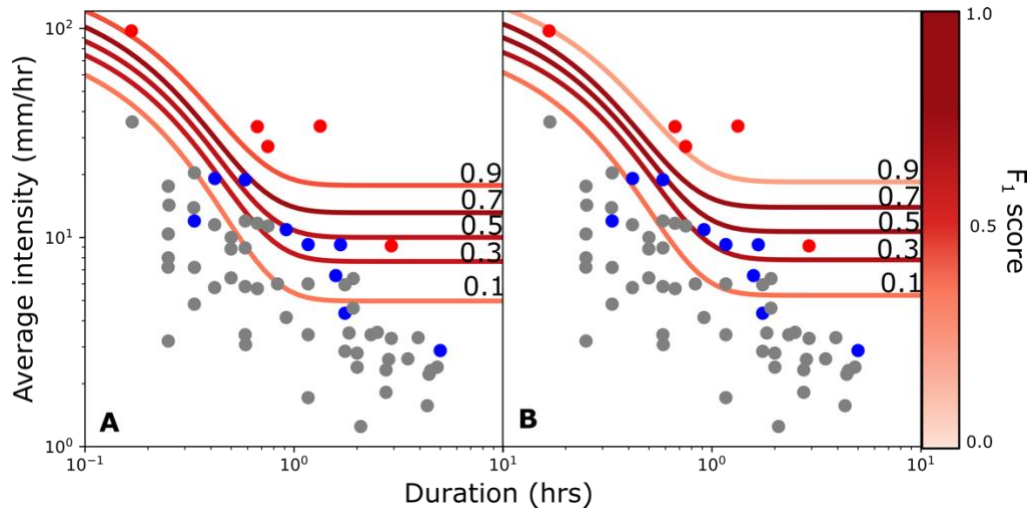


287
 288 *Figure 2: The model-generated rainfall intensity-duration (ID) thresholds. Thresholds generated with q^* are on the left while τ^* is*
 289 *on the right. In A and B the grey lines are rainfall ID thresholds generated by the Monte Carlo framework. Darker areas indicate*
 290 *parts where many thresholds overlap. The points show the intensity, duration, and runoff response of recorded rainfall events. The*
 291 *red dashed line is the empirical threshold from Berti et al. 2020. C and D show the debris flow probability phase space of the*
 292 *catchment. This spans storms which are above a rainfall ID threshold >80% of the time (dark red) to <20% (pale orange). In all*
 293 *four panels the τ^* thresholds level out at slightly higher intensities than the q^* .*

294 We combine the ensemble of possible rainfall ID thresholds to estimate the probability of
 295 a particular storm triggering a debris flow (Figure 2C and 2D). Storms with high average intensity
 296 and long durations are highly likely to be classified as debris flow triggering (top right in Figures
 297 3a and 3b). In contrast, low-intensity and short-duration storms have a very low probability of
 298 triggering debris flows (bottom left in Figures 2C and 2D). The contour lines roughly follow power
 299 law relationships until the rainstorm duration exceeds one hour. After this duration, the contour
 300 lines become nearly flat. The contour maps based on τ^* and q^* are very similar, but the former
 301 levels out at higher intensities (~ 5 mm/hr) on average (Figures 2C and 2D).

303 Finally, we calculate the F_1 scores of probability thresholds to evaluate our modelling
 304 framework's ability to classify debris flow triggering rainfall events. We find that probability
 305 thresholds close to 0.5 produce the highest F_1 scores (Figure 3). False positives are common at
 306 lower thresholds, while higher thresholds produce false negatives resulting in lower F_1 scores
 307 (Figure S4). While both τ^* and q^* follow similar trends, there is a slight difference in the peak F_1
 308 scores and probability threshold required to achieve this score. τ^* has a higher maximum F_1 score
 309 (0.94 and 0.87) and reaches this at a lower probability threshold (0.48 and 0.59). The minimal
 310 difference between the two metrics suggests either can be used to classify debris flow triggering
 311 storms in the Dimai catchment.

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Figure 3. Five probability thresholds colored by their F_1 score A) show thresholds derived from dimensionless discharge and B) are from Shields stress. The probability value of the threshold is shown on the right-hand side of the threshold. Light colors indicate a low F_1 score, while dark colors indicate thresholds with more predictive power. Probability thresholds around 0.5 produce the highest F_1 scores.

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319 5 Discussion

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Using a numerical model within a Monte Carlo framework, we derived probabilistic rainfall intensity thresholds for runoff-generated debris flows. The framework used four debris flow events to constrain the variation of a multi-year period within the Dimai catchment. The framework uses this constrained variation and 1000 synthetic rainfall events to derive an ensemble of potential rainfall ID thresholds. This ensemble is then combined to produce probability thresholds, which are tested against the recorded debris flow events. Finally, we find these thresholds are best described by a negative exponential function rather than by a power law, as is commonly assumed (Berti et al., 2020; Guzzetti et al., 2008; Hirschberg et al., 2021; Hürlimann et al., 2019).

The calibration process retains >50 simulations for three of four recorded debris flow events before the iteration limit is reached (Figure S1). For the debris flow event on 24th August 2013, only eleven parameter sets were retained before the iteration limit was reached. Using the identified parameter space, the model cannot consistently reproduce the magnitude of the peak discharge, suggesting infiltration was lower for this event. Infiltration is controlled by two parameters (see supplemental text 2), saturated conductivity and the water content of the subsurface. For each simulation, we assumed that the initial water content was the same; however, an initially wetter catchment could produce more runoff than expected (Dunne, 1983; Mein & Larson, 1973). Five days before 24th August 2013, another debris flow occurred. If it rained during the intervening days or the catchment did not completely drain, a higher initial water content could have occurred. This result has been observed in other debris flow catchments, highlighting the importance of considering the variability in hydrological conditions of the catchment in model calibrations (Jakob et al., 2005; Simoni et al., 2020).

The rainfall ID thresholds derived for the Dimai catchment can be separated into two sections, shorter and longer than one-hour duration. Prior to this duration, the thresholds follow a

345 negative power law, while afterwards, the threshold flattens out (Figures 2 and 3). This indicates
346 that, for most rainfall events, the average intensity is the main controlling factor on the stress and
347 discharge of the generated runoff. When controlling for rainfall event intensity, we find that the
348 τ^* and q^* of the generated runoff hydrodynamics increase with duration until the duration is close
349 to one hour (Figure S5). After this point, the hydrodynamic metrics become steady and remain
350 close to the selected intensity's median. Lower-intensity storms, however, take longer to reach
351 stability indicating the influence of the initial conditions on the resulting hydrodynamic metrics.
352 From our calibration phase, we know that a particular storm's total volume and peak runoff, and
353 therefore the hydrodynamics, is a function of its saturated conductivity (Figure S6). Therefore, the
354 inflection point from a power law relationship represents the time required for the storm to
355 overcome its initial infiltration conditions. After this inflection point, the resulting hydrodynamic
356 metrics of a storm are only controlled by its intensity. This result further extends the work done
357 by Berti et al. 2020 by showing that infiltration affects not only the y-axis intersection of the
358 threshold but also the overall function used to describe the threshold. Therefore, in areas where
359 large changes in saturated conductivity can occur, such as following wildfires (Raymond et al.,
360 2020; Tang et al., 2019b; Thomas et al., 2021), a change in the threshold function must be
361 considered.

362
363 With our probabilistic thresholds, we can classify storms that trigger debris flows with a
364 high degree of accuracy. The maximum F1 score of 0.95 represents a near-perfect classification,
365 though at a relatively low probability threshold of ~ 0.5 (Figures 4 and S4). Over time the accuracy
366 of this threshold will likely decrease as an increase in the number of false positives is expected.
367 As compliance with a warning system is linked to its performance (LeClerc & Joslyn, 2015;
368 Roulston & Smith, 2004), practitioners should regularly update the thresholds to maintain
369 performance. Our framework improves on existing methodologies in two crucial ways. First, it
370 significantly reduces the time and data required to produce reliable rainfall ID thresholds for debris
371 flow forecasts with reasonable accuracy. This has the potential to greatly improve hazard response
372 in the aftermath of earthquakes and wildfires. Secondly, the framework allows context to be added
373 to warning systems increasing trust in the system. Probabilistic warning thresholds can be easily
374 adjusted for different stakeholders while ensuring compliance rates remain consistent through
375 time.

376 **6 Conclusions**

377 Here we present a new modelling framework to generate probabilistic rainfall intensity-
378 duration thresholds for runoff-generated debris flows. We apply this framework to the Dimai
379 catchment in the Italian Dolomites. By calibrating the numerical model on four separate debris
380 flow events, we constrain the variation in catchment runoff response. This variation is then
381 sampled from to generate many possible rainfall ID thresholds, which are then combined to
382 produce probabilistic classifications for the debris flow record. The thresholds are best described
383 by a negative exponential function indicating the importance of the catchment infiltration for
384 debris flow generation in carbonate catchments. The probabilistic thresholds are highly accurate
385 and can provide vital context to early warning systems, potentially increasing trust and compliance
386 with the warning system.

387

388 **Acknowledgments**

389 Jens Turowski and Erin Harvey read earlier versions of this manuscript and provided comments
390 which greatly improved the paper.

391 **Open Research**

392 The data from the Dimai monitoring site can be found is publically available in the supporting
393 information of Berti et al 2020. The calibration framework, analysis, and visualization Python
394 scripts and notebooks are publically available from <https://zenodo.org/record/8163997>. Here you
395 can also find the data used to produce the figures in this paper.

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Supporting Information for

Physically based Probabilistic Rainfall Intensity-Duration (ID) Thresholds for Runoff-Generated Debris Flows

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Introduction

Here you will find the supplemental text, figures, and table referred to in the main text of the paper. Supporting Text S1 provides further information on the monitoring station at Dimai, in particular we provide more detail on how discharge is measured. Supporting Text S2 provides more information on the Green-Ampt infiltration model. Supporting Text S3 details the F_1 score used to measure the accuracy of the thresholds derived by the modelling framework.

Text S1.

Here, we briefly describe the monitoring set up at Dimai, as shown in Figure 1C. For a more complete description and photos of the monitoring setup we direct the reader to Gregoretti et al., 2016 and Berti et al., 2020. The monitoring station consisted of a weir and two rain gauges (one at the weir one at the top of the headwater catchment) and was supported by video cameras for confirmation of debris flow events. The weir was used to measure the discharge leaving the headwater catchment during rainfall events while the rain gauges captured the intensity and duration of the rainfall event. Rainfall intensity was measured using a standard tipping-bucket rain gauge with a sensitivity of 0.2mm. The rain gauge starts recording once the rainfall intensity exceeds 0.2mm/hr. The duration of the rainfall event is defined by the length of time the rainfall remains above this threshold. The rainfall intensity is summed and recorded for five-minute intervals.

Discharge exiting the headwater catchment was calculated by recording the change in volume of a stilling basin downstream of a purpose-built weir (Figure 1C). The discharge was estimated by calculating the change in volume of the basin through time:

$$Q_w(t) = \frac{dV_b}{dt} + Q_l + Q_b \quad (1)$$

where Q_w is the volume of water in the basin as a function of time (t), dV_b/dt is the change in the volume of water in the basin, Q_b is the discharge of water leaving the basin above the weir wall, and Q_1 is the discharge from the incompletely sealed basin base ($\sim 0.6 \times 10^{-4} \text{ m}^3/\text{s}$). The height of the water in the stilling basin was measured with a pressure sensor located in its base. The water height is then combined with the height and width of the weir to determine Q_b :

$$Q_b = C_D B_E \sqrt{2g} (h_b - H)^{\frac{3}{2}} \quad (2)$$

Here C_D is a discharge coefficient (0.4). B_E is the effective weir width ($1.55 - 0.1(h_b - H)$), where h_b is the height of the water table, and H is the height of the weir wall. The temporal resolution of these measurements is five minutes, the same as the rain gauges. When rainfall events are recorded, video cameras are activated to determine whether a debris flow is triggered.

Text S2.

Infiltration has been shown to be an important control on the final discharge exiting the headwater basin at Dimai. Within the SWEHR model, infiltration is modelled by the Green-Ampt equation which includes the calibratable parameter, saturated conductivity of the surface. As this model has a strong impact on our simulation results, we describe it here. Infiltration (I_c) is calculated by:

$$I_c = K_s \frac{Z_f + h_f + h}{Z_f} \quad (3)$$

where K_s is saturated conductivity, Z_f is the wetting front depth, h_f is the wetting front capillary pressure head, and h is the pressure the runoff depth acting on the saturated zone below the surface. Saturated conductivity is the speed at which the water can pass through the saturated medium. Z_f is the depth from the surface, which marks the transition between the saturated and unsaturated zones. h_f describes the pressure on the water resulting from the suction of the unsaturated pore spaces below. Z_f is calculated as:

$$Z_f = \frac{I_d}{\theta_s - \theta_i} \quad (4)$$

where I_d is the cumulative infiltrated depth, θ_s and θ_i are the saturated and initial volumetric water content, respectively. Within the SWEHR model, we can provide separate saturated conductivities for bedrock and sediment. Hence, we need to calibrate both saturated conductivities and Manning's roughness coefficient. The initial ranges of parameters are constrained from previous work on the catchment and can be viewed in Table S1.

Text S3.

In order to test the accuracy of our modelling framework, we compare our probabilistic thresholds with the historical record of rainfall events from the monitoring station. Here, we use the F_1 score, which is defined as the harmonic mean of a given threshold's precision and recall:

$$F_1 = \frac{R+P}{2} \quad (5)$$

Precision is calculated by:

$$P = \frac{TP}{TP+FP} \quad (6)$$

where TP is the number of true positives, i.e., a storm that has been correctly classified as debris flow triggering, and FP is the number of false positives, i.e., a storm that has been incorrectly predicted as debris flow triggering. The recall is calculated by

$$R = \frac{TP}{TP+FN} \quad (7)$$

where FN is the number of false negatives, storms incorrectly classified as not triggering a debris flow. The final F_1 score varies between zero and one, where one is a perfect classification with no false classifications, and zero represents no predictive power. By calculating the F_1 score for different probabilities, we can determine the predictive power of the framework.

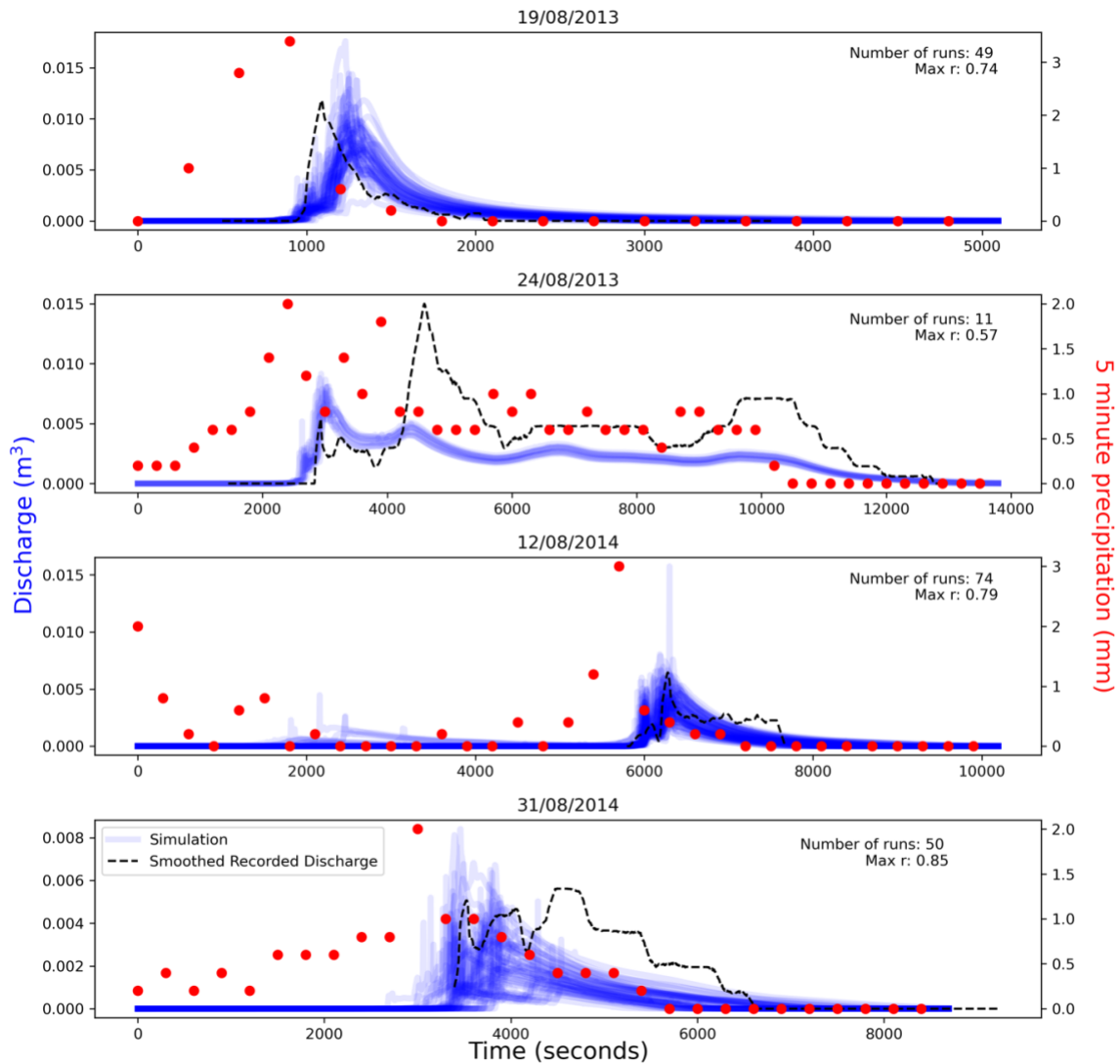


Figure S1. Results of the calibration process for the four debris flow events. For each event, we show the recorded discharge as a black dashed line and the recorded precipitation as red dots. The simulations resulting from the calibration process are shown in blue, and areas where many cases overlap are shown as darker. We only show the simulations that are deemed well calibrated, i.e., they have a Pearson's Correlation coefficient (r) greater than 0.5 and a maximum discharge greater than half the recorded discharge. In each panel, we list the number of runs shown in the figure and the maximum Pearson's correlation coefficient of these runs (r).

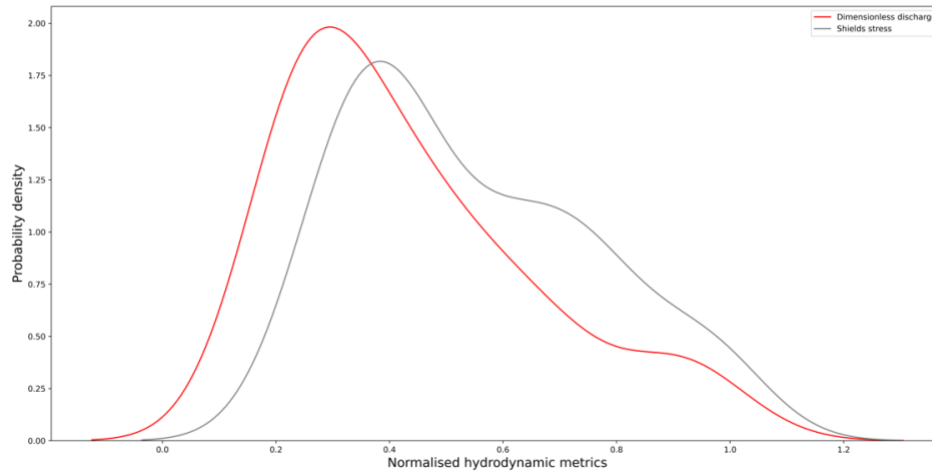


Figure S2. Kernel density plots of the distributions of Dimensionless discharge and Shields stress derived from the maximum values of the well-calibrated cases. The values are normalized by dividing the distribution by its maximum value.

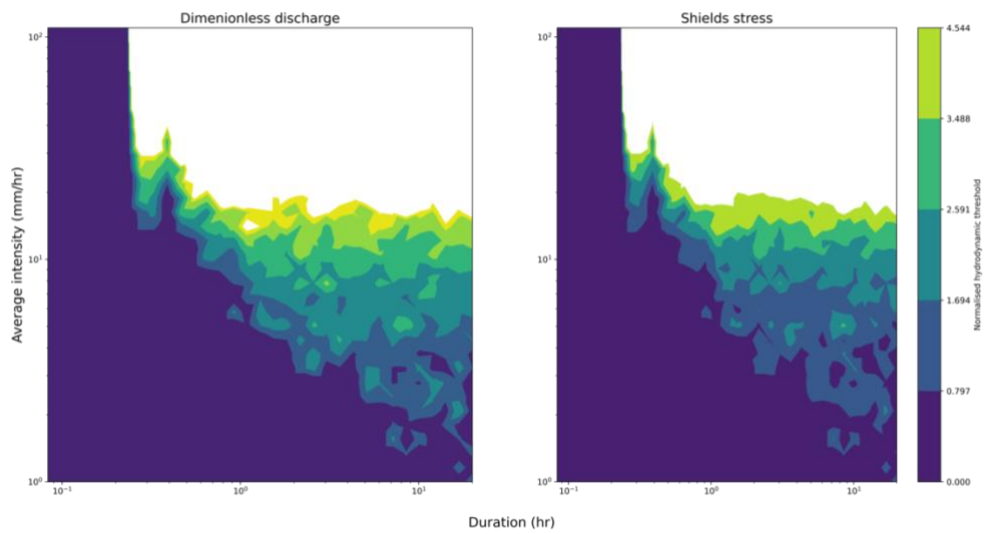


Figure S3. Contour plots of dimensionless discharge and Shields stress derived from the thousand simulated rainfall events using the tricountorf Python package. The color bar is normalized by the median value of each hydrodynamic metric. White areas produce hydrodynamic metrics greater than two standard deviations from the median critical value used to derive the rainfall intensity-duration thresholds. The contours are complex due to the variation in input parameters used to derive the simulations.

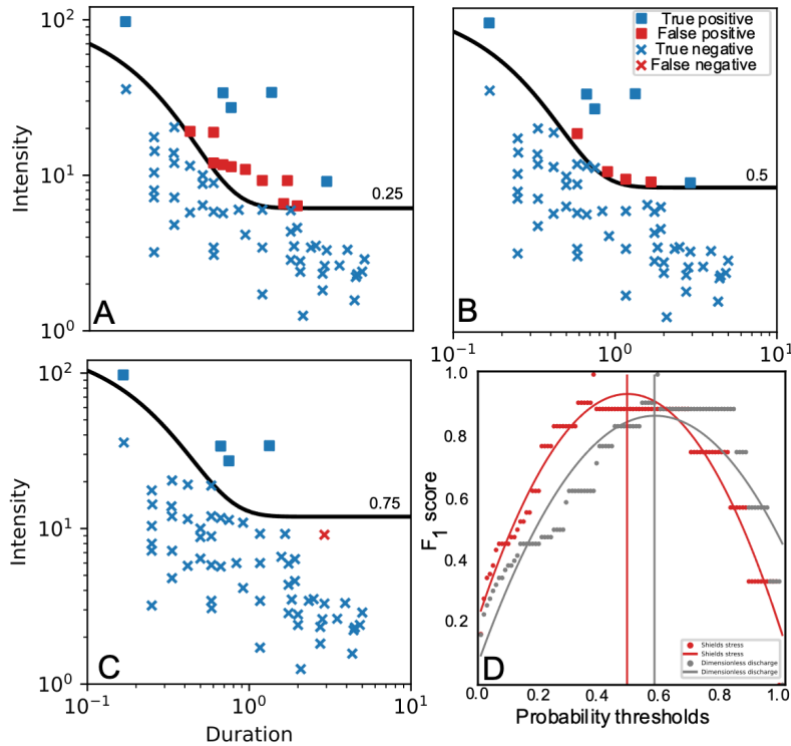


Figure S4. The numbers of false predictions vary depending on the probability threshold chosen as a classifier. In A, a probability threshold of 0.25 is used which results in a high number of false positives. While in C, a probability threshold of 0.75 is used, and a false negative is produced. This results in a parabolic shape of F1 scores as shown in D (Red is Shields stress, and grey is dimensionless discharge).

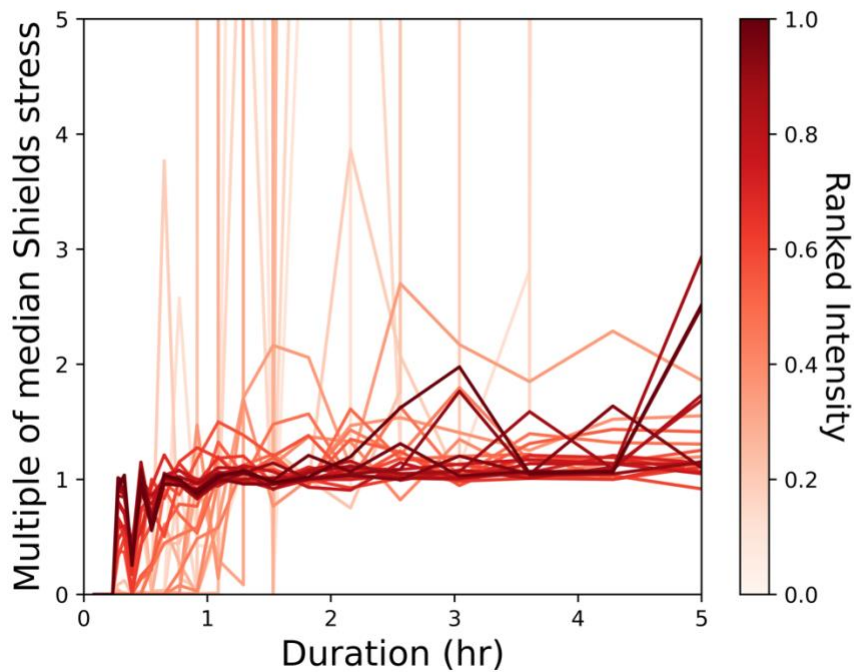


Figure S5. Rainfall events are grouped by their intensity which is then ranked so that the darkest red lines have the highest intensity. The Shields stress generated by each storm is normalized by the median for all storms with that intensity. All intensity groups tend to their median by an hour in duration. Light (low intensity) groups are highly variable.

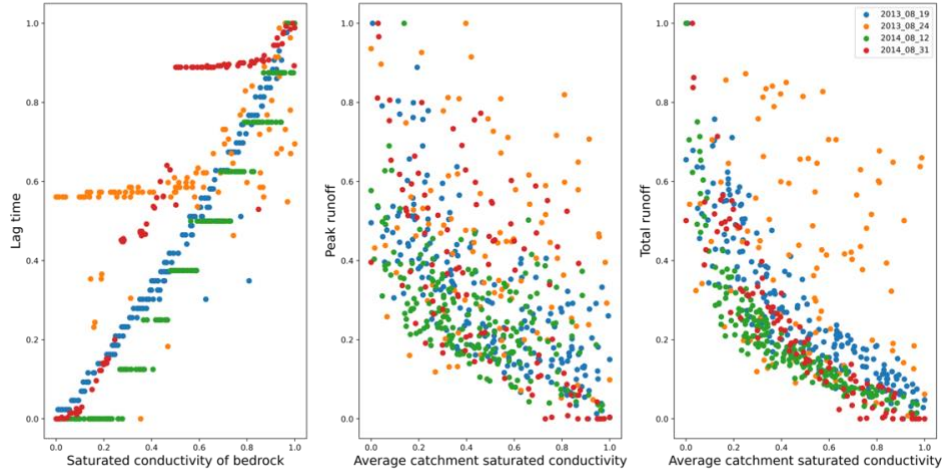


Figure S6. Correlations between the lag time, peak runoff, and total runoff volume and saturated conductivity for the calibration debris flow events. All values are normalized using the min-max method.

Event Date	Saturated conductivity of sediment (K_s) (m/s)	Saturated conductivity of bedrock (K_s) (m/s)	Manning's roughness coefficient (n)
Initial range	$1 \times 10^{-7} - 1 \times 10^{-5}$	$1 \times 10^{-7} - 1 \times 10^{-5}$	0.1 – 0.5
19/08/2013	$1.41 \times 10^{-7} - 9.68 \times 10^{-6}$	$1.19 - 9.33 \times 10^{-7}$	0.15 – 0.19
24/08/2013	$3.02 - 8.22 \times 10^{-7}$	$5.14 - 8.74 \times 10^{-7}$	0.18 – 0.20
12/08/2014	$7.15 \times 10^{-7} - 7.6 \times 10^{-6}$	$1.08 - 9.95 \times 10^{-7}$	0.15 – 0.20
31/08/2014	$3.80 \times 10^{-7} - 8.12 \times 10^{-6}$	$1.07 - 9.96 \times 10^{-7}$	0.15 – 0.20
Combined	$2.38 \times 10^{-7} - 4.13 \times 10^{-6}$	$2.57 - 7.63 \times 10^{-7}$	0.16 – 0.19

Table S1. The Initial and calibrated parameter spaces for each event and the final combined space.