

**Thermo-hydraulic analysis of desiccation cracked soil strata considering
ground temperature and moisture dynamics under the influence of soil-
atmosphere interactions**

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1 **Thermo-hydraulic analysis of desiccation cracked soil strata considering ground**
2 **temperature and moisture dynamics under the influence of soil-atmosphere interactions**

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4 **Abstract**

5 Global warming and climate change significantly affect ground temperature and flow patterns.
6 Moreover, areas prone to cracking experience intensified temperature and moisture variations.
7 Therefore, the aim of this study is to investigate ground temperature and moisture dynamics
8 considering soil-atmosphere interaction through a coupled thermo-hydraulic analysis. Heat
9 transfer, advective, and non-advective fluxes were simulated using CODE_BRIGHT finite
10 element program to study water flow and energy transfer within the soil. Statistical analyses
11 were conducted using an existing dataset to match the crack geometry with previous studies
12 and find the best distribution for the width-to-depth ratio of cracks (C_R) as a dimensionless
13 parameter. The results indicated that C_R variations follow a lognormal distribution. Numerical
14 modeling scenarios were developed using statistical analysis results. The findings indicate that
15 temperature variations decrease exponentially with depth, while surface soil temperature shows
16 higher uncertainty due to atmospheric temperature fluctuations. Collecting various temperature
17 trends in cracked soil at different time intervals, defined a limited region as maximum range of
18 temperature variations (ΔT). Results reveal that ΔT in different crack scenarios can vary up to
19 4 times higher than intact soil. For the prediction of ΔT , considering the impact of climate
20 variations on cracked soil, a 3D boundary surface was developed based on two variables: soil
21 depth (z) and crack depth (C_D). Furthermore, an equation for estimating ΔT for uncracked soils
22 was proposed. Additionally, cracked soil showed approximately 1.4 times higher desiccation

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23 rates than uncracked soil. Deeper cracks exhibited even more severe desiccation rates, being
24 about 1.2 times higher.

25 **Keywords:** Thermo-hydraulic analysis; Soil desiccation cracking; Soil-atmosphere
26 interaction; Ground temperature fluctuations; Soil moisture dynamics.

27 **1. Introduction**

28 Global warming has emerged as a significant concern in recent years, with its far-reaching
29 impacts being felt across various fields, including soil science. One notable consequence of
30 global warming is the rise in ground temperature, impacting geotechnical aspects like
31 geothermal energy extraction.^{1,2} Geothermal systems, like ground source heat pumps (GSHP)
32 utilise the natural heat stored within the ground for heating and cooling purposes, offering an
33 energy-efficient and sustainable alternative to traditional fossil fuel-based systems.³⁻⁷ However,
34 the changing climate and associated phenomena challenge the efficient operation of geothermal
35 systems.⁸ The importance of comprehending ground temperature fluctuations becomes evident
36 when considering specific applications. For instance, in the design and operation of
37 underground infrastructure, such as underground cables or pipelines, temperature variations
38 can impact their structural integrity and performance. In agricultural practices, precise
39 knowledge of ground temperature variations also helps optimise crop growth, irrigation
40 strategies, nutrient availability, and microbial activity.⁹⁻¹¹ Moreover, soil temperature
41 influences the rate of organic matter decomposition and mineralisation of organic materials.¹²

42 Soil desiccation cracking is an indirect result of global warming and the subsequent changes in
43 soil moisture content and temperature. The thermal radiation-induced changes in water-
44 saturated soil increase the kinetic energy of water molecules¹³, leading to evaporation and
45 subsequent soil drying. Soil suction, also known as negative matric potential, represents the
46 energy change per unit volume of water when transferred from the soil to the free water state.¹⁴

47 The drying process induces increased matric suction, which generates tensile stress that can
48 surpass the soil's tensile strength and ultimately lead to the formation of desiccation cracks.^{15,16}
49 Furthermore, the tensile strength is influenced by the degree of saturation.¹⁷ From a fracture
50 energy perspective, cracks tend to form in a manner that minimises fracture energy release by
51 efficiently creating new solid surfaces with the same amount of accumulated strain energy
52 during desiccation.¹⁸ As desiccation progresses, the network of cracks becomes stable, and no
53 new cracks are formed. Instead, the existing cracks widen further.¹⁹ Cracks significantly impact
54 soil thermo-hydraulic (TH) behaviour by facilitating water movement and heat transfer, thereby
55 accelerating the drying process by creating preferential pathways.²⁰ During dry periods,
56 desiccation cracks facilitate evaporation by providing direct pathways for moisture to escape
57 from the soil surface. This increases water loss and reduces soil moisture content, affecting the
58 overall hydrological balance. Additionally, when heavy rainfall occurs following a dry spell,
59 the existing desiccation cracks act as conduits for water infiltration, potentially altering the
60 soil's hydraulic properties and moisture distribution.

61 Based on recent studies, the formation and propagation of cracks have been extensively
62 investigated, including field observations²¹⁻²⁴, laboratory experiments²⁵⁻³⁰, and numerical
63 simulations³¹⁻³⁶. Besides, numerous studies have focused on the soil-atmosphere interaction³⁷⁻
64 ⁴⁰, particularly the influence of cracks on surface evaporation.⁴¹⁻⁴⁶ It was found that turbulent
65 air movement within shrinkage cracks significantly impacts evaporation, even with a constant
66 vapour pressure deficit above the crack aperture.⁴⁷ A significant increase in evaporation from
67 moist cracked soil was observed, with a 60% to 65% higher evaporation rate than intact soil.⁴⁵
68 Similarly, a 12% to 16% increase in evaporation from bare soil due to the presence of cracks
69 was reported.⁴⁸ Various studies have also demonstrated that the evaporation rate from crack
70 opening is a proportion of the evaporation rate from the soil surface. This proportion has been
71 reported at different values, ranging from 30% to 60%.⁴¹⁻⁴³ Therefore, existing studies

72 generally agree on the increased evaporation rate due to soil cracking. However, these studies
73 have limitations in properly acknowledging the variations caused by different crack
74 geometries, spatial arrangements, and resulting water flow regimes. Moreover, these studies do
75 not consider the moisture distribution and rate of soil dryness in both surface and deeper soil
76 depths. Besides, desiccation cracks influence heat transfer within the soil. As the cracks
77 develop, they create channels for enhanced air circulation, affecting thermal conductivity and
78 heat exchange processes. This can result in intensified ground temperature fluctuations and
79 irregular heat distribution within the soil profile, impacting the overall energy balance at the
80 soil surface. The reduced moisture content in cracked soil also leads to decreased thermal
81 conductivity, further contributing to temperature variations.

82 Although the ground temperature and moisture dynamics have various applications in different
83 fields, further investigations are needed to understand their variations in clay layers susceptible
84 to desiccation cracking, considering climate changes and global warming. Cracks induce
85 moisture redistribution with flow regime changes in cracked regions during seasonal climate
86 variations, leading to alterations in the soil drying pattern. These long-term changes in soil
87 moisture distribution can significantly impact ground temperature profiles to considerable
88 depths, making them relevant for planning, designing, and evaluating the performance of
89 underground infrastructures and biogeomechanics purposes. Hence, the objective of this
90 research is to investigate desiccation cracked soil temperature and moisture dynamics,
91 considering soil-atmosphere interaction. This will be accomplished by analysing different
92 modeling scenarios that take into account various predefined crack geometries and spacing.
93 The scenarios are achieved through rigorous statistical analysis of diverse data sets containing
94 crack dimensions on desiccated soils. The study employed a coupled thermo-hydraulic
95 numerical analysis of the soil-atmosphere interaction problem using the CODE_BRIGHT
96 program.

97 2. Material and methods

98 To conduct the simulations, it is essential to calculate the influxes and outfluxes in the soil
99 medium, such as infiltration and evaporation, by analysing the soil-atmosphere interactions.
100 These fluxes can occur in three modes: energy, water, and gas exchange. They result in changes
101 in the mass and energy balance at the soil surface, which need to be continuously accounted
102 for by solving the balance equations. Additionally, determining the total fluxes is crucial for
103 solving these equations, as they consist of both advective and non-advective components.
104 Therefore, defining the governing constitutive equations for the problem is necessary. To
105 indicate this relationship between the balance and the constitutive equations for an unsaturated
106 porous medium, a flowchart, as shown in Fig. 1, is provided. By solving these equations, the
107 unknowns of the balance equations are determined, leading to the solution of the coupled
108 thermo-hydraulic problem. In the following, the governing equations for the problem are
109 presented, including the balance equations, constitutive equations, and equations related to soil-
110 atmosphere interactions.

111 2.1 Balance and constitutive equations

112 Analysing porous media under atmospheric conditions requires considering the mutual
113 interactions between different factors. One important aspect is the thermal expansion of water
114 in the pores, which affects saturation degree and water pressure. Additionally, thermal-induced
115 vapour diffusion has a significant impact on water transfer. These mutual influences between
116 water and heat transfer are key factors that must be considered.⁴⁹⁻⁵¹ The theoretical framework
117 employed in this study utilises a multiphase and multispecies approach. Phases are
118 distinguished using subscripts (*s*: solid, *l*: liquid, *g*: gas), while species are identified using
119 superscripts (*w*: water, *a*: dry air). A compositional approach⁵² was used to formulate mass
120 balance equations, which balance species rather than phases.^{53,54} The formulation's central

121 component comprises two balance conditions corresponding to equations for water mass
122 balance and internal energy balance. These two conditions must be solved simultaneously to
123 account for the interdependent interactions between various phenomena accurately. Then, the
124 formulation is further complemented by a set of constitutive equations and an equilibrium
125 restriction.

126 2.1.1 Mass balance of water

127 Water is considered a species in all three phases: solid, liquid, and gas, within a porous medium.
128 This implies that water can be as adsorbed water on solid surfaces, as liquid water fills the pore
129 spaces and water vapour occupies the void spaces within the medium.⁵⁵ Hence, the total mass
130 balance of water can be expressed as:^{53,54}

$$131 \frac{\partial}{\partial t} \left((\omega_l^w \rho_l S_l + \omega_g^w \rho_g S_g) \phi \right) + \nabla \cdot (\mathbf{j}_l^w + \mathbf{j}_g^w) = f^w \quad (1)$$

132 where ω is mass fraction, ρ is mass content per unit volume of phase, S is degree of saturation,
133 ϕ is porosity, \mathbf{j} is total mass flux and f^w is an external supply of water. The water mass balance
134 equation excludes internal supply, focusing solely on the total mass balance within the medium.
135 By solving this equation, assessing the dynamics and distribution of pore water within the soil
136 voids becomes feasible.

137 2.1.2 Energy balance

138 While in most cases, the energy balance is simplified to an enthalpy balance, it can also be
139 expressed in terms of internal energy. Assuming thermal equilibrium between phases and
140 uniform temperature across all phases, a single equation of total energy is sufficient.⁵⁶
141 Therefore, the internal energy balance for the porous medium by considering the internal
142 energy within each phase can be written as:^{53,54}

$$143 \quad \frac{\partial}{\partial t} (E_l \rho_l S_l \phi + E_g \rho_g S_g \phi) + \nabla \cdot (\mathbf{i}_c + \mathbf{j}_{El} + \mathbf{j}_{Eg}) = f^E \quad (2)$$

144 where E_l and E_g are specific internal energy corresponding to each phase, \mathbf{i}_c is energy flux due
 145 to conduction through the porous medium, \mathbf{j}_{El} , and \mathbf{j}_{Eg} are advective fluxes of energy caused
 146 by mass motions and f^E is an internal or external energy supply.

147 2.1.3 Constitutive equations and equilibrium restriction

148 The constitutive equations establish the relationship between the independent variables
 149 (unknowns) and dependent variables.⁴⁶ When these constitutive equations are substituted into
 150 the balance equations, the governing equations are expressed in terms of the unknowns.
 151 Darcy's law is defined for the advective flow of liquid and gas phases, due to total head
 152 difference, within a porous medium:

$$153 \quad \mathbf{q}_\alpha = -\frac{\mathbf{k}k_{r\alpha}}{\mu_\alpha} (\nabla P_\alpha - \rho_\alpha \mathbf{g}) \quad (3)$$

154 where \mathbf{k} (m^2) is the intrinsic permeability tensor, $k_{r\alpha}$ is the relative permeability, μ_α (Pa.s) is
 155 the dynamic viscosity, P_α (Pa) is the pressure, and \mathbf{g} is the gravity vector. The letter α can
 156 represent either l or g , depending on the phase it is describing. The intrinsic permeability
 157 tensor, as the primary parameter determining the advective flow of liquid and air, is defined by
 158 the Kozeny equation in terms of the porosity for a continuum medium as:

$$159 \quad \mathbf{k} = \mathbf{k}_o \left(\frac{\phi}{\phi_o} \right)^3 \left(\frac{1 - \phi_o}{1 - \phi} \right)^2 \quad (4)$$

160 where ϕ_o is the reference porosity, \mathbf{k}_o (m^2) is the intrinsic permeability which is assumed
 161 isotropic at the reference porosity. Several studies have reported Boom clay intrinsic
 162 permeability between $1 \times 10^{-19} \text{ m}^2$ and $5 \times 10^{-19} \text{ m}^2$.⁵⁷⁻⁶¹ In this study, the isotropic intrinsic
 163 permeability of Boom clay is considered as $2.5 \times 10^{-19} \text{ m}^2$. In addition, to compute the liquid

164 phase relative permeability, the van Genuchten-Mualem model is employed which expresses
 165 the variation of permeability with the degree of saturation:^{62,63}

$$166 \quad k_{rl} = \sqrt{S_e} \left[1 - \left(1 - S_e^{1/\lambda} \right)^\lambda \right]^2 \quad (5)$$

167 where λ is the shape parameter, and S_e is the effective liquid saturation defined in terms of the
 168 residual liquid saturation S_{rl} and the maximum liquid saturation S_{ls} . Notably, as the degree of
 169 saturation decreases, the water meniscus radius decreases, resulting in increased water potential
 170 or soil suction ($P_g - P_l$). It is important to emphasise that this effect becomes particularly
 171 prominent when dealing with suctions in the capillary range. The soil-water retention and
 172 hydraulic conductivity, crucial parameters in unsaturated porous media, can be significantly
 173 influenced by various hydraulic properties, such as pore fluid chemistry and thermal
 174 conditions.⁶⁴⁻⁶⁶ For the establishment of the relationship between suction and saturation degree,
 175 the van Genuchten model⁶² is employed to characterise the soil-water retention curve (SWRC)
 176 which has an important impact on unsaturated flow:

$$177 \quad S_e = \frac{S_l - S_{rl}}{S_{ls} - S_{rl}} = \left[1 + \left(\frac{P_g - P_l}{P_0 (\sigma/\sigma_0)} \right)^{\frac{1}{1-\lambda}} \right]^{-\lambda} \quad (6)$$

178 where P_0 (Pa) is the air entry value, σ (N/m) is the surface tension at a temperature T (°C), and
 179 σ_0 is the surface tension at a temperature at which P_0 is measured. In this study, the surface
 180 tension is assumed to be 0.072 N/m, based on the measured air entry value of 3.44 MPa.
 181 Additionally, the shape parameter is set to 0.3, following the findings reported by Delahaye
 182 and Alonso.⁶⁷ To calculate the relative permeability of the gas phase, the generalised power
 183 method is utilised:

$$184 \quad k_{rg} = AS_{eg}^\lambda \quad (7)$$

$$185 \quad S_{eg} = \frac{S_g - S_{rg}}{S_{gs} - S_{rg}} \quad (8)$$

186 where A is a constant, λ is the model parameter and S_{eg} is the effective gas saturation. Based
 187 on the definition of fluid phase saturation ($S_l + S_g = 1$), S_{rg} and S_{gs} are defined as $1 - S_{rl}$ and
 188 $1 - S_{ls}$, respectively.

189 Fick's law explains the diffusion process in the system, encompassing water vapour diffusion
 190 in the gas phase and air diffusion in water. It allows for the computation of non-advective fluxes
 191 of species within the fluid phases, including molecular diffusion and mechanical dispersion. In
 192 this study, Fick's law is used to express the water vapour diffusion in the gas phase:

$$193 \quad \mathbf{i}_g^w = -(\tau \phi \rho_g S_g D_g^w \mathbf{I}) \nabla \omega_g^w \quad (9)$$

$$194 \quad D_g^w = D \left(\frac{(273.15 + T)^n}{P_g} \right) \quad (10)$$

195 where τ is the tortuosity coefficient which is defined as a constant value ($\tau = \tau_0$), D_g^w (m^2/s) is
 196 the diffusion coefficient of vapour in the air, \mathbf{I} is the identity matrix, D and n are model
 197 parameters. The primary mechanisms of energy transfer in a porous medium involve
 198 conduction, advection resulting from mass flux, and phase change.^{56,68} Therefore, to consider
 199 heat transfer, Fourier's law is employed to calculate the conductive heat flux:

$$200 \quad \mathbf{i}_c = -\kappa \nabla T \quad (11)$$

201 where κ (W/mK) is the thermal conductivity of the porous media. The overall thermal
 202 conductivity decreases with increasing porosity and decreasing degree of saturation. Moreover,
 203 thermal conductivity of a multiphase medium is influenced by the microstructural arrangement
 204 of the material, leading to an average value that combines the conductivities of the individual
 205 phases.³⁸ For intermediate phase arrangements, the weighted geometric mean of thermal
 206 conductivities provides a reliable estimate:

207 $\kappa = \kappa_s^{(1-\phi)} \kappa_l^{\phi S_l} \kappa_g^{\phi(1-S_l)} = \kappa_{sat}^{S_l} \kappa_{dry}^{1-S_l}$ (12)

208 where κ_{sat} and κ_{dry} (W/mK) are the thermal conductivity of the saturated and dry porous
 209 medium, respectively. A linear relationship between thermal conductivity and the degree of
 210 soil saturation is approximated as follows:⁶⁹

211 $\kappa = 0.7936 S_l + 0.646$ (13)

212 From the above equation, the thermal conductivity corresponding to the saturated condition (S_l
 213 = 1) is $\kappa_{sat} = 1.4396$ and dry condition ($S_l = 0$) is $\kappa_{dry} = 0.646$.

214 It is assumed that phase changes, such as water evaporation, occur quickly compared to the
 215 characteristic timescales of the problem. As a result, these phase changes can be considered to
 216 reach local equilibrium rapidly. This assumption leads to an equilibrium restriction that must
 217 always be satisfied.⁵⁶ Hence, the psychrometric law is used to obtain vapour concentration in
 218 the gaseous phase (θ_g^w) as an equilibrium restriction:

219 $\theta_g^w = (\theta_g^w)^0 \exp \left[\frac{-(P_g - P_l) M_w}{R(273.15 + T) \rho_l} \right]$ (14)

220 where M_w is the water molecular weight (0.018 kg/mol), R is the universal gas constant (8.314
 221 J/(mol.K)), and $(\theta_g^w)^0$ is the saturated vapour concentration in the gas phase in equilibrium
 222 with a liquid with a flat surface at the same temperature, defined as:

223 $(\theta_g^w)^0 = \frac{M_w P_{v(T)}}{R(273.15 + T)}$ (15)

224 $P_{v(T)} = 136075 \exp \left[\frac{-5239.7}{273.15 + T} \right]$ (16)

225 where $P_{v(T)}$ (Pa) is the saturated vapour pressure. In [Table 1](#), defined constitutive equations and
 226 model input values are summarised.

227 2.2 Soil-atmosphere interaction

228 Soil-atmosphere interaction refers to the dynamic exchange of energy, water, and gases
229 between the soil surface and the surrounding atmosphere.³⁷ The soil-atmosphere interface is a
230 critical boundary where heat, moisture, and gases are transferred through complex mechanisms
231 such as evaporation, transpiration, and gas diffusion.^{39,40}

232 The adopted model incorporates atmospheric variables, including temperature, precipitation,
233 wind speed, relative humidity, and radiation. Subsequently, it calculates the water fluxes (via
234 gas and liquid phases) driven by evaporation and rainfall, the air and energy fluxes
235 encompassing radiation, advective effects, and convective energy fluxes. The following section
236 explains the governing equations of soil-atmosphere interaction.

237 2.2.1 Mass fluxes of gas and water

238 The advective air flux (j_a) is utilised in the gas phase and defined as:

$$239 \quad j_a = \omega_g^a q_g = (1 - \omega_g^w) \gamma_g (P_g - P_{ga}) \quad (17)$$

240 where q_g is the flux of the gas phase, P_{ga} (Pa) is the atmospheric pressure and γ_g is the leakage
241 coefficient. The calculation of evaporation (E) involves an aerodynamic diffusion relation, as
242 follows:⁷⁰⁻⁷²

$$243 \quad E = \frac{1}{r_a} (\rho_{va} - \rho_v) = \frac{k^2 v_a \varphi}{\left(\ln \frac{z_a}{z_0}\right)^2} (\rho_{va} - \rho_v) \quad (18)$$

244 where r_a (s/m) is the aerodynamic resistance of air, k is the von Karman's constant, φ is the
245 stability factor, v_a (m/s) is the wind speed, z_0 (m) is the ground surface roughness length, z_a
246 (m) is the screen height at which v_a and ρ_{va} are measured, ρ_{va} and ρ_v (kg/m³) are the vapour
247 mass per volume of the gas of the atmosphere and ground, respectively. Therefore, to define
248 the movement of vapour induced by water evaporation through the soil pores or air-filled

249 spaces driven by pressure difference, the advective flux of vapour by the gas phase is defined
 250 as:⁷²

$$251 \quad \begin{cases} j_g^w = \omega_g^w q_g & \text{if } P_g > P_{ga} \\ j_g^w = \frac{\rho_{va}}{\rho_{ga}} q_g & \text{if } P_g \leq P_{ga} \end{cases} \quad (19)$$

252 where ρ_{ga} is the atmospheric air density. When considering rainfall effects, it is important to
 253 mention that water flows over the land surface due to precipitation may exceed the soil's
 254 infiltration capacity. In this case, the shallow soil becomes saturated ($P_l > P_{ga}$), and any rainfall
 255 that cannot infiltrate will result in runoff. The surface runoff (j_{sr}) represents the rate at which
 256 water flows through the liquid phase is written as:⁷²

$$257 \quad \begin{cases} j_{sr} = \gamma_w (P_l - P_{ga}) & \text{if } P_l > P_{ga} \\ j_{sr} = 0 & \text{if } P_l \leq P_{ga} \end{cases} \quad (20)$$

258 where γ_w is the water leakage coefficient. It is important to note that the simulation does not
 259 explicitly account for ponding. When assuming no ponding, a sufficiently high value for γ_w
 260 can be utilised (while avoiding numerical instabilities). Finally, the total water flux (j_w) is
 261 expressed as the sum of previously defined parameters, as follows:

$$262 \quad j_w = k_{rain}P + k_{evap}E + j_g^w + j_{sr} \quad (21)$$

263 where P is the rainfall, k_{rain} and k_{evap} are the coefficients that can change the effect of related
 264 parameters. In this study, the values of these coefficients were set to one to account for the
 265 complete impact of evaporation and precipitation.

266 2.2.2 Flux of energy

267 According to the aerodynamic diffusion equation, the sensible heat flux (H_s), involves the
 268 transfer of thermal energy due to the temperature difference between the particles, is calculated
 269 as:^{39,71,72}

$$270 \quad H_s = \frac{\rho_{ga} C_a}{r_a} (T_a - T) = \frac{k^2 v_a \varphi}{\left(\ln \frac{z_0}{z_a}\right)^2} \rho_{ga} C_a (T_a - T) \quad (22)$$

271 where C_a (J/(kg.K)) is the specific heat of the air, T_a and T are the atmospheric and ground
 272 temperature, respectively. The convective heat flux (H_c) is computed by considering the
 273 internal energy of liquid water, vapour, and air, as:⁷²

$$274 \quad H_c = h_v (E + j_w^g) + h_{la} (P + j_w^l) + h_{a0} j_a \quad (23)$$

275 where h_v , h_{la} , and h_{a0} (J/kg) are the free energy of vapour, liquid water and air, respectively.
 276 Therefore, the total energy flux (j_e) can be written as:

$$277 \quad j_e = k_{rad} R_n + H_s + H_c \quad (24)$$

278 where k_{rad} (assumed equal to one) is the coefficient that can change the effect of net radiation
 279 (R_n). Through the solution of the governing equations for gas flux (j_a) (Eq. (17)), water flux
 280 (j_w) (Eq. (21)), and energy flux (j_e) (Eq. (24)), which form the fundamental theoretical
 281 framework of soil-atmosphere interaction analysis, valuable insights can be obtained regarding
 282 the heterogeneous distribution of heat, water, and gases in the desiccation cracked soils due to
 283 permanent climate change.

284 2.3 Thermo-hydraulic model

285 The thermo-hydraulic interaction between desiccation cracked soil and the surrounding
 286 environment is a complex process involving simultaneous heat and moisture transfer. Heat
 287 transfer depends on factors like temperature gradients, thermal conductivity, and radiative heat
 288 flux, while moisture transfer is influenced by hydraulic properties like soil-water retention and

289 hydraulic conductivity, along with the presence of cracks as preferential flow pathways. For
290 considering these requirements of a coupled thermo-hydraulic model, the simulations were
291 performed using the CODE_BRIGHT program⁵³, employing the finite element method. Based
292 on this framework, triangular elements were utilised for spatial discretisation, while the finite
293 difference method was adopted for temporal discretisation. The final geometry of the model
294 and the procedure for determining the model's geometry is explained in Section 3.1.

295 The material of interest chosen for this investigation is Boom clay, a type of clayey soil that is
296 particularly susceptible to soil desiccation cracking. Boom clay has been extensively the
297 subject of previous studies; hence its thermo-hydraulic parameters have been widely
298 reported.⁵⁷⁻⁶¹ As a first initial condition of porous medium, the initial soil temperature is set at
299 15 °C, based on the research by De Bruyn and Labat⁷³, where the Boom clay temperature at a
300 depth of 223 m was reported as 16.6 °C. Therefore, a constant soil temperature of 15 °C is
301 assumed as the equilibrium temperature (T_{eq}) of the soil, as the near-surface depths are
302 influenced by temperature fluctuations due to climatic variations. The second initial condition
303 considers an assumed soil porosity of 0.487, falling within the reported range for Boom clay in
304 literature.^{57,59,61} Thermo-hydraulic characteristics of Boom clay used in this study are given in
305 [Table 1](#).

306 One of the critical input parameters for this TH model is climatic data, including temperature,
307 precipitation, relative humidity, wind speed, and radiation. Due to higher rates of drying and
308 susceptibility to cracking in tropical regions, the study area selected for this research is the
309 Qom city. This region is well-known for its high temperatures, prevailing winds, and its
310 vulnerability to subsidence hazards and crack propagation networks.⁷⁴ Climatic data
311 corresponding to the study area were collected for a three-year period from 2015 to 2017 as
312 depicted in [Fig. 2](#).

313 3. Results and discussion

314 3.1 Statistical analysis of crack geometry and spacing

315 Understanding the behaviour of cracked soils requires reliable determination of the geometry
316 and spacing of cracks. However, measuring these characteristics in large-scale field studies can
317 be difficult. To overcome this challenge, a statistical approach is employed to make the best
318 use of available dataset in the literature for crack geometries in desiccated soils. This approach
319 enables us to draw valuable insights from limited data and make dependable predictions about
320 crack behaviour under varying conditions. Furthermore, in order to utilise data from small-
321 scale laboratory and numerical studies, a dimensionless parameter called the crack ratio (C_R)
322 has been introduced. This parameter is defined as the ratio of crack width (C_W) to depth (C_D)
323 and is summarised in [Table 2](#) for various studies.

324 In field-scale studies, crack depth of up to 6 m have been reported²¹, and in the study area of
325 this research, deep cracks with considerable lengths have been observed. To cover the full range
326 of crack depths, three values of 1 m, 2.5 m, and 5 m were considered, based on previous
327 researches^{21,42} and field observations. According to [Table 2](#), the C_R can vary between 1% and
328 42%, encompassing a relatively large range. However, selecting only the average value of this
329 range may not be representative of the entire range of C_R . In order to accurately estimate the
330 representative value of the C_R , the variable inference was conducted using RTx software.⁷⁵ In
331 [Fig. 3\(a\)](#) the Gaussian and lognormal distributions are plotted alongside the C_R data from [Table](#)
332 [2](#). As observed, the crack width-to-depth ratio follows a lognormal distribution. By utilising
333 the mean and coefficient of variation of the C_R as 13.2 and 85.7%, the probability density
334 function (PDF) for the C_R data was depicted, as shown in [Fig. 3\(b\)](#). The peak of this curve
335 indicates the mode of the data, representing the C_R with the highest density among the available
336 data. Therefore, the first representative value for the C_R was estimated as the mode of the data,

337 which is 5%. The second representative value for the C_R was chosen to be the mean point, being
338 approximately 13% for the data reported in Table 2. To observe the larger C_R effect in this
339 study, the third representative value was estimated using the mean plus twice the standard
340 deviation (i.e., $\text{mean} + 2\sigma = 35\%$). As a result, the three representative points for the C_R have
341 been shown in Fig. 3(b). The modeling scenarios for crack geometry are in accordance with
342 Table 3, where each scenario is named using the format “DiRj”. For example, the model
343 D2.5R13 refers to the crack depth of 2.5 m, crack ratio of 13%, and consequently, crack width
344 of 0.325 m.

345 The spacing between cracks is also of great importance when studying the thermo-hydraulic
346 response of soil to climate change. This is because closely spaced cracks significantly impact
347 the thermo-hydraulic behaviour of soil due to their strong interaction, while widely spaced
348 cracks have a less pronounced effect. In this study, random values for crack spacing were used
349 to cover various crack spacings, assess their effects on soil behaviour, and compare them with
350 the case of intact soil. The crack spacing was defined by two parameters, x and r_i . Parameter
351 x was defined to establish the upper limit of the crack spacing. Based on field observations and
352 the limited geometry of the simulated model, a maximum crack spacing of 10 m was assumed
353 for parameter x . Parameter r_i was introduced to allow for random variation in the crack spacing.
354 This parameter encompasses random values ranging from 0.1 to 1. The product of these two
355 parameters determines the crack spacing ($C_S = x \cdot r_i$). For example, if r_i is set to 0.5, the crack
356 spacing would be 5 m in this case. Based on this definition, the determined range for crack
357 spacing is from 1 m to 10 m. The randomly generated crack spacings are presented in Fig. 4.

358 The crack geometry and spacing parameters are illustrated schematically in Fig. 5. The final
359 geometry of the model consists of a length of 100 m, representing 14 cracks. According to Fig.
360 4, the minimum and maximum distances between the cracks are 1.3 m (between 3rd and 4th
361 cracks) and 9.1 m (between 12th and 13th cracks), respectively. The first and last cracks are also

362 positioned approximately 10 m away from the lateral boundaries of the model, ensuring that
363 the lateral boundaries have minimal influence on the behaviour of the cracks. The depth of the
364 soil is considered to be 50 m, covering the range of thermal variations in the soil depth. The
365 geometry of the model and the finite element mesh for model D2.5R13 are illustrated in Fig.
366 6.

367 3.2 Soil temperature distribution

368 The ground temperature determines the availability and accessibility of heat energy. Moreover,
369 cracks, fractures, or fissures in geological formations can act as pathways for heat transfer,
370 altering heat flow patterns. Therefore, in this section, the impact of climatic conditions on the
371 thermal variations of the ground is examined, and results for the cracked and uncracked soil is
372 compared.

373 3.2.1 Effect of climate change on intact soil

374 Specific time points were selected based on their distinct temperature characteristics to
375 investigate the impact of climatic conditions on temperature distribution in intact soil. These
376 time points correspond to specific days during the three years of study from 2015 to 2017,
377 including the last day of 2017 (experienced a temperature of 8.75°C), the coldest day in 2016
378 (-2.8°C), and the warmest day in 2015 (39.1°C). Additionally, three days with an average
379 temperature (20.6 °C) over the three years were chosen.

380 The temperature distributions for each selected day are presented in Fig. 7. The results indicate
381 that the soil surface temperature is almost equal to the air temperature (T_a) and each soil depth
382 has a different value. The temperature distribution does not show a consistent trend until
383 reaching the equilibrium value of 15 °C (T_{eq}). Fig. 7(a) shows that at the end of 2017, soil
384 temperature increased from 8.6 °C to a maximum of 19 °C at a depth of 4.2 m. Then, the
385 temperature distribution changed until it reached T_{eq} at a depth of 20 m. The distribution of

386 temperature on the coldest day followed the same trend. When the T_a was $-2.8\text{ }^\circ\text{C}$, the
387 maximum temperature occurred at a depth of 2.7 m and was equal to $21.2\text{ }^\circ\text{C}$. The opposite
388 trend was observed on the warmest day, where the soil temperature decreased from $38.5\text{ }^\circ\text{C}$ to
389 $14.8\text{ }^\circ\text{C}$ at a depth of 6.1 m . In other words, when $T_a < T_{eq}$, a soil depth should be expected to
390 have a higher temperature than T_{eq} . On the other hand, when $T_a > T_{eq}$, a soil depth will have
391 a lower temperature than T_{eq} .

392 Ground temperature is affected by various factors, including soil properties, moisture content,
393 and thermal conductivity. Soil with lower moisture content is more sensitive to temperature
394 changes than moist soil. In general, deeper soil experiences less extreme temperature variations
395 than shallow soil. By delving deeper into the soil, the amplitude of temperature variation
396 diminishes exponentially, primarily influenced by the soil's thermal inertia. Thermal inertia
397 refers to the soil's ability to resist rapid temperature changes. The soil acts as a thermal buffer,
398 absorbing and storing heat during warmer periods and releasing it during cooler periods. This
399 property causes temperature variations to smooth out with increasing depth, as the soil's
400 thermal inertia dampens the impact of external temperature fluctuations. Consequently, the
401 deeper layers of the soil exhibit a more stable temperature profile with reduced thermal
402 fluctuations amplitude. Finally, it should be noted that heat changes in soil can be diverse and
403 non-uniform, particularly when considering average air temperatures.

404 According to [Fig. 7\(b\)](#), the soil temperature at a depth ranging from 0.9 m to 1.9 m falls between
405 $13.1\text{ }^\circ\text{C}$ to $14.2\text{ }^\circ\text{C}$, which is notably lower than the T_{eq} . Of particular interest is the observation
406 of another peak in the thermal distribution during both 2016 and 2017, found at a depth of
407 approximately 7.5 m . Analysing the thermal distribution of 2017, distinct temperature readings
408 were recorded at various depths: $20.6\text{ }^\circ\text{C}$ at the soil surface, $14.2\text{ }^\circ\text{C}$ at 1.9 m depth, $17\text{ }^\circ\text{C}$ at
409 7.5 m depth, and $15\text{ }^\circ\text{C}$ at 25 m depth. These observations indicate a considerable temperature

410 variation within different soil depths during the same time. Furthermore, Fig. 7(b) provides a
411 comprehensive illustration of the relationship between climatic conditions and the equilibrium
412 depth (z_{eq}). Notably, the graph clearly depicts the z_{eq} values for the years 2015, 2016, and
413 2017, which stand at 7 m, 14 m, and 20 m, respectively. This graphical representation
414 effectively underscores the influence of varying climatic factors on the depth at which thermal
415 equilibrium is attained within the soil profile. These findings are consistent with the research
416 outcomes reported by An et al.⁴⁰, Nwankwo and Ogagarue⁷⁶, Wang et al.⁷⁷, Badache et al.⁷⁸.

417 3.2.2 Effect of cracking and spacing

418 To investigate the impact of cracks on the soil temperature distribution and make a comparison
419 with intact soil, the D1R5 model was utilised for selected days across three successive years,
420 as depicted in Fig. 8(a). This figure provides a representation of the thermal distribution
421 between the 3rd and 4th cracks, which are spaced 1.3 m apart. The findings indicate that the
422 presence of 1m-depth cracks led to an increase in ground temperature, particularly up to a depth
423 of approximately 2.5 m. Notably, this change resulted in a downward and rightward shift of the
424 curve, ultimately increasing in the minimum temperatures formed in the soil deeper layers. At
425 a depth of 2.5 m, the temperature distribution exhibited an increase of 0.5°C to 1°C due to the
426 presence of cracks. More precisely, the minimum temperature recorded at this depth was 71
427 cm, 40 cm, and 68 cm deeper than the corresponding point in intact soil for the years 2015,
428 2016, and 2017, respectively. Furthermore, the minimum temperature at this depth experienced
429 an increase of 0.23°C, 0.36°C, and 0.34°C in those respective years, due to soil cracking. It is
430 important to emphasise that this distribution may vary depending on the depth and spacing of
431 the cracks, as explained in Section 3.2.3.

432 As the distance between cracks increases, the interaction between them diminishes, and the
433 temperature distribution between the cracks becomes similar to that of the intact soil, as shown

434 in Fig. 8(b). Specifically, the temperature distribution between the 12th and 13th cracks closely
435 matches the thermal distribution observed in the intact soil. However, the behaviour observed
436 near the cracks deviates from the overall trend. When the distance between the cracks
437 decreases, and the air temperature surpasses the soil's equilibrium temperature (e.g., on the
438 warmest day, see Fig. 8(b)), the soil up to a depth of approximately 3.5 m exhibits higher
439 temperatures than the distant cracks. Conversely, when T_a is lower than T_{eq} (e.g., on the coldest
440 day, see Fig. 8(b)), the soil up to a depth of about 3 m demonstrates lower temperatures than
441 the distant cracks. This finding implies that the thermal response of closely-spaced cracked soil
442 is contingent upon the relative difference between the air and the soil equilibrium temperature.
443 For instance, on the warmest day, at a depth of 3 m, the temperature distribution between the
444 12th and 13th cracks yielded a temperature of 17.5 °C, while between the 3rd and 4th cracks, it
445 reached 19.5 °C, resulting in a temperature difference of 2 °C due to the variation in crack
446 distance. The observations above highlight the complex interplay between crack proximity, air
447 temperature, and thermal behaviour of the soil.

448 3.2.3 Effect of crack width and depth

449 The influence of crack width was specifically investigated in models with C_D of 1 m and C_R of
450 5%, 13%, and 35%, comparing the coldest and warmest days as shown in Fig. 9(a). Since
451 cracks provide preferential pathways for heat and moisture transfer, the crack depth allows for
452 a deeper connection between soil layers and the enclosed crack atmosphere. However, the crack
453 width only has a marginal effect and does not significantly impact the temperature distribution
454 inside the soil, as depicted in Fig. 9(a).

455 Furthermore, the impact of crack depth was examined by comparing models with C_D of 1 m,
456 2.5 m, and 5 m, all with the same C_R of 13%, as illustrated in Fig. 9(b). This figure clearly
457 demonstrates that with increasing crack depth under various climatic conditions, z_{eq} also

458 increases. Furthermore, the temperature pattern on the coldest day highlights that maximum
459 temperature are observed at depths of 3 m, 4.4 m, and 7.2 m when the crack depths are 1 m,
460 2.5 m, and 5 m, respectively. In other words, increasing the crack depth from 1 m to 5 m, results
461 in the maximum temperature occurring 4.2 m deeper. Similarly, on the warmest day,
462 z_{eq} corresponds to depths of 6.4 m, 7.5 m, and 10 m for the crack depths mentioned above. As
463 depicted in the figure, increasing the crack depth reveals greater thermal fluctuations at
464 different depths. For the warmest day, in D1R13, the temperature decreases with a specific
465 gradient until reaching the equilibrium depth. However, in D2.5R13, the temperature decreases
466 from the ground surface to a depth of 0.6 m, followed by a steeper gradient from 0.6 m to 2.5
467 m, and then continues to decrease with the same gradient as D1R13 from 2.5 m to the
468 equilibrium depth. A similar trend is observed in D5R13, but with the difference that
469 temperature variations persist with a steeper gradient beyond a depth of approximately 0.8 m
470 to 5 m. Thus, in closely spaced cracks, temperature fluctuations intensify with increasing crack
471 depth. These fluctuations can be divided into two parts: the first part corresponds to temperature
472 fluctuations up to the soil depth equal to the crack depth with two different gradients, and the
473 second part extends from the crack depth to the equilibrium depth. These findings also hold
474 true for the coldest days trends.

475 As depicted in Fig. 8, the temperature distribution between the 12th and 13th cracks for the
476 D1R5 model closely resembles the temperature distribution of intact soil due to reduced
477 interaction. However, as depicted in Fig. 9(c), when the crack depth increases from 1 m to 5
478 m, it leads to a stronger interaction between the 12th and 13th cracks. Consequently, the
479 temperature distribution between these cracks deviates from the pattern observed in intact soil.
480 Therefore, it can be concluded that as the distance between cracks decreases or the crack depth
481 increases, there is an amplified interaction among the cracks. This results in a distinct
482 temperature pattern within the soil depth, where temperatures lower than T_{eq} experience a

483 greater decrease, while temperatures higher than T_{eq} undergo a greater increase. Indeed, as
484 shown in Fig. 9(c), the temperature trend corresponding to a crack depth of 5 m displays
485 noticeable peaks at depths around 2.5 m and 8 m. These peaks lead to lower temperatures (about
486 0.7 °C compared to intact soil) at a depth of 2.5 m and higher temperatures (about 0.4 °C
487 compared to intact soil) at a depth of 8 m compared to the trends observed at other crack depths.

488 3.2.4 Maximum range of temperature variation

489 The thermal distribution in a specific soil depends on various factors such as climatic
490 conditions, surface cracks geometry, and distance between them, as discussed in Sections 3.2.1,
491 3.2.2, and 3.2.3. This means that different thermal patterns can be observed for different crack
492 scenarios over the three-year period. It is important to note that every soil depth should
493 experience a limited range of temperature variations under different climate and crack
494 conditions. Therefore, 30 scenarios of the ground thermal distributions are considered, as
495 shown in Fig. 10(a). The results indicate that the temperature distributions among different
496 C_D of 1 m, 2.5 m, and 5 m. It provides a representation of the temperature patterns between the
497 closest cracks and the furthest cracks under varying atmospheric temperatures. The results
498 reveal that all the temperature distributions fall within a certain range, regardless of the crack
499 geometry or climate conditions. This range represents the maximum temperature variations
500 (ΔT) at a specific soil depth.

501 Considering the Gaussian distribution for the temperature variation in the different soil depths,
502 one can achieve the probability density functions (PDF) as shown in Fig. 10(b) and (c). The
503 results show that, by increasing the soil depth, the average soil temperature decreased from
504 20°C, at the soil surface to 15°C at a depth of 15 m. Also, the coefficient of variation in the soil
505 temperature decreases from 65% to 1.85% by reaching a depth of 15 m from the ground surface.
506 This leads to a narrower PDF of the soil temperature that shifts towards a lower temperature

507 regime as the soil depth increases. This indicates that the peak of PDF progressively moves
508 from the average air temperature (20.6 °C) towards the equilibrium temperature of the soil (15
509 °C). Accordingly, it is evident that the uncertainty in the variation of soil temperature decreases
510 by increasing the soil depth, and the average soil temperature tends to a lower temperature.
511 This finding suggests a more stabilised thermal environment with lower temperature
512 fluctuations at greater depths.

513 Fig. 11 illustrates the lower and upper bounds shown in Fig. 10(a) based on crack depth
514 separately. As the crack depth decreases, the range of maximum temperature fluctuations
515 becomes narrower. Eventually, the minimum range of temperature fluctuations is noticed in the
516 intact soil trends. For example, at a soil depth of 5 m, the maximum temperature variations
517 (ΔT) can extend up to 5 °C, 5.5 °C, 7.6 °C, and 21.9 °C for the intact soil, cracked soil with C_D
518 of 1 m, 2.5 m, and 5 m, respectively. In other words, at a soil depth of 5 m, the temperature
519 fluctuations in the presence of a 5-meter-depth crack are considerably higher, exhibiting an
520 enhanced temperature of 16.9 °C (338%), 16.4 °C (298%), and 14.3 °C (188%) compared to
521 the intact soil, cracked soil with C_D of 1 m, and 2.5 m, respectively. As the equilibrium depth
522 is reached, the range of temperature variations becomes narrower in all cases, eventually
523 converging at a depth close to 20 m.

524 Fig. 12 provides an estimation of the maximum temperature variation range (ΔT) based on the
525 findings from Fig. 11. It is noted that the influence of crack spacing was taken into account, as
526 the most critical spacing scenarios were considered in Fig. 11. Additionally, the influence of
527 climate change by including different climatic scenarios that align with those depicted in Fig.
528 11, was incorporated into the results of Fig. 12. Therefore, a comprehensive visualisation of
529 the maximum range of temperature variations (ΔT) at various cracked soil depths is presented
530 in Fig. 12. As depicted in Fig. 12, at the soil surface (Boundary 1, see Fig. 12(b)), ΔT is
531 approximately 42 °C, which indicates the difference between the maximum and minimum air

532 temperatures ($\Delta T_{z=0} = T_{a,max} - T_{a,min}$) experienced by the ground over time. Notably, this
533 value is independent of the crack depth. Moving towards deeper soil layers, variations in ΔT
534 are strongly influenced by crack depth. According to Fig. 12(b), boundary 3, indicates ΔT
535 changes for a crack depth of 1 m, demonstrates a uniformly decreasing pattern. However,
536 boundary 2, representing ΔT variations for a crack depth of 5 m, exhibits non-uniform changes
537 influenced by different decreasing slopes. Moreover, ΔT variations at a soil depth of 10 m
538 (Boundary 4) exhibit an ascending slope from a crack depth of 1 m to 5 m. This outcome
539 suggests that, a wider temperature fluctuation range is encompassed with an increasing crack
540 depth at a soil depth of 10 m.

541 Fig. 13(a) clearly shows the decreasing rate of ΔT in deeper cracked soil layers. According to
542 the results, the soil with a 1-meter crack depth exhibits the highest ΔT decreasing rate with
543 increasing soil depth, while the soil with a 5-meter crack depth shows the lowest decreasing
544 rate. For example, at a depth of 1 m below the ground surface, the corresponding ΔT for crack
545 depths of 1 m, 2.5 m, and 5 m are 25 °C, 27.2 °C, and 33 °C, respectively. In other words, ΔT
546 for a soil with 5 m crack depth is 33% and 21.3% higher than that of 1 and 2.5 m crack depths,
547 respectively (as Fig. 13(b)). However, at 4 m soil depth, ΔT for a soil with 5 m crack depth is
548 264% and 150% higher than for 1 m and 2.5 m crack depths, respectively. As shown in Fig.
549 13(b), this difference reaches its maximum value at 5 m below the ground surface (298.2% and
550 187.5%). Beyond this depth, the difference gradually decreases, suggesting that it tends to
551 approach zero at the equilibrium depth. In other terms, it is demonstrated in Fig. 13(b) that for
552 each soil depth, ΔT corresponds to D5R13, how much percentage is higher than ΔT corresponds
553 to D1R13 and D2.5R13.

554 Predicting the maximum temperature variations (ΔT) at a specific intact soil depth is relatively
555 simpler. As shown in Fig. 14, the trend of ΔT for intact soil depends solely on the soil depth

556 and follows an exponential trend. By fitting the available data, the relationship can be expressed
557 as:

$$558 \quad \Delta T = 23.2708 \exp[-0.30381z] \quad (25)$$

559 where z is the soil depth. Additionally, this equation can approximate the equilibrium depth at
560 which $\Delta T = 0$. This equilibrium depth in recent studies has been reported to be approximately
561 15 m.^{76,78} The proposed equation for temperature variations at a soil depth of 15 m yielded a
562 negligible result of 0.24 °C, which is consistent with recent studies.

563 3.3 Soil saturation distribution

564 The ground surface experiences substantial hydraulic fluctuations in response to seasonal
565 climatic changes, leading to variations in soil moisture content, infiltration rates, and so on. On
566 rainy days, infiltration occurs, leading to saturation of the shallow soil layer and runoff
567 formation in heavy rainfall conditions. On the other hand, cracks facilitate water penetration to
568 deeper soil layers. However, during those days with intense radiation and high temperatures,
569 the ground surface suffers evaporation much more severe. Evaporation occurs at different rates
570 from the soil surface and crack walls. Therefore, in the vicinity of cracks, there are more
571 pronounced changes in the flow regime. In this section, the variation of soil saturation degree
572 (S_r) and the influence of different crack geometries and spacing will be examined.

573 3.3.1 Effect of crack width

574 The two-dimensional distribution of soil saturation degree (S_r) in cracked soil at the end of
575 2017 is compared based on the crack width, as shown in Fig. 15. This distribution is influenced
576 by various factors such as the history of hydraulic loading (seasonal changes in climatic
577 conditions) and soil hydraulic parameters. Therefore, the chosen timeframe specifically
578 addresses the impact of crack width on the extent of soil desiccation. According to this figure,

579 desiccation is more severe in the cracked regions. Moreover, this intensity becomes more
580 pronounced with decreasing distance between cracks. The distribution of saturation degree in
581 all three cases indicates that cracks intensify the soil drying along the vertical direction. This is
582 evident from the downward deflection of contour lines towards the cracks and their upward
583 inclination as they move away from the cracks. As a result, this leads to heterogeneous moisture
584 variations at different soil depths, unlike the case of intact soil with a homogeneous moisture
585 distribution along the horizon. On the other hand, in closely spaced cracks, narrower cracks
586 demonstrate higher surface desiccation. As observed, the contour line between two cracks in
587 Fig. 15(a) represents a value of 23.3%, while the same point in Fig. 15(c) represents a value of
588 30%. Furthermore, comparing the contour lines between the two cracks in this figure reveals
589 that the contour lines tilt upwards with an increase in crack width, indicating less drying in
590 wider cracks. The observed results are consistent with field observations and laboratory
591 experiments findings reported by Adam and Hanks⁴¹, Ritchie and Adam⁴², Poulsen⁴⁵.

592 3.3.2 Effect of crack depth and spacing

593 For a more detailed examination of the impact of crack parameters, with a focus on crack depth
594 and distance, the degree of soil saturation was considered as a measure of soil dryness for
595 different distances from the 3rd crack, as depicted in the schematic diagram of Fig. 16(d). The
596 degree of saturation was determined up to a distance of 60 cm from the 3rd crack aperture, with
597 intervals of 10 cm. The comparison between Fig. 16(a) and (b) reveals the influence of different
598 periods on the degree of soil dryness due to seasonal climate changes. Since homogeneous and
599 isotropic soil has a uniform moisture distribution, the degree of saturation remains constant at
600 a specific depth within the soil. However, in cracked soil, the degree of saturation at a constant
601 depth depends on the distance from the crack.

602 Fig. 16(a) demonstrates that at the end of 2015 (after one year), the overall intact soil surface
603 dryness is higher compared to the cracked soil (less S_r). The cracked soil within a distance of
604 60 cm from the crack, reaches its minimum level of dryness (maximum saturation degree),
605 indicating approximately 4.5% higher S_r compared to the intact soil. Additionally, in the
606 regions closer to the crack especially at a distance of 20 cm, the soil dryness is higher in crack
607 ratios of 5% and 13% (D1R5 and D1R13) compared to other cases. Among all the cases of
608 cracked soil, the surface soil dryness was highest in D1R5, and with increasing crack depth and
609 width, less surface dryness was observed.

610 After three years, at the end of 2017, the cracked soil generally exhibited higher ground surface
611 dryness compared to the intact soil, as shown in Fig. 16(b). By comparing the two different
612 periods, it is evident that during this time, S_r in the intact soil decreased from 31% to 22.5%,
613 indicating an 8.5% reduction. On the other hand, S_r in the cracked soil at a distance of 60 cm
614 from the crack decreased from approximately 35.5% to 23.3%, indicating a 12.2% reduction.
615 Therefore, it can be concluded that over time, despite climate fluctuations involving rainfalls,
616 various wind velocity, relative humidity, radiation, and temperatures, the cracked soil exhibits
617 a higher rate of ground surface drying compared to intact soil.

618 Among the cases of cracked soil, at a distance of 30 cm, S_r in D1R5 decreased from 31.8% to
619 21.9% after two years, indicating a 9.9% reduction. Similarly, S_r in D2.5R13 decreased from
620 34% to 22%, indicating a 12% reduction. Hence, it can be inferred from the results that among
621 the different crack scenarios, deeper cracks experience a higher rate of drying. Furthermore,
622 the results demonstrate that the maximum dryness at the ground surface occurs at a specific
623 distance from the crack aperture, which was obtained to be 20 cm in this study.

624 It is important to mention that the moisture distribution difference between intact and cracked
625 soil is not limited to surface dryness. Cracks also lead to significant drying of the considerable

626 soil depth, which is consistent with the results shown in Fig. 15. Therefore, to investigate the
627 impact of cracks on deeper soil drying, the same diagrams were plotted for a depth of 1 m
628 below the ground surface, as shown in Fig. 16(c). According to the results, the drying at a depth
629 of 1 m in cracked soil is obviously greater than intact soil, which is more significant near the
630 crack walls. Among the crack scenarios, D2.5R13 consistently showed a 5% lower degree of
631 saturation than D1R5, D1R13, and D1R35 at all distances from the crack wall. However, the
632 crack ratio did not impact the soil depth drying, and all crack scenarios with a crack depth of 1
633 m matched each other. In 60 cm and zero distance, corresponding to the crack wall, the soil
634 saturation in D2.5R13 was 10% and 37.5% lower than intact soil, respectively. Therefore, it
635 can be concluded that the increased drying in cracked soils is mainly due to the intensity of
636 crack effects on moisture redistribution at deeper soil levels.

637 **Conclusions**

638 The focus of this study was on the thermo-hydraulic analysis of cracked soil to investigate the
639 temperature and moisture dynamics, under the soil-atmosphere interaction. The governing
640 equations for unsaturated porous media, including balance and constitutive equations, and
641 those governing soil-atmosphere interaction, were incorporated into a coupled TH formulation.
642 Advective and non-advective fluxes were calculated using Darcy and Fick's law, respectively,
643 to account for water and gas flow, as well as water vapour diffusion. Additionally, heat transfer
644 through the porous medium was considered using Fourier's law and implemented in the
645 CODE_BRIGHT finite element program. In order to utilise the full range of crack geometry
646 data reported from large-scale field studies, small-scale laboratory experiments, and numerical
647 simulations, a dimensionless parameter, crack ratio (C_R), was defined. Based on statistical
648 analysis using the RTx software, the best distribution for the reported crack geometries from
649 several studies was found to be lognormal. Different geometries were selected to cover the
650 entire range of data. Afterwards, crack scenarios were implemented in the numerical model.

651 Meteorological data for Qom city, considered as one of the most prone areas to subsidence in
652 Iran, were obtained for the years 2015 to 2017 and used as input for the numerical model. Boom
653 clay was chosen as the geomaterial for numerical model which is well-justified due to its
654 geotechnical relevance, extensive data availability, and practical applications. Boom clay's
655 unique properties, especially its high sensitivity to desiccation, make it a suitable choice for
656 investigating the complex thermo-hydraulic behaviour of cracked soils. Finally, numerical
657 computations were performed to investigate the interaction between cracked soil and
658 atmosphere, and its impact on temperature and moisture dynamics in the soil. Based on the
659 results, some key conclusions can be drawn as follows:

660 (1) Ground temperature fluctuations induced by climate changes are particularly
661 significant in shallow soil depths (up to about 20 m), and their magnitude depends on
662 soil properties as well as the depth and spacing of cracks. The findings demonstrate that
663 when the air temperature is lower than the equilibrium temperature ($T_a < T_{eq}$), the
664 temperature at a certain soil depth is expected to be higher than T_{eq} . Conversely, when
665 $T_a > T_{eq}$, the temperature at that soil depth will be lower than T_{eq} . This suggests that
666 while the soil is subjected to climatic conditions, the deeper ground temperature is
667 predominantly influenced by history of the thermo-hydraulic loading rather than
668 instantaneous climate variations.

669 (2) The results showed that a specific region can be identified for temperature variations in
670 the ground, where the temperature profile pattern under any climatic conditions and
671 crack scenarios is necessarily bounded to that region. Thus, significant temperature
672 variations can be expected and estimated at every soil depth. In this study, the range of
673 temperature variations was determined for each soil depth and classified based on the
674 crack depth. It should be noted that these are maximum values simulated for the most
675 critical scenarios. Finally, for cracked soil, a surface was defined to estimate the ΔT in

676 the ground up to a depth of 10 m. Additionally, for intact soil, a relationship was
677 proposed to estimate ΔT for each soil depth.

678 (3) Statistical analyses highlighted considerable uncertainty in surface soil temperature due
679 to its sensitivity to climatic changes and surface cracks. However, as the soil depth
680 increases, the uncertainty in temperature variation decreases, and the average soil
681 temperature approaches an equilibrium value.

682 (4) Lastly, the impact of different crack scenarios on soil desiccation due to climatic
683 variations was investigated. A two-dimensional distribution of the degree of saturation
684 was presented to examine the influence of crack width on surface and deeper soil
685 desiccation. The results showed that over time, cracked soil exhibited about 1.4 times
686 higher rates of desiccation than uncracked soil. Among the crack scenarios, deeper
687 cracks demonstrated more severe desiccation rates, about 1.2 times higher, because they
688 lead to the desiccation of a greater depth of soil.

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693 **References**

- 694 1. Wang D, Lu L, Zhang W, Cui P. Numerical and analytical analysis of groundwater
695 influence on the pile geothermal heat exchanger with cast-in spiral coils. Applied
696 energy. 2015;160:705-14. <https://doi.org/10.1016/j.apenergy.2015.04.037>
- 697 2. Lou Y, Fang PF, Xie XY, Chong CS, Li FY, Liu CY, Wang ZJ, Zhu DY. Numerical
698 research on thermal response for geothermal energy pile groups under groundwater

- 699 flow. *Geomechanics for Energy and the Environment*. 2021;28:100257.
700 <https://doi.org/10.1016/j.gete.2021.100257>
- 701 3. Fuentes R, Pinyol N, Alonso E. Effect of temperature induced excess porewater
702 pressures on the shaft bearing capacity of geothermal piles. *Geomechanics for Energy
703 and the Environment*. 2016;8:30-7. <https://doi.org/10.1016/j.gete.2016.10.003>
- 704 4. Guo Y, Zhang G, Liu S. Investigation on the thermal response of full-scale PHC
705 energy pile and ground temperature in multi-layer strata. *Applied Thermal
706 Engineering*. 2018;143:836-48. <https://doi.org/10.1016/j.applthermaleng.2018.08.005>
- 707 5. Faizal M, Bouazza A, McCartney JS, Haberfield C. Effects of cyclic temperature
708 variations on thermal response of an energy pile under a residential building. *Journal
709 of Geotechnical and Geoenvironmental Engineering*. 2019;145(10):04019066.
710 [https://doi.org/10.1061/\(ASCE\)GT.1943-5606.0002147](https://doi.org/10.1061/(ASCE)GT.1943-5606.0002147)
- 711 6. Laloui L, Sutman M. Experimental investigation of energy piles: From laboratory to
712 field testing. *Geomechanics for Energy and the Environment*. 2021;27:100214.
713 <https://doi.org/10.1016/j.gete.2020.100214>
- 714 7. Loria AF, Ravera E, Laloui L. Thermo-hydro-mechanical behavior of energy
715 barrettes: Field experiments and numerical simulations. *Geomechanics for Energy and
716 the Environment*. 2023;34:100451. <https://doi.org/10.1016/j.gete.2023.100451>
- 717 8. Radioti G, Sartor K, Charlier R, Dewallef P, Nguyen F. Effect of undisturbed ground
718 temperature on the design of closed-loop geothermal systems: A case study in a semi-
719 urban environment. *Applied Energy*. 2017;200:89-105.
720 <https://doi.org/10.1016/j.apenergy.2017.05.070>
- 721 9. Dahl MB, Priemé A, Brejnrod A, Brusvang P, Lund M, Nymand J, Kramshøj M, Ro-
722 Poulsen H, Haugwitz MS. Warming, shading and a moth outbreak reduce tundra

- 723 carbon sink strength dramatically by changing plant cover and soil microbial activity.
724 Scientific Reports. 2017;7(1):16035. <https://doi.org/10.1038/s41598-017-16007-y>
- 725 10. Farooq M, Nazir A, Anjum S, Farooq B, Yousuf S. Soil Microbial Biochemical
726 Activity and Influence of Climate Change. In Climate Change and Microbiome
727 Dynamics. 2023;137-155. https://doi.org/10.1007/978-3-031-21079-2_10
- 728 11. Hernández-Lara A, Ros M, Cuartero J, Vivo JM, Lozano-Pastor P, Pascual JA. Effects
729 of solarisation combined with compost on soil pathogens and the microbial
730 community in a spinach cropping system. Agriculture, Ecosystems & Environment.
731 2023;346:108359. <https://doi.org/10.1016/j.agee.2023.108359>
- 732 12. Davidson EA, Janssens IA. Temperature sensitivity of soil carbon decomposition and
733 feedbacks to climate change. Nature. 2006;440(7081):165-73.
734 <https://doi.org/10.1038/nature04514>
- 735 13. Shi BX, Chen SS, Han HQ, Zheng CF. Expansive soil crack depth under cumulative
736 damage. The Scientific World Journal. 2014;2014.
737 <https://doi.org/10.1155/2014/498437>
- 738 14. Zhang C, Lu N. Unitary definition of matric suction. Journal of Geotechnical and
739 Geoenvironmental Engineering. 2019;145(2):02818004.
740 [https://doi.org/10.1061/\(ASCE\)GT.1943-5606.0002004](https://doi.org/10.1061/(ASCE)GT.1943-5606.0002004)
- 741 15. Pouya A, Vo TD, Hemmati S, Tang AM. Modeling soil desiccation cracking by
742 analytical and numerical approaches. International Journal for Numerical and
743 Analytical Methods in Geomechanics. 2019;43(3):738-63.
744 <https://doi.org/10.1002/nag.2887>
- 745 16. Tang CS, Zhu C, Cheng Q, Zeng H, Xu JJ, Tian BG, Shi B. Desiccation cracking of
746 soils: A review of investigation approaches, underlying mechanisms, and influencing

- 747 factors. *Earth-Science Reviews*. 2021;216:103586.
748 <https://doi.org/10.1016/j.earscirev.2021.103586>
- 749 17. Rodríguez R, Sanchez M, Ledesma A, Lloret A. Experimental and numerical analysis
750 of desiccation of a mining waste. *Canadian Geotechnical Journal*. 2007;44(6):644-58.
751 <https://doi.org/10.1139/t07-016>
- 752 18. Yin P, Vanapalli SK, Yu S. Morphological characteristics of desiccation-induced
753 cracks in cohesive soils: a critical review. *Bulletin of Engineering Geology and the*
754 *Environment*. 2022;81(12):503. <https://doi.org/10.1007/s10064-022-03003-4>
- 755 19. Tang CS, Cui YJ, Tang AM, Shi B. Experiment evidence on the temperature
756 dependence of desiccation cracking behaviour of clayey soils. *Engineering Geology*.
757 2010;114(3-4):261-6. <https://doi.org/10.1016/j.enggeo.2010.05.003>
- 758 20. Luo Y, Zhang J, Zhou Z, Victor C. Modelling preferential flow induced by dynamic
759 changes of desiccation cracks: A comparative numerical study. *Geoderma*.
760 2023;433:116471. <https://doi.org/10.1016/j.geoderma.2023.116471>
- 761 21. Morris PH, Graham J, Williams DJ. Cracking in drying soils. *Canadian Geotechnical*
762 *Journal*. 1992;29(2):263-77. <https://doi.org/10.1139/t92-030>
- 763 22. Konrad JM, Ayad R. Desiccation of a sensitive clay: field experimental observations.
764 *Canadian Geotechnical Journal*. 1997;34(6):929-42. <https://doi.org/10.1139/t97-063>
- 765 23. Philip LK, Shimell H, Hewitt PJ, Ellard HT. A field-based test cell examining clay
766 desiccation in landfill liners. *Quarterly Journal of Engineering Geology and*
767 *Hydrogeology*. 2002;35(4):345-54. <https://doi.org/10.1144/1470-9236/2001-37>
- 768 24. Yu Z, Eminue OO, Stirling R, Davie C, Glendinning S. Desiccation cracking at field
769 scale on a vegetated infrastructure embankment. *Géotechnique Letters*. 2021;11(1):88-
770 95. <https://doi.org/10.1680/jgele.20.00108>

- 771 25. Miller CJ, Mi H, Yesiller N. Experimental analysis of desiccation crack propagation in
772 clay liners 1. JAWRA Journal of the American Water Resources Association.
773 1998;34(3):677-86. <https://doi.org/10.1111/j.1752-1688.1998.tb00964.x>
- 774 26. Wang LL, Tang CS, Shi B, Cui YJ, Zhang GQ, Hilary I. Nucleation and propagation
775 mechanisms of soil desiccation cracks. Engineering Geology. 2018;238:27-35.
776 <https://doi.org/10.1016/j.enggeo.2018.03.004>
- 777 27. Ferreira SR, Araújo AG, Barbosa FA, Silva TC, Bezerra IM. Analysis of changes in
778 volume and propagation of cracks in expansive soil due to changes in water content.
779 Revista Brasileira de Ciência do Solo. 2020;44.
780 <https://doi.org/10.36783/18069657rbcs20190169>
- 781 28. Tang CS, Cheng Q, Leng T, Shi B, Zeng H, Inyang HI. Effects of wetting-drying
782 cycles and desiccation cracks on mechanical behaviour of an unsaturated soil. Catena.
783 2020;194:104721. <https://doi.org/10.1016/j.catena.2020.104721>
- 784 29. Zeng H, Tang CS, Zhu C, Vahedifard F, Cheng Q, Shi B. Desiccation cracking of soil
785 subjected to different environmental relative humidity conditions. Engineering
786 Geology. 2022;297:106536. <https://doi.org/10.1016/j.enggeo.2022.106536>
- 787 30. Tian BG, Cheng Q, Tang CS, Shi B. Healing behaviour of desiccation cracks in a
788 clayey soil subjected to different wetting rates. Engineering Geology.
789 2023;313:106973. <https://doi.org/10.1016/j.enggeo.2022.106973>
- 790 31. Amarasiri AL, Kodikara JK. Numerical modelling of a field desiccation test.
791 Géotechnique. 2013;63(11):983-6. <https://doi.org/10.1680/geot.12.P.010>
- 792 32. Sánchez M, Manzoli OL, Guimarães LJ. Modeling 3-D desiccation soil crack
793 networks using a mesh fragmentation technique. Computers and Geotechnics.
794 2014;62:27-39. <https://doi.org/10.1016/j.compgeo.2014.06.009>

- 795 33. Stirling RA, Glendinning S, Davie CT. Modelling the deterioration of the near surface
796 caused by drying induced cracking. *Applied Clay Science*. 2017;146:176-85.
797 <https://doi.org/10.1016/j.clay.2017.06.003>
- 798 34. Levatti HU, Prat PC, Ledesma A. Numerical and experimental study of initiation and
799 propagation of desiccation cracks in clayey soils. *Computers and Geotechnics*.
800 2019;105:155-67. <https://doi.org/10.1016/j.compgeo.2018.09.015>
- 801 35. Houcem T, Alassaf Y, Jamei M, Olivella S. Desiccation cracks prediction using a 3D
802 finite elements model. *International Journal of Geotechnical Engineering*. 2023:1-5.
803 <https://doi.org/10.1080/19386362.2023.2202595>
- 804 36. Guo L, Chen G, Ding L, Zheng L, Gao J. Numerical simulation of full desiccation
805 process of clayey soils using an extended DDA model with soil suction consideration.
806 *Computers and Geotechnics*. 2023;153:105107.
807 <https://doi.org/10.1016/j.compgeo.2022.105107>
- 808 37. Blight GE. Interactions between the atmosphere and the earth. *Géotechnique*.
809 1997;47(4):715-67. <https://doi.org/10.1680/geot.1997.47.4.713>
- 810 38. Gens A. Soil–environment interactions in geotechnical engineering. *Géotechnique*.
811 2010;60(1):3-74. <https://doi.org/10.1680/geot.9.P.109>
- 812 39. Hemmati S, Gatmiri B, Cui YJ, Vincent M. Thermo-hydro-mechanical modelling of
813 soil settlements induced by soil-vegetation-atmosphere interactions. *Engineering*
814 *Geology*. 2012;139:1-6. <https://doi.org/10.1016/j.enggeo.2012.04.003>
- 815 40. An N, Hemmati S, Cui Y. Numerical analysis of soil volumetric water content and
816 temperature variations in an embankment due to soil-atmosphere interaction.
817 *Computers and Geotechnics*. 2017;83:40-51.
818 <https://doi.org/10.1016/j.compgeo.2016.10.010>

- 819 41. Adams JE, Hanks RJ. Evaporation from soil shrinkage cracks. Soil Science Society of
820 America Journal. 1964;28(2):281-4.
821 <https://doi.org/10.2136/sssaj1964.03615995002800020043x>
- 822 42. Ritchie JT, Adams JE. Field measurement of evaporation from soil shrinkage cracks.
823 Soil Science Society of America Journal. 1974;38(1):131-4.
824 <https://doi.org/10.2136/sssaj1974.03615995003800010040x>
- 825 43. Hatano R, Nakamoto H, Sakuma T, Okajima H. Evapotranspiration in cracked clay
826 field soil. Soil Science and Plant Nutrition. 1988;34(4):547-55.
827 <https://doi.org/10.1080/00380768.1988.10416470>
- 828 44. Poulsen TG, Cai W, Garg A. Water evaporation from cracked soil under moist
829 conditions as related to crack properties and near-surface wind speed. European
830 Journal of Soil Science. 2020;71(4):627-40. <https://doi.org/10.1111/ejss.12926>
- 831 45. Poulsen TG. Predicting evaporation from moist, cracked soil, based on near-surface
832 wind speed, crack width and crack distance. European Journal of Soil Science.
833 2022;73(1):e13215. <https://doi.org/10.1111/ejss.13215>
- 834 46. Cuadrado A, Najdi A, Ledesma A, Olivella S, Prat PC. THM analysis of a soil drying
835 test in an environmental chamber: The role of boundary conditions. Computers and
836 Geotechnics. 2022;141:104495. <https://doi.org/10.1016/j.compgeo.2021.104495>
- 837 47. Adams JE, Ritchie JT, Burnett E, Fryrear DW. Evaporation from a simulated soil
838 shrinkage crack. Soil Science Society of America Journal. 1969;33(4):609-13.
839 <https://doi.org/10.2136/sssaj1969.03615995003300040034x>
- 840 48. Selim HM, Kirkham D. Soil temperature and water content changes during drying as
841 influenced by cracks: A laboratory experiment. Soil Science Society of America
842 Journal. 1970;34(4):565-9.
843 <https://doi.org/10.2136/sssaj1970.03615995003400040010x>

- 844 49. Tong F, Jing L, Zimmerman RW. An effective thermal conductivity model of
845 geological porous media for coupled thermo-hydro-mechanical systems with
846 multiphase flow. *International Journal of Rock Mechanics and Mining Sciences*.
847 2009;46(8):1358-69. <https://doi.org/10.1016/j.ijrmms.2009.04.010>
- 848 50. Du C, Yu J, Wang P, Zhang Y. Analysing the mechanisms of soil water and vapour
849 transport in the desert vadose zone of the extremely arid region of northern China.
850 *Journal of Hydrology*. 2018;558:592-606.
851 <https://doi.org/10.1016/j.jhydrol.2017.09.054>
- 852 51. Wen W, Lai Y, You Z. Numerical modeling of water–heat–vapor–salt transport in
853 unsaturated soil under evaporation. *International Journal of Heat and Mass Transfer*.
854 2020;159:120114. <https://doi.org/10.1016/j.ijheatmasstransfer.2020.120114>
- 855 52. Panday S, Corapcioglu MY. Reservoir transport equations by compositional approach.
856 *Transport in Porous Media*. 1989;4:369-93. <https://doi.org/10.1007/BF00165780>
- 857 53. Olivella S, Gens A, Carrera J, Alonso EE. Numerical formulation for a simulator
858 (CODE_BRIGHT) for the coupled analysis of saline media. *Engineering*
859 *computations*. 1996;13(7):87-112. <https://doi.org/10.1108/02644409610151575>
- 860 54. Olivella S, Carrera J, Gens A, Alonso EE. Nonisothermal multiphase flow of brine and
861 gas through saline media. *Transport in porous media*. 1994;15:271-93.
862 <https://doi.org/10.1007/BF00613282>
- 863 55. Fredlund DG, Rahardjo H. *Soil mechanics for unsaturated soils*. United
864 Kingdom: Wiley, 1993.
- 865 56. Gens A, Olivella S. THM phenomena in saturated and unsaturated porous media:
866 Fundamentals and formulation. *Revue française de génie civil*. 2001;5(6):693-717.
867 <https://doi.org/10.1080/12795119.2001.9692323>

- 868 57. Delage P, Sultan N, Cui YJ. On the thermal consolidation of Boom clay. Canadian
869 Geotechnical Journal. 2000;37(2):343-54. <https://doi.org/10.1139/t99-105>
- 870 58. Dehandschutter B, Vandycke S, Sintubin M, Vandenberghe N, Wouters L. Brittle
871 fractures and ductile shear bands in argillaceous sediments: inferences from Oligocene
872 Boom Clay (Belgium). Journal of Structural Geology. 2005;27(6):1095-112.
873 <https://doi.org/10.1016/j.jsg.2004.08.014>
- 874 59. Gens A, Vaunat J, Garitte B, Wileveau Y. In situ behaviour of a stiff layered clay
875 subject to thermal loading: observations and interpretation. In Stiff Sedimentary Clays:
876 Genesis and Engineering Behaviour: Géotechnique Symposium in Print. 2007;123-
877 144. <https://doi.org/10.1680/ssc.41080.0011>
- 878 60. François B, Laloui L, Laurent C. Thermo-hydro-mechanical simulation of ATLAS in
879 situ large scale test in Boom Clay. Computers and Geotechnics. 2009;36(4):626-40.
880 <https://doi.org/10.1016/j.compgeo.2008.09.004>
- 881 61. Bernier F, Li XL, Bastiaens W. Twenty-five years' geotechnical observation and
882 testing in the Tertiary Boom Clay format. In Stiff Sedimentary Clays: Genesis and
883 Engineering Behaviour: Géotechnique Symposium in Print. 2007;223-231.
884 <https://doi.org/10.1680/ssc.41080.0020>
- 885 62. van Genuchten MT. A closed-form equation for predicting the hydraulic conductivity
886 of unsaturated soils. Soil science society of America journal. 1980;44(5):892-8.
887 <https://doi.org/10.2136/sssaj1980.03615995004400050002x>
- 888 63. Mualem Y. A new model for predicting the hydraulic conductivity of unsaturated
889 porous media. Water resources research. 1976;12(3):513-22.
890 <https://doi.org/10.1029/WR012i003p00513>

- 891 64. Sadeghi H, Nasiri H. Hysteresis of soil water retention and shrinkage behaviour for
892 various salt concentrations. *Géotechnique Letters*. 2021;11(1):21-9.
893 <https://doi.org/10.1680/jgele.20.00047>
- 894 65. Hedayati-Azar A, Sadeghi H. Semi-empirical modelling of hydraulic conductivity of
895 clayey soils exposed to deionized and saline environments. *Journal of Contaminant*
896 *Hydrology*. 2022;249:104042. <https://doi.org/10.1016/j.jconhyd.2022.104042>
- 897 66. Darzi AG, Sadeghi H, Zhou C. A soil-brine retention model for wetting processes
898 considering the hysteresis effects. *Transportation Geotechnics*. 2023;41:101032.
899 <https://doi.org/10.1016/j.trgeo.2023.101032>
- 900 67. Delahaye CH, Alonso EE. Soil heterogeneity and preferential paths for gas migration.
901 *Engineering geology*. 2002;64(2-3):251-71. [https://doi.org/10.1016/S0013-](https://doi.org/10.1016/S0013-7952(01)00104-1)
902 [7952\(01\)00104-1](https://doi.org/10.1016/S0013-7952(01)00104-1)
- 903 68. Bear J, Bensabat J, Nir A. Heat and mass transfer in unsaturated porous media at a hot
904 boundary: I. One-dimensional analytical model. *Transport in porous media*.
905 1991;6:281-98. <https://doi.org/10.1007/BF00208954>
- 906 69. Volckaert G, Bernier F, Alonso E, Gens A, Samper J, Villar M, Martin-Martin PL,
907 Cuevas J, Campos R, Thomas H, Imbert C. Thermal-hydraulic-mechanical and
908 geochemical behaviour of the clay barrier in radioactive waste repositories (model
909 development and validation). EUR(Luxembourg) 1996.
- 910 70. Sellers PJ, Mintz YC, Sud YE, Dalcher A. A simple biosphere model (SiB) for use
911 within general circulation models. *Journal of the atmospheric sciences*.
912 1986;43(6):505-31. [https://doi.org/10.1175/1520-](https://doi.org/10.1175/1520-0469(1986)043<0505:ASBMFU>2.0.CO;2)
913 [0469\(1986\)043<0505:ASBMFU>2.0.CO;2](https://doi.org/10.1175/1520-0469(1986)043<0505:ASBMFU>2.0.CO;2)

- 914 71. Noilhan J, Planton S. A simple parameterization of land surface processes for
915 meteorological models. *Monthly weather review*. 1989;117(3):536-49.
916 [https://doi.org/10.1175/1520-0493\(1989\)117<0536:ASPOLS>2.0.CO;2](https://doi.org/10.1175/1520-0493(1989)117<0536:ASPOLS>2.0.CO;2)
- 917 72. Samat S. *Thermomechanical modelling of ground response under environmental*
918 *actions*. Universitat Politècnica de Catalunya 2016.
- 919 73. De Bruyn D, Labat S. The second phase of ATLAS: the continuation of a running
920 THM test in the HADES underground research facility at Mol. *Engineering Geology*.
921 2002;64(2-3):309-16. [https://doi.org/10.1016/S0013-7952\(01\)00109-0](https://doi.org/10.1016/S0013-7952(01)00109-0)
- 922 74. Sadeghi H, Darzi AG, Voosoghi B, Garakani AA, Ghorbani Z, Mojtahedi SF.
923 Assessing the vulnerability of Iran to subsidence hazard using a hierarchical FUCOM-
924 GIS framework. *Remote Sensing Applications: Society and Environment*.
925 2023;31:100989. <https://doi.org/10.1016/j.rsase.2023.100989>
- 926 75. Mahsuli M, Haukaas T. Computer program for multimodel reliability and optimization
927 analysis. *Journal of computing in civil engineering*. 2013;27(1):87-98.
928 [https://doi.org/10.1061/\(ASCE\)CP.1943-5487.0000204](https://doi.org/10.1061/(ASCE)CP.1943-5487.0000204)
- 929 76. Nwankwo C, Ogagarue D. An investigation of temperature variation at soil depths in
930 parts of Southern Nigeria. *American journal of environmental engineering*.
931 2012;2(4):142-7. <https://doi.org/10.5923/j.ajee.20120205.05>
- 932 77. Wang CL, Liu HL, Kong GQ, Ng CW, Wu D. Model tests of energy piles with and
933 without a vertical load. *Environmental Geotechnics*. 2016;3(4):203-13.
934 <https://doi.org/10.1680/jenge.15.00020>
- 935 78. Badache M, Eslami-Nejad P, Ouzzane M, Aidoun Z, Lamarche L. A new modeling
936 approach for improved ground temperature profile determination. *Renewable Energy*.
937 2016;85:436-44. <https://doi.org/10.1016/j.renene.2015.06.020>

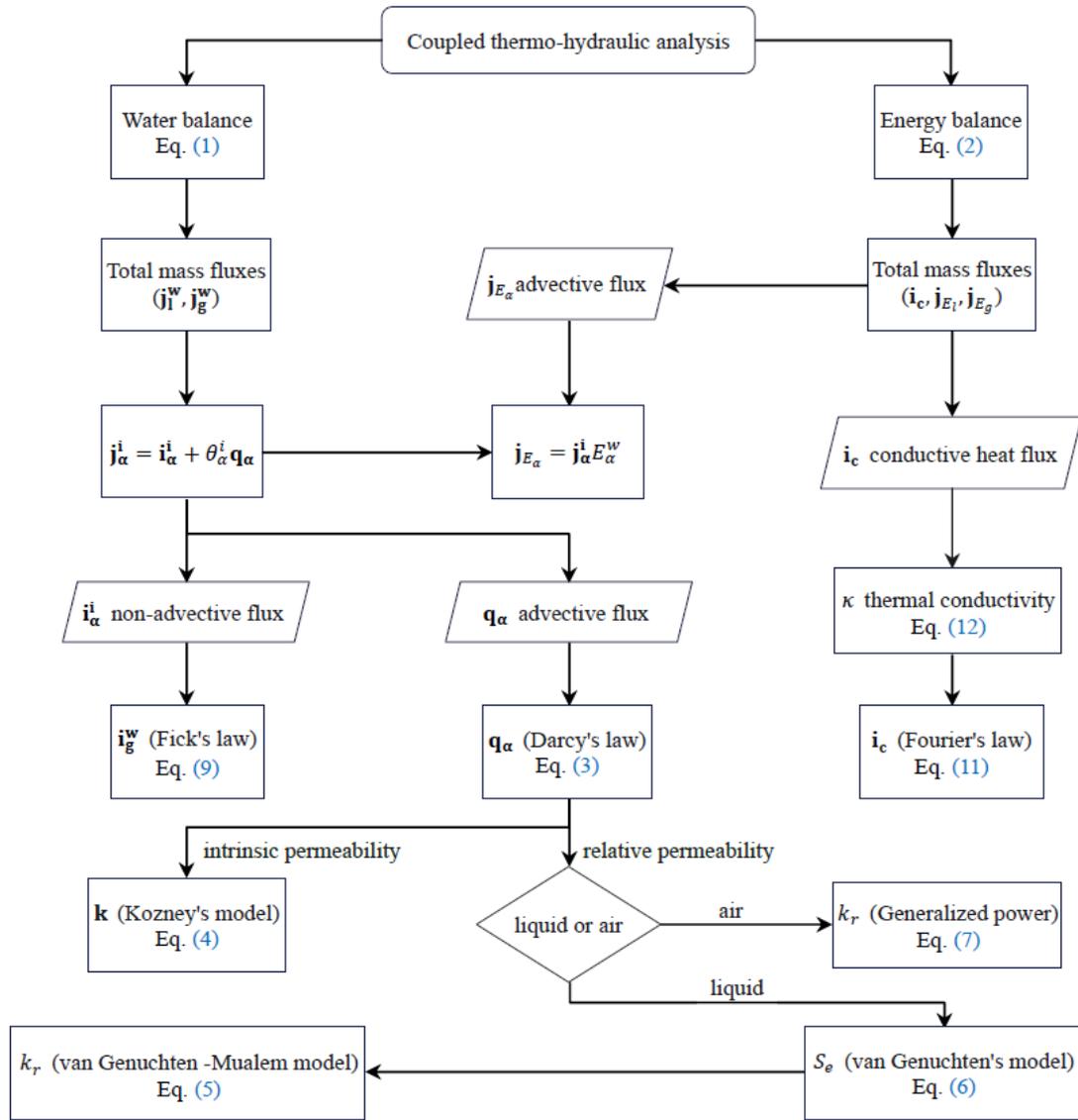


Fig. 1. Flowchart of the relation between balance and constitutive equations.

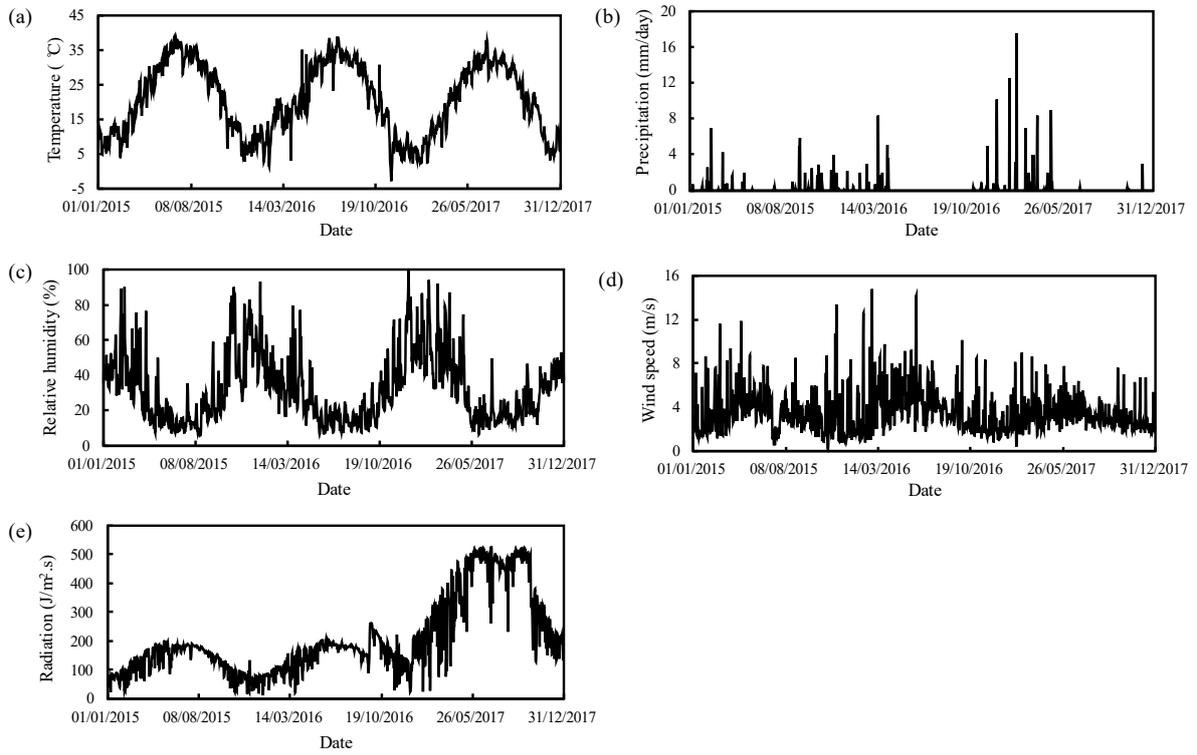


Fig. 2. Climate data for Qom city, (a) temperature, (b) precipitation, (c) relative humidity, (d) wind speed, (e) radiation.

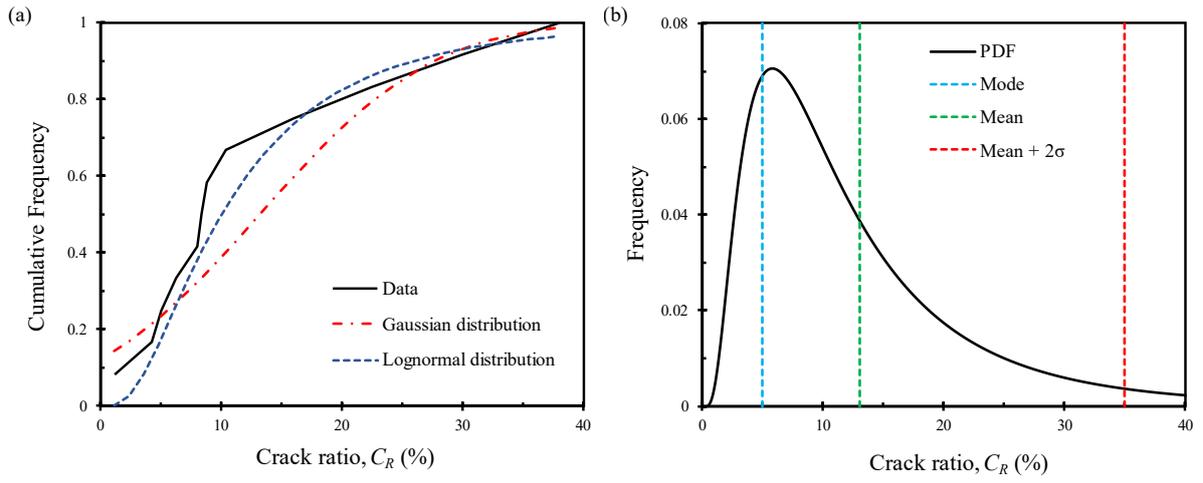


Fig. 3. (a) Gaussian and lognormal distributions and (b) PDF of the crack ratio data.

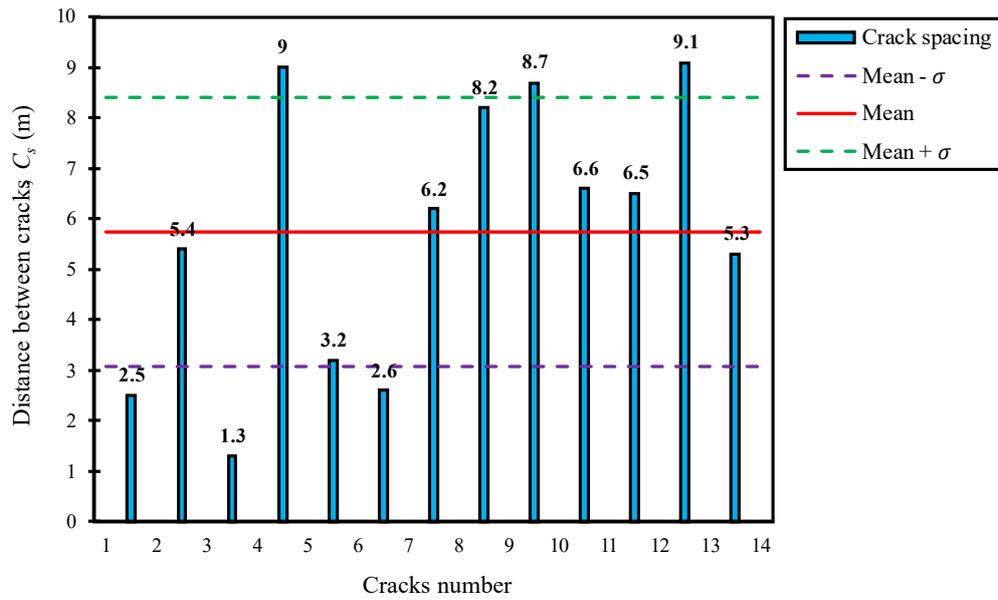


Fig. 4. Randomly generated crack spacing.

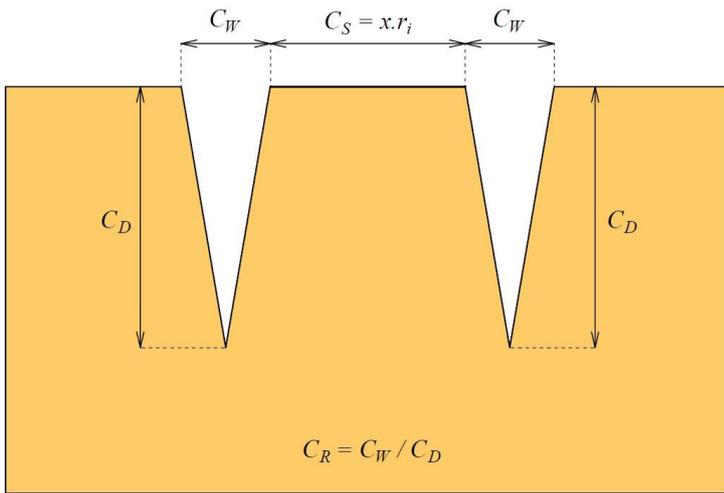


Fig. 5. Schematic diagram of the crack geometry and spacing.

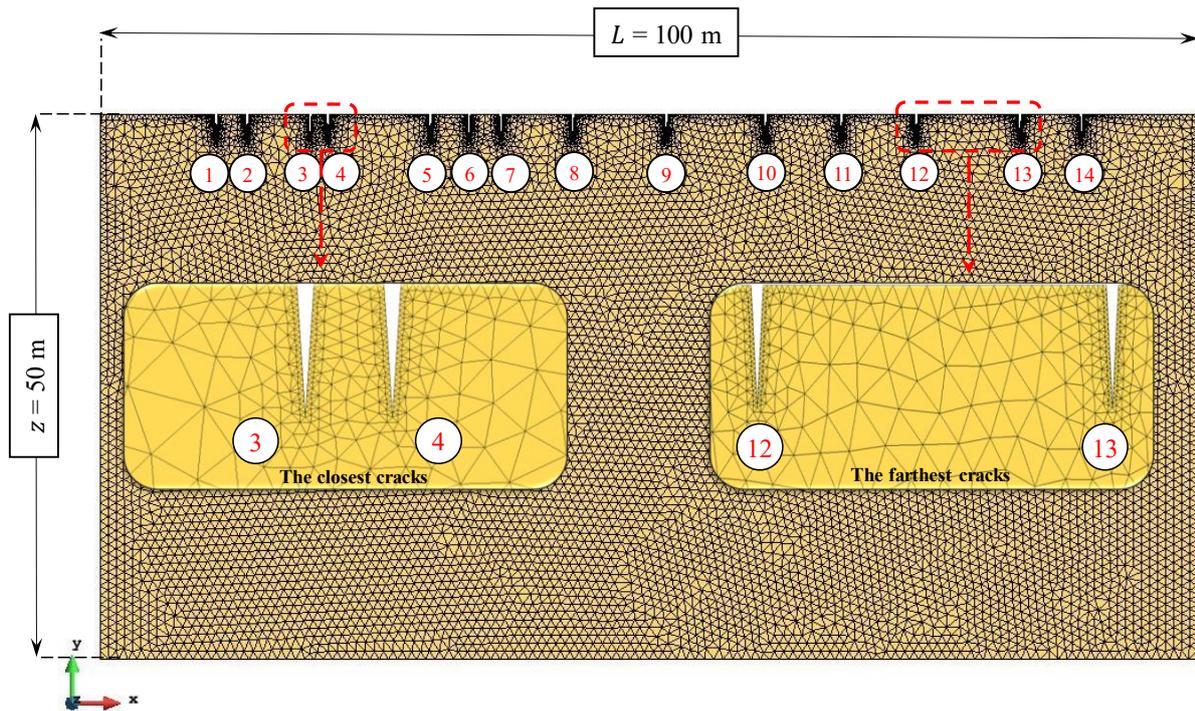


Fig. 6. Model geometry and finite element mesh for model D2.5R13 (Table 3).

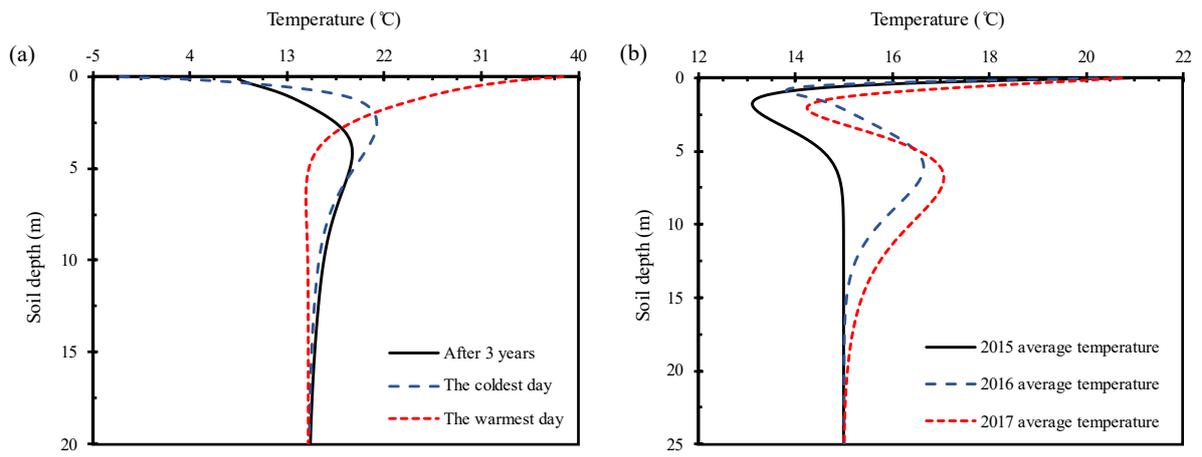


Fig. 7. Effects of climate change on the temperature distribution in the intact soil.

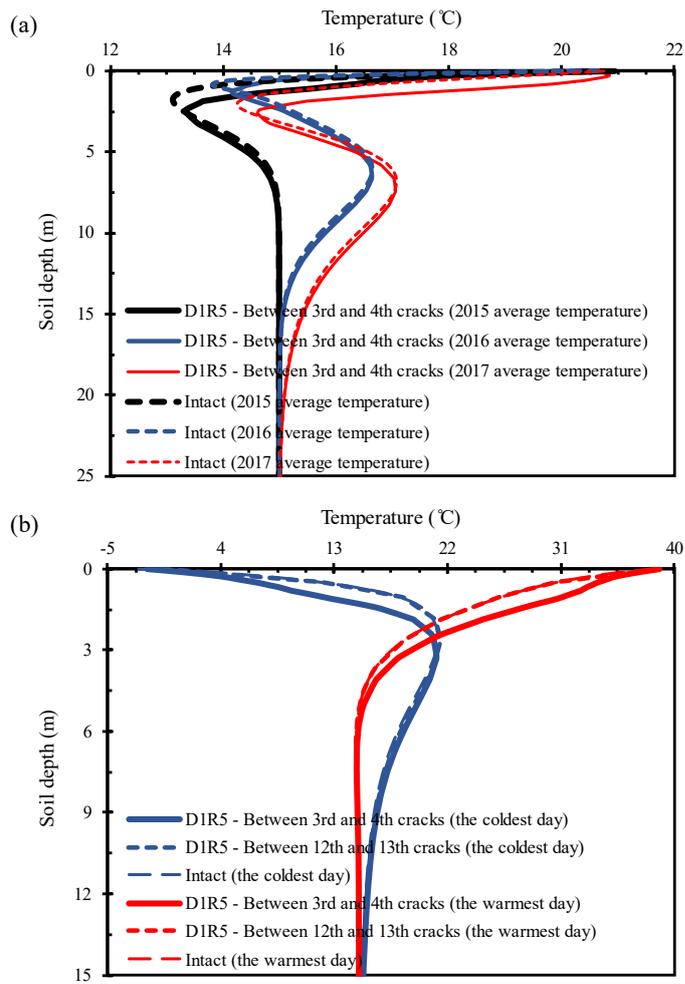


Fig. 8. (a) Comparison of the cracked and intact soil and (b) effect of the crack spacing on temperature distribution.

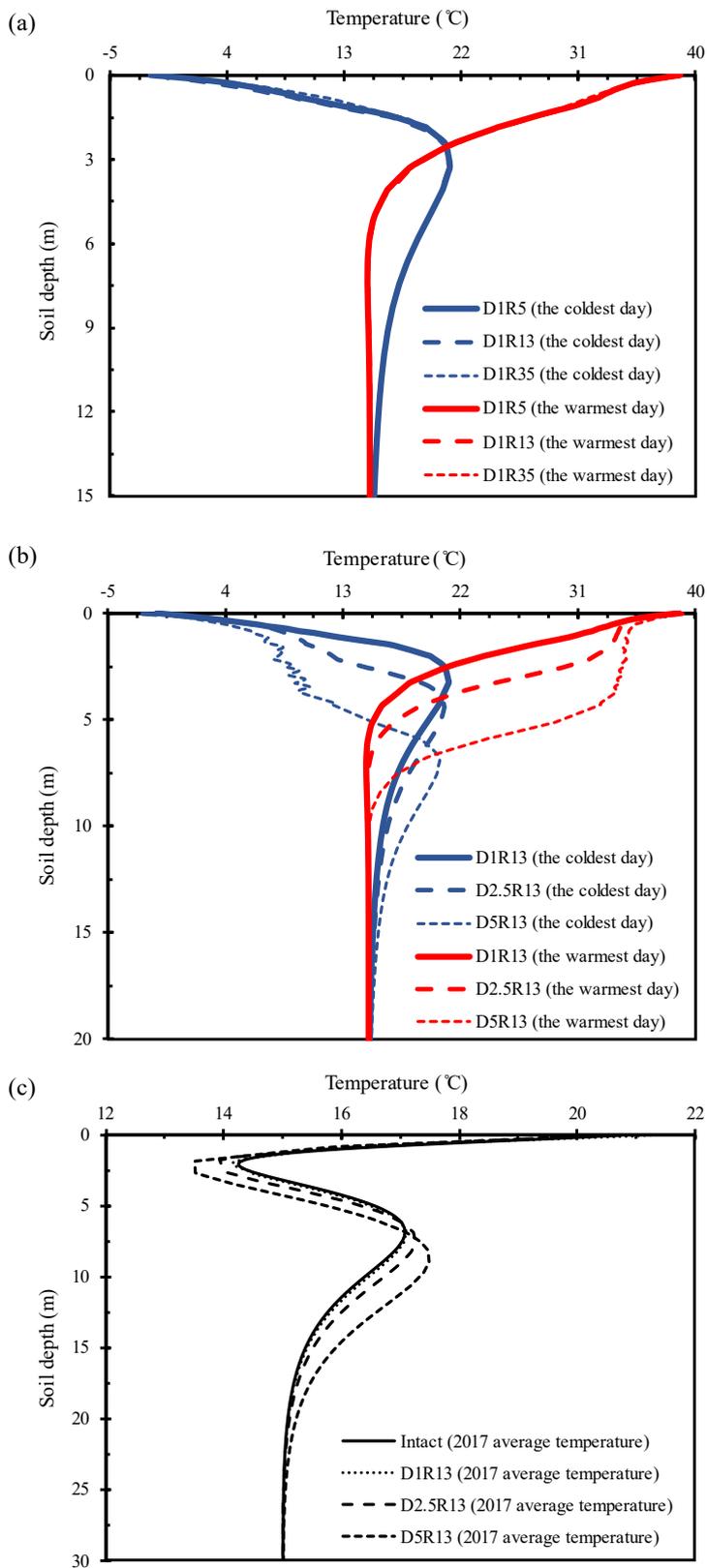


Fig. 9. (a) Crack width and (b), (c) crack depth effects on temperature distribution between 3rd and 4th cracks.

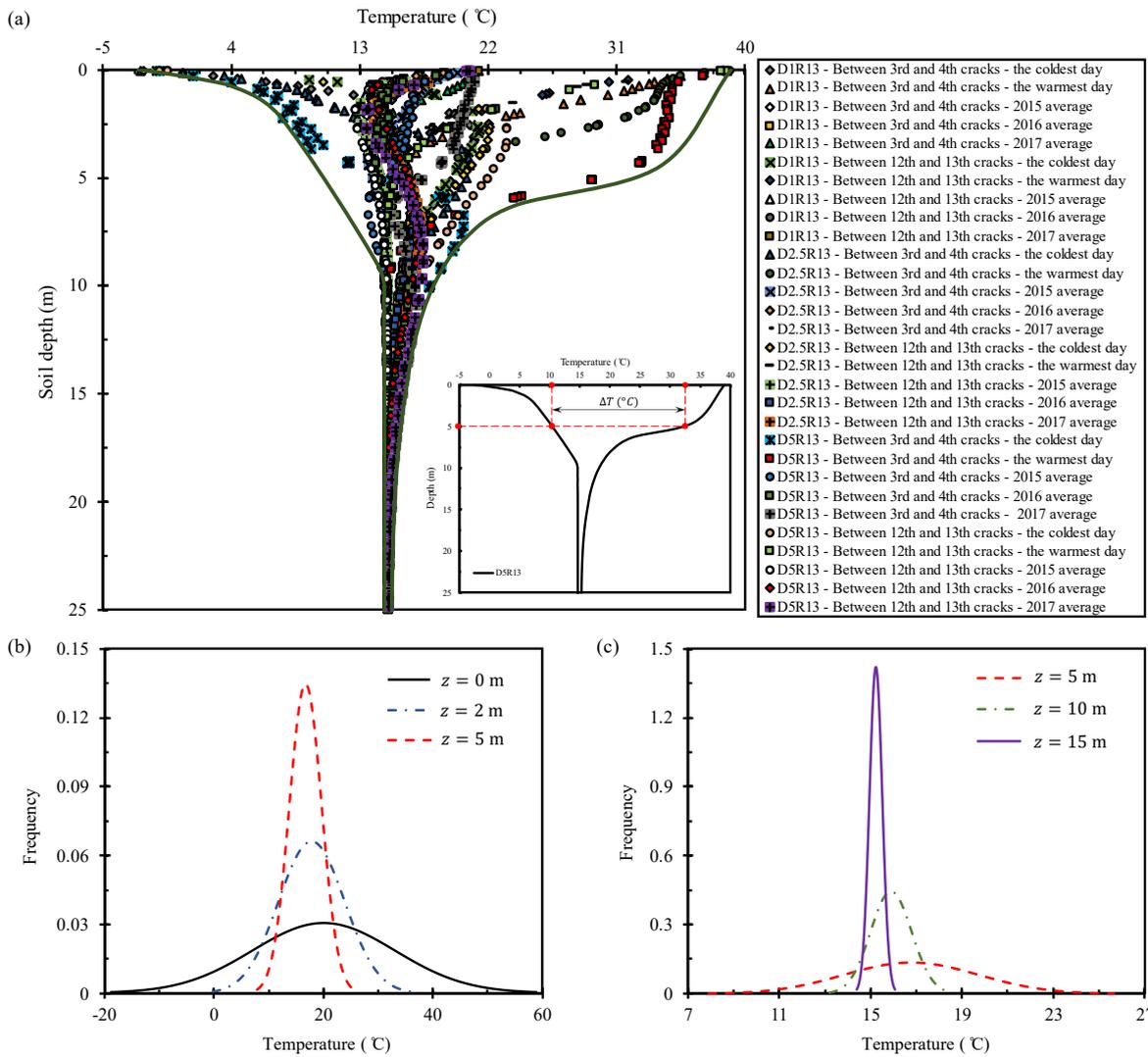


Fig. 10. (a) Variations in temperature for all crack depth and spacing at each climate conditions, and probability density function (PDF) of temperature for different soil depths of (b) 0, 2, 5 m, and (c) 5, 10, 15 m.

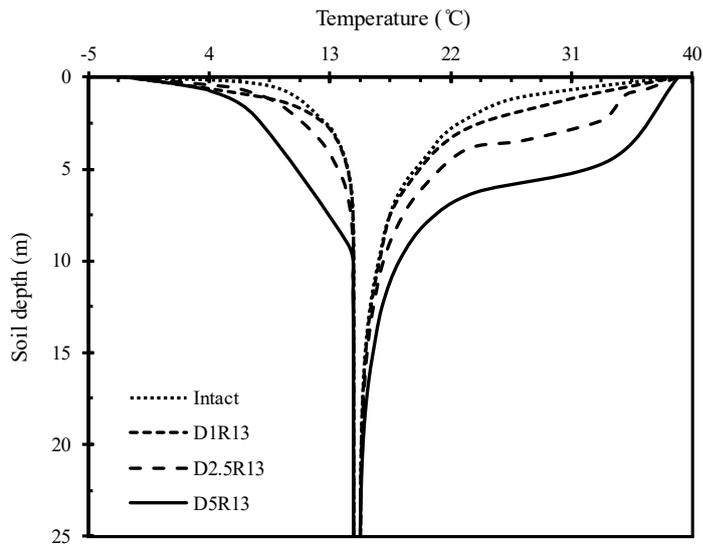


Fig. 11. Variations in temperature for various crack depths.

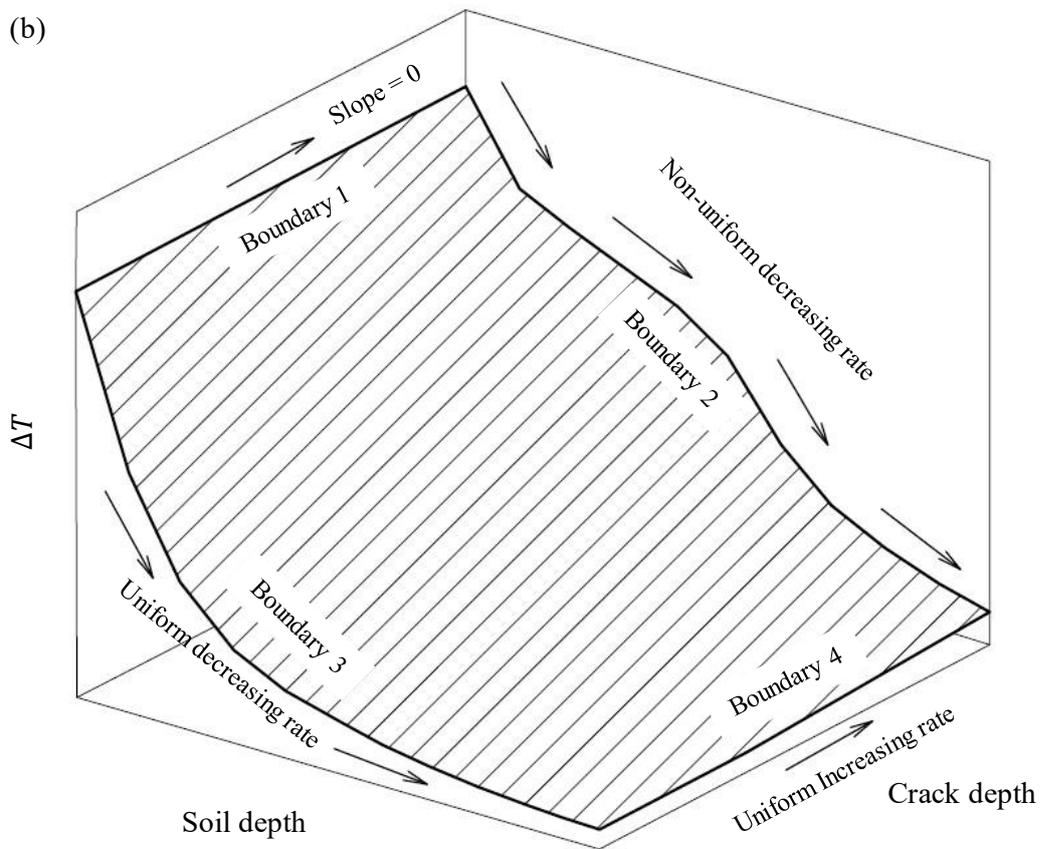
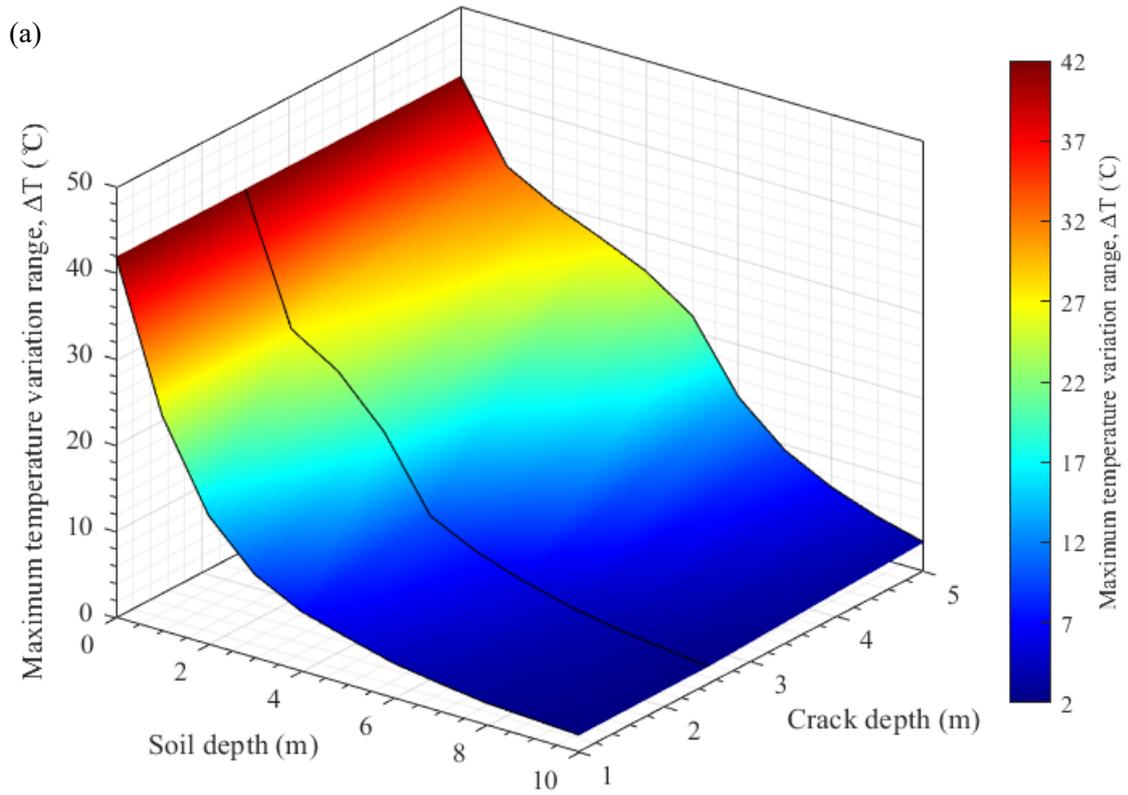


Fig. 12. 3D boundary surface for prediction of the maximum temperature variation against the crack depth and soil depth.

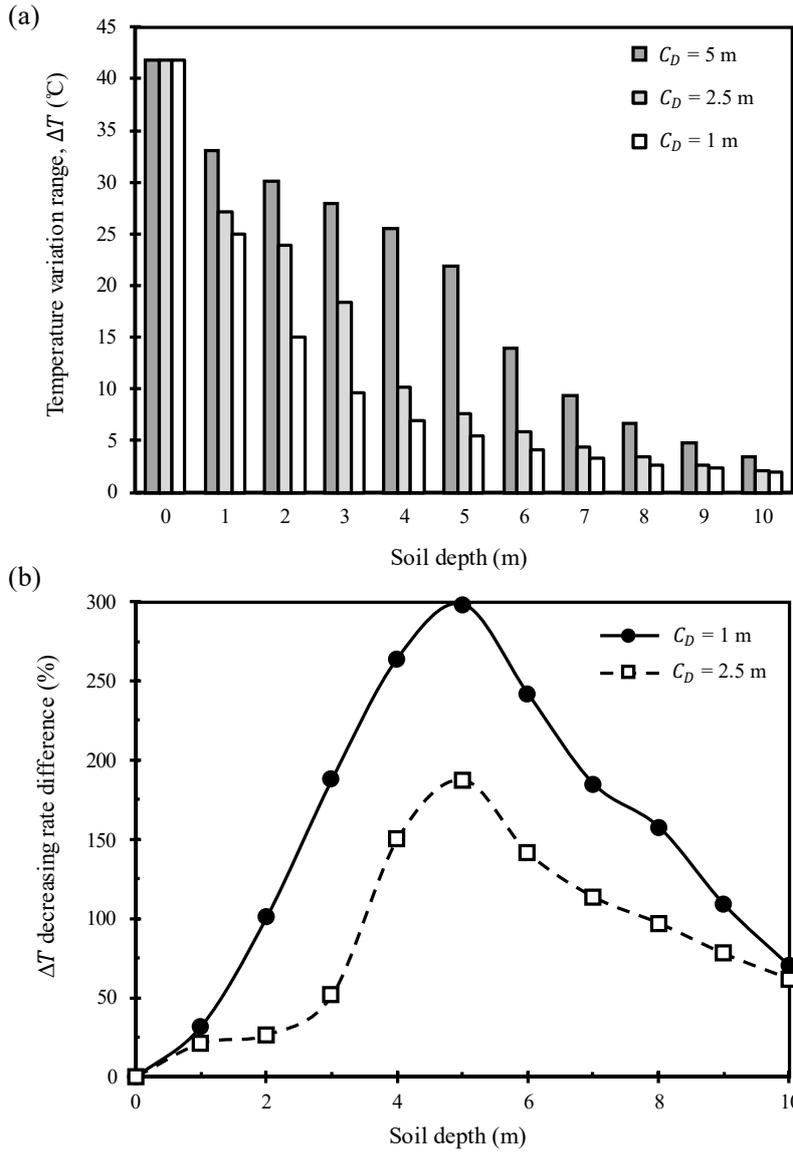


Fig. 13. (a) Difference between ΔT at various soil depths, (b) ΔT decreasing rate difference compared to $C_D = 5$ m.

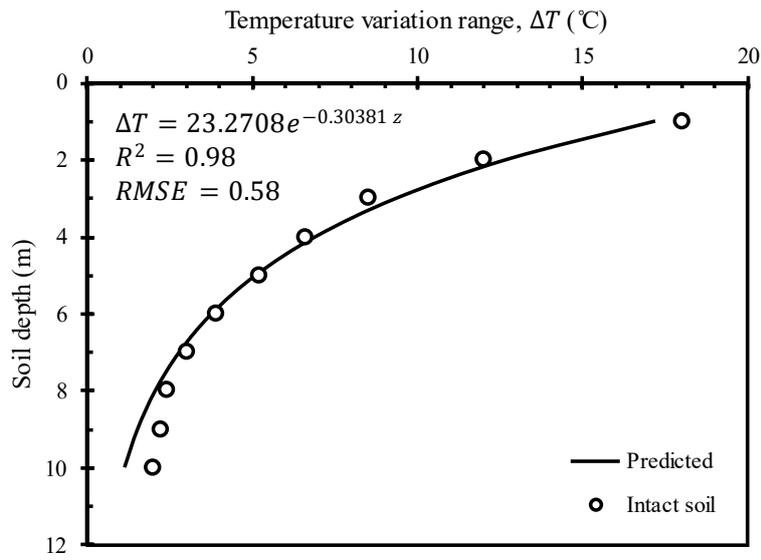


Fig. 14. Variations in temperature in the intact soil, fitted curve and proposed equation.

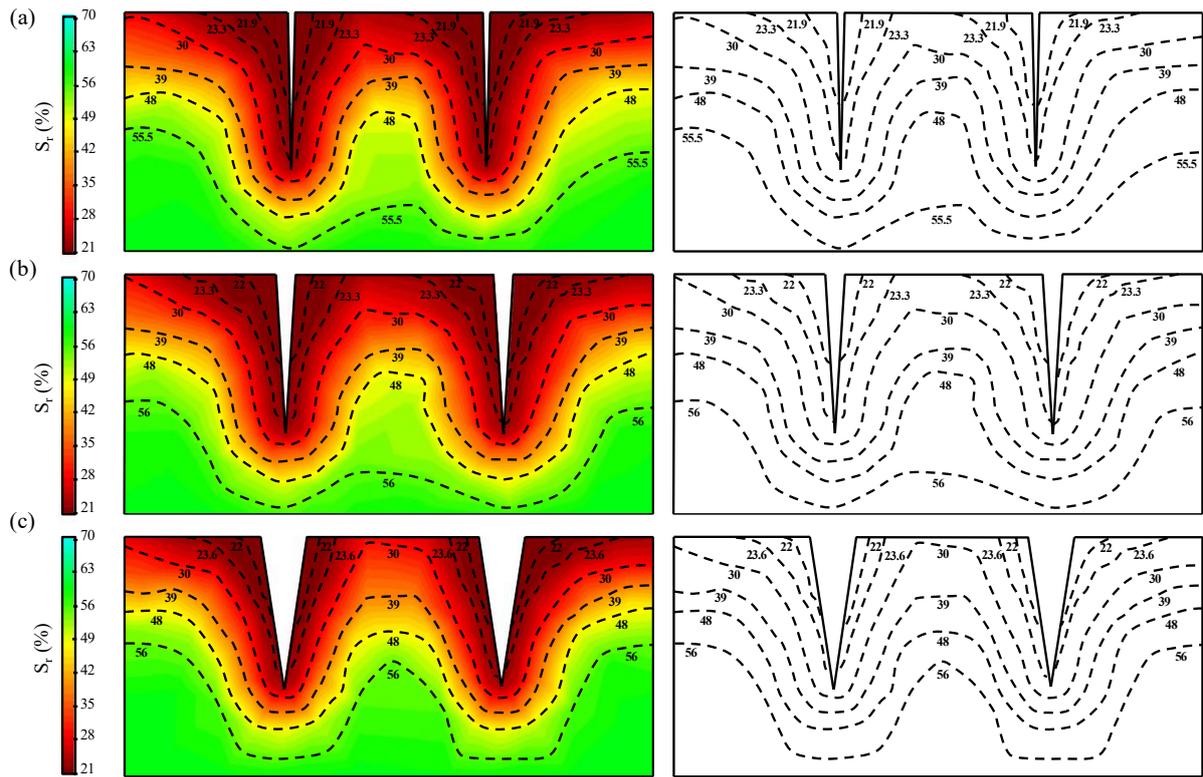


Fig. 15. Two-dimensional distribution of degree of saturation in cracked soil after 3 years, (a) D1R5, (b) D1R13, (c) D1R35 for 3rd and 4th cracks.

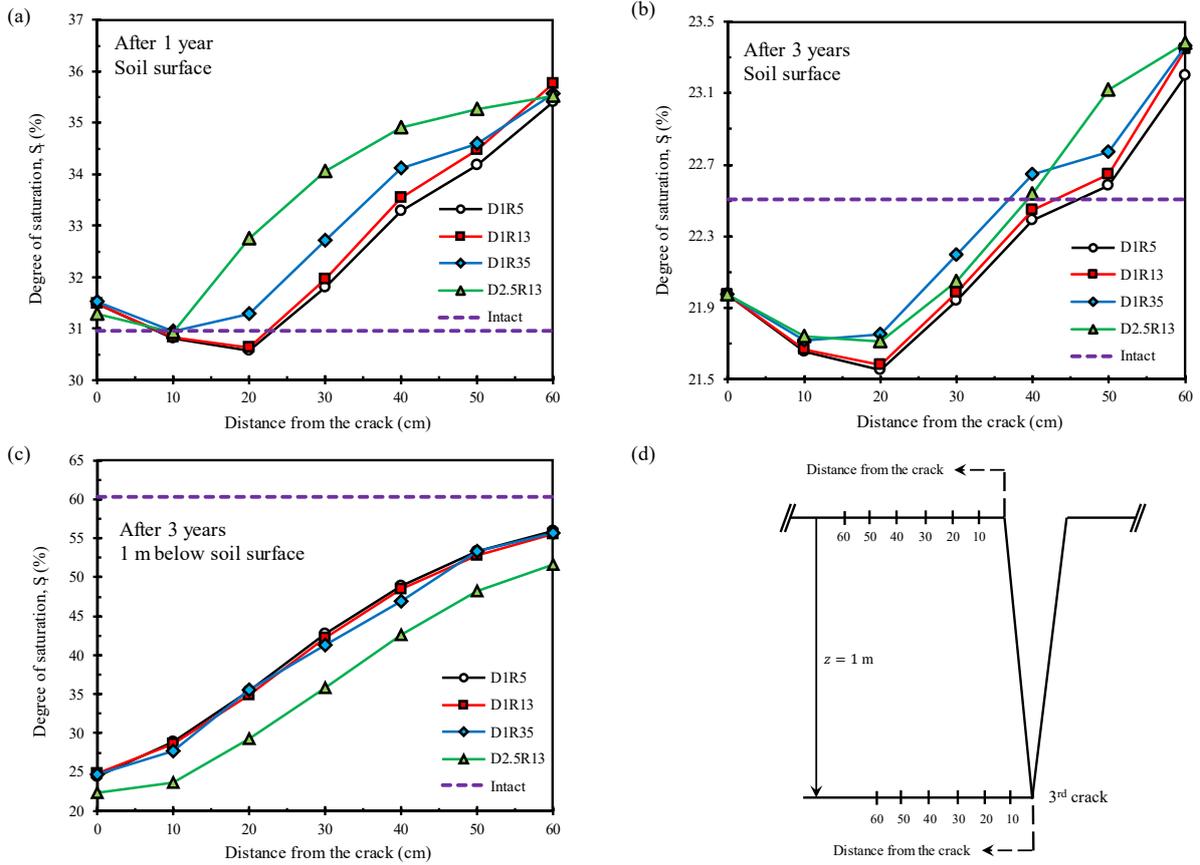


Fig. 16. Drying progress in intact and cracked soil, (a) soil surface drying after 1 year, (b) soil surface drying after 3 years, (c) drying at 1 m depth after 3 years, and (d) schematic diagram defining the distance convention from the 3rd crack.

Table 1

Summary of the constitutive equations and model input values.

Parameter	Description	Value	Equation
Retention curve			
P_0	Measured P at certain temperature (MPa)	3.44	
σ_0	Surface tension (N/m)	0.072	
λ	Shape function for retention curve	0.3	Eq. (6)
S_{rl}	Residual saturation	0	
S_{ls}	Maximum saturation	1	
Liquid phase permeability			
k_0	Intrinsic permeability (m ²)	2.5×10^{-19}	Eq. (5)
λ	Model parameter	0.3	
Gas phase permeability			
A	Constant	1	Eq. (7)
λ	Model parameter	3	
Vapour diffusion			
D	Model parameter (m ² . Pa. s ⁻¹ . K ⁻ⁿ)	5.9×10^{-6}	
n	Model parameter	2.3	Eq. (9)
τ_{au}	Coefficient of tortuosity	1	
Heat conduction			
κ_{dry}	Thermal conductivity of the dry porous medium (W/m ² K)	0.646	
κ_{sat}	Thermal conductivity of the water saturated porous medium (W/m ² K)	1.4396	Eq. (11)

Table 2

Parameters of the crack geometry reported in the previous studies adopted various methodologies.

Crack type	Procedure	C_W (mm)	C_D (mm)	C_R (%)	Reference
Natural	Field observation	50.8 – 76.2	609.6	8.30 – 12.50	[41]
Artificial	Experimental test	6.4 – 19.1	300.0	2.13 – 6.36	[48]
Natural	Field observation	50.0 – 70.0	600.0 – 900.0	7.70 – 8.30	[42]
Natural	Field observation	5.0	400.0 – 500.0	1.00 – 1.25	[43]
Natural	Field observation	N.A.	500.0 – 6000.0	N.A.	[21]
Natural	Field observation	17.0 – 21.0	50.0	34.00 – 42.00	[22]
Natural	Experimental test	10.0	160.0	6.25	[25]
Natural	Field observation	25.0	300.0	8.33	[23]
Natural	Numerical modeling	12.0 – 18.0	300.0	4.00 – 6.00	[31]
Artificial	Experimental test	5.0 – 20.0	50.0 – 100.0	5.00 – 40.00	[44]
Natural	Experimental test	4.5	15.0	30.00	[27]
Natural	Experimental test	0.4 – 1.0	8.0	5.00 – 12.50	[29]
Artificial	Experimental test	4.0 – 10.0	50.0	8.00 – 24.00	[45]

N.A.: Not Applicable

Table 3

Scenarios defined to study the influence of crack geometry.

Name	C_D (m)	C_R (%)	C_W (m)
D1R5	1	5	0.05
D1R13	1	13	0.13
D1R35	1	35	0.35
D2.5R13	2.5	13	0.325
D5R13	5	13	0.65