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3 **Title:**

4 The 'trench pull' force: constraints from elasto-plastic bending models

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The ‘trench pull’ force: constraints from elasto-plastic bending models

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Key Points:

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- Trench pull refers to the net force associated with the pressure deficit beneath the trench, relative to an isostatic column in the plate
- The force can be quantified using an extended concept of ‘GPE*’ which accounts for a pseudo-density (ρ^*)
- Elasto-plastic models are used to constrain the distribution of ρ^* and the magnitude of the trench pull, estimated to be around 2.5 TN m^{-1}

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20 **Abstract**

21 Stresses transmitted through slabs are thought to provide an important component
 22 of the driving force on the trailing plates. This ‘net slab pull’ is usually conceptualised
 23 in terms of in-plane differential stress, acting in the sense of deviatoric tension. However,
 24 an additional component of the net slab pull arises due to the pressure deficit created
 25 by plate downbending. The purpose of this paper is to investigate the mechanics and
 26 typical magnitude of this mechanism, which is termed ‘trench pull’. The challenge is that
 27 because trench topography is non-isostatic, the pressure distribution cannot be treated
 28 with the lithostatic approximation that has been exploited, with much insight, in other
 29 settings. Here, the relative pressure reduction depends on the vertical distribution of *hor-*
 30 *izontal gradients of the vertical shear stress*. These stress gradients are denoted $Q(z)$,
 31 and $Q(z)/g$ can be interpreted as a pseudo-density ρ^* . The concept of the force due to
 32 gravitational potential energy differences (ΔGPE^*) is extended to include the effect of
 33 $\rho^*(z)$. In terms of the contribution to the ΔGPE^* , the distribution of $\rho^*(z)$ functions
 34 exactly like the real density - notably there is the same dependence on the vertical center
 35 of mass. In this study, elastic and elasto-plastic models are used to investigate this
 36 problem, specifically the distribution of the vertical shear stress and its partial deriva-
 37 tives. A key conclusion is that the length scale over which the trench pressure deficit acts
 38 is half the mechanical thickness of the lithosphere. Based on this model, a typical trench
 39 pull force is estimated to be about 2.5 TN m^{-1} . The total topography that exists between
 40 ridges and trenches is associated with a net driving force of about 5 TN m^{-1} , enough to
 41 balance basal drag of 1 MPa, over a plate length of 5000 km.

42 **1 Introduction**

43 A long-standing goal of geodynamics has been to understand the force balance
 44 associated with the motion of the tectonic plates. This has led to the characterisation
 45 and analysis of various distinct contributions (e.g., Lister (1975); Forsyth and Uyeda (1975)).
 46 Some of these, such as the difference in integrated pressure due to isostatic subsidence
 47 (or ridge push) can be estimated with relatively few assumptions, and the magnitudes
 48 are uncontroversial (Lister, 1975; Bird, 1998). Other components are inherently harder
 49 to estimate, and rely on inferences based on different combinations of modelling and con-
 50 straints, with a range of different conclusions resulting. A case in point is the magnitude
 51 of the horizontal force that is propagated directly from slabs to the trailing plates (the
 52 so-called net slab pull) (Forsyth & Uyeda, 1975; Conrad & Lithgow-Bertelloni, 2002).
 53 As previous studies have recognised, stresses propagated through the slab may actually
 54 produce two kinds of driving force in the trailing plate (Richter et al., 1977; Bird, 1998;
 55 Bird et al., 2008; Bercovici et al., 2015). The standard (textbook) conceptual model em-
 56 phasises only one of these - where the net slab pull arises due to an in-plane differential
 57 stress (i.e. deviatoric tension). An additional component of the net slab pull arises due
 58 to the topography of the trench, and has been referred to as ‘trench pull’ (e.g, Bird (1998)).
 59 Of the various mechanisms proposed to contribute to the tectonic force balance, trench
 60 pull is perhaps one of the least well understood (or recognised). Fundamentally this owes
 61 to the challenge of constraining the pressure distribution with depth in the bending plate.
 62 The relative pressure reduction depends the vertical distribution of *horizontal gradients*
 63 *of the vertical shear stress*, and is therefore fundamentally linked to the way in which the
 64 non-isostatic deflection is supported within the plate. The purpose of this study is to in-
 65 vestigate the mechanics, and try to estimate the typical magnitude of this trench pull
 66 force.

67 **2 Background**

68 The correlation between plate velocity and attached slab length, corroborated by
 69 various types of modelling, underpins the current consensus regarding the importance

of slabs in the overall tectonic force balance (Forsyth & Uyeda, 1975; Conrad & Lithgow-Bertelloni, 2002; Saxena et al., 2023). Several modes of slab-plate coupling have been proposed; one widely discussed dichotomy is that of slab pull versus slab suction (Forsyth & Uyeda, 1975; Conrad & Lithgow-Bertelloni, 2002). Suction-type forces, for instance driven by ancient slab masses in the lower mantle, are argued to be important drivers of plate motion (Bird, 1998; Becker & O’Connell, 2001). The current study addresses only the pulling-type mode, which arises due to the slab’s capacity to act as a stress guide (Elasasser, 1969).

The stresses that are available to pull directly on the trailing plate, will arise from a residual of the sum of body forces and tractions acting on the slab (Forsyth & Uyeda, 1975; Bird, 1998). The horizontal component of this residual is often referred to as the ‘net slab pull’. This force will be expressed in a stress anomaly across a section of the plate at the trench. This residual may also contribute to vertical loads (i.e., a shear stress resultant) and bending moments acting on the trailing plate. These are the loading patterns typically associated with the non-isostatic downbending of the trailing plate (Parsons & Molnar, 1976; Turcotte et al., 1978; Turcotte & Schubert, 2002; Garcia et al., 2019).

Because net slab pull arises from a sum of contributions, many involving significant uncertainty, direct estimation is not possible (c.f. ridge push). Instead, various studies (too many to list in full) have sought to infer the relative influence of net slab pull, using a variety of modelling approaches and constraints on both the global and an individual plate scale (Forsyth & Uyeda, 1975; Bird, 1998; Conrad & Lithgow-Bertelloni, 2002; Copley et al., 2010; England & Molnar, 2022). Net slab pull has also been investigated through direct forward modelling, via analysis of the stress/deformation state in the trailing plate (Schellart, 2004; Capitanio et al., 2010; D. Sandiford & Craig, 2023). The results from various approaches have a significant degree of inconsistency: some studies suggest that the net slab pull is similar (within about a factor of 2) to the estimated ridge push in old lithosphere (Forsyth & Uyeda, 1975; Richardson et al., 1979; M. Sandiford et al., 2005; Schellart, 2004; Bird et al., 2008; Copley et al., 2010; England & Molnar, 2022). Others have argued that (depending on assumptions), net slab pull could be between about 3-4 times larger (e.g., van Summeren et al. (2012); Clennett et al. (2023)), to almost an order of magnitude larger (e.g. Conrad and Lithgow-Bertelloni (2002)) than typical ridge push estimates.

Although the magnitude of net slab pull remains debated, the mode of force propagation has typically been less controversial. The prevailing conceptual model is that slabs support an anomalous in-plane differential stress that is transmitted ‘through the bend’ to the trailing plate (Elasasser, 1969; Isacks & Molnar, 1971; Conrad & Lithgow-Bertelloni, 2002; Schellart, 2004; Capitanio et al., 2009). In describing the state of stress in slabs we find frequent reference to slabs undergoing downdip extension or stretching and sometimes as being under tension (Richter et al., 1977; Richardson et al., 1979; Schellart, 2004; Molnar & Bendick, 2019; Spence, 1987). In this study the term ‘in-plane resultant’ (symbolised F_D) encapsulates the stress state envisaged in this paradigmatic model. The subscript D is employed to signify differential or deviatoric. Fig. 1 shows a simple example of a stress state giving rise to positive in-plane resultant (i.e., deviatoric tension).

However, as previous studies have recognised, there is additional mechanism that contributes to the net slab pull (Richter et al., 1977; Bird et al., 2008; Bercovici et al., 2015). Bird (1998) refers to this force as ‘trench pull’, which is the term adopted here. As described in Richter et al. (1977), the force arises due to the topography of trenches:

A driving force may arise in the same way as that at ridges. Because mantle rock is replaced by water, the lithostatic pressure at all depths is reduced¹ by ($\rho_m -$

¹ for reference, this reduction is about 80 MPa, for a relative trench depth of 3.5 km

119 $\rho_w)gH$, where H is the depth of the trench. However, unlike ridges, trenches are
 120 not isostatically compensated and must be maintained by elastic forces. Unfor-
 121 tunately, very little is yet known about the distribution of these stresses. It is not
 122 even clear whether any of the pressure reduction is available to drive the plates.

123 Of the various mechanisms proposed to contribute to the tectonic force balance,
 124 trench pull is perhaps one of the least well understood. Although the development of a
 125 pressure reduction has recognised by previous studies, essential aspects of the mechan-
 126 ics have remained unexplored/undeveloped (Richter et al., 1977; Bird et al., 2008; Bercovici
 127 et al., 2015). The primary reason for this, as mentioned in the above quote, relates to
 128 the non-isostatic nature of trench topography.

129 Fundamentally, there are two key questions we would like to know in regard to trench
 130 pull: 1) can we constrain the typical magnitude of trench pull force from basic princi-
 131 ples/assumptions, in a similar way to ridge push?; and 2) could trench pull represent a
 132 significant component of the net slab pull? The first question is the primary issue ad-
 133 dressed in the current study. Even if we can resolve this first question, the second ques-
 134 tion is inevitably tied to the broader debate regarding the magnitude of the net slab pull.
 135 As we have noted, previous studies show significant divergence on this issue. One method-
 136 ology that has been used to investigate net slab pull is forward subduction modelling (e.g.
 137 Schellart (2004)). Recent numerical subduction modelling suggests that trench pull may
 138 indeed be the dominant ($\sim 75\%$) component of the net slab pull (D. Sandiford & Craig,
 139 2023). It is not yet known if this is a property of subduction models more generally (in-
 140 cluding 3D, or global spherical models). However, it is relatively trivial to test this par-
 141 titioning as part of the retro-analysis of numerical models, as will be discussed in this
 142 paper.

143 In relation to the above quote from Richter et al. (1977), the key conclusions of this
 144 study are:

- 145 • some of the pressure reduction is available to drive the plates
- 146 • pressure is not reduced *at all depths*
- 147 • the pressure deficit equilibrates across the mechanical lithosphere, and is controlled
 148 by the distribution of horizontal gradients in the vertical shear stress (Q)
- 149 • the length scale over which trench pressure deficit acts is half the mechanical thick-
 150 ness of the lithosphere
- 151 • the trench pull magnitude is similar to the estimated ridge push for old lithosphere

152 The organisation of the paper is as follows: Section 2 contains the mathematical
 153 framework and general assumptions. The emphasis is to clearly explain the vertically-
 154 integrated, horizontal force balance on the lithosphere. Section 3 considers the distribu-
 155 tion of vertical shear stress (and its horizontal gradients: Q , or ρ^*) based on an analytic
 156 model of elastic plate flexure. The elasto-perfectly plastic case, is also considered. Sec-
 157 tion 4 combines these results to provide an estimate of the typical trench pull force. Sec-
 158 tion 5 provides a brief discussion on some of the implications, observations and tests that
 159 are relevant to further investigation of the trench pull mechanism.

160 **3 Modelling assumptions and underlying equations**

161 **3.1 preliminaries**

162 Throughout this paper, Earth’s subduction dynamics will be approximated by con-
 163 sidering a 2D, Cartesian domain, assuming plane strain. We therefore refer to the hor-
 164 izontal (x , positive to the right in figures) and vertical (z , positive down), rather than
 165 radial and tangential. We use the continuum-mechanics convention of stress being neg-

166 active in compression (as shown in Fig. 1). Although we have introduced the concept of
 167 trench pull as an effect related to pressure deficit (following (Richter et al., 1977; Bercovici
 168 et al., 2015)), the mathematical analysis is developed in terms of the mean stress, σ_I (i.e.
 169 the negative of the pressure, as shown in Fig. 1).

170 For timescales of interest, the mantle/lithosphere can be treated as being in static
 171 equilibrium. For the symmetric Cauchy stress tensor ($\sigma_{i,j}$, where the first index is the
 172 normal vector), the static balance of forces and moments is expressed through the stress
 173 equilibrium equation:

$$\int_{\Omega} \sigma_{ij} n_j dA + \int_V \rho g \delta_{i,z} dV = 0 \quad (1)$$

174 Using Gauss' theorem this can be written in the equivalent form:

$$\sigma_{ij,j} + \rho \delta_{i,z} g = 0 \quad (2)$$

175 With the gravitational acceleration assumed constant and vertical, the horizontal
 176 component of integrated tractions on any connected subdomain (Ω) must be zero:

$$\int_{\Omega} \sigma_{jx} n_j dA = 0 \quad (3)$$

177 **3.2 The vertically-integrated horizontal force balance**

178 The derivation in this section is simply a representation of the fundamental state-
 179 ment of stress equilibrium (Eq. 3) where the domain is chosen to be representative of
 180 a section of lithosphere. Fig. 2 shows a hypothetical section of a subducting plate, ex-
 181 tending from the trench to an arbitrary seaward location. We make use of 2 coordinate
 182 systems. The z system represents vertical distance from a fixed datum that represents
 183 the average shape of the earth (e.g. the ellipsoid or geoid). z_I represents the isostatic
 184 level of the lithosphere, we can also write: $z_s(x) = z_I + w(x)$, where $w(x)$ is a stan-
 185 dard symbol for the non-isostatic deflection. The z' system denotes distances relative
 186 to the plate surface. This local system is more appropriate for describing quantities like
 187 the mechanical thickness (z'_m), or the thermal thickness (z'_t).

188 The choice of domain allows for a key simplification: for the first three boundaries,
 189 alignment with the coordinate axes means that the only contribution to the traction is
 190 the horizontal normal stress (σ_{xx}) in the case of the vertical boundaries, and the shear
 191 stress ($\sigma_{zx} = \tau_{zx}$), in the case of the basal boundary. For these boundaries only the sign
 192 of the dot product in Eq. 3 is relevant. Denoting the 4 boundaries shown in Fig. 2 as
 193 $\Omega_{0,1,2,3}$, Eq. 3 can be expressed as:

$$\begin{aligned} & - \int_{\Omega_0} \sigma_{xx}(x_0, z) dz + \int_{\Omega_1} \sigma_{xx}(x_1, z) dz \\ & + \int_{\Omega_2} \tau_{zx}(x, z_c) dx - \int_{\Omega_3} \tau_{jx} n_j(x, z_s) ds = 0 \end{aligned} \quad (4)$$

194 Of the 4 boundaries of the domain in Fig. 2, only the top one (Ω_3) may vary in terms
 195 of its angle WRT to the coordinate system. It is useful to make a simple estimate of this
 196 contribution. For subducting plates on Earth, the shear stress of the rock-water inter-
 197 face is of course negligible, the Ω_3 term is dominated by the component of mean stress
 198 acting in the x direction due to the local slope. The total hydrostatic contribution is given
 199 by integrating the hydrostatic stress, from a depth typical of old isostatic lithosphere (e.g,

200 4 km), to a depth of additional trench bathymetry (e.g., + 3.5 km). The resulting net
 201 force, due to pressure acting on the vertical projection of the slope, is about 0.2 TN m^{-1} .
 202 Based on the conclusions of this study, the hydrostatic term is more than an order of mag-
 203 nitude smaller than the trench pull due to the same total deflection.

204 We can simplify the analysis by considering the horizontal force balance on the litho-
 205 sphere and the water column (i.e., combining the green and blue domains in Fig. 2). The
 206 vertical boundaries (Ω_0) now represent the domain extending from z_0 to z_c , and stresses
 207 on (Ω_3) make no contribution to the horizontal force balance. What this choice does is
 208 take the contribution of the pressure acting on the trench slope, and incorporates it as
 209 a small (i.e., second order) change in the quantity we will define as the GPE* at the trench.
 210 The advantage is twofold: we have less terms to consider and all vertical integrals have
 211 a common lower bound.

212 It is important to note that the idea of a compensation level is fundamentally a state-
 213 ment about lateral pressure equilibration relative to the average shape of the earth (i.e.
 214 $z = \text{constant}$), but *not* at the same depth from Earth's surface (z'). Note that the inte-
 215 gration depth in Eq. 4 (z_c) represents a distance relative to the fixed system (z). We now
 216 choose a more compact notation, where an overbar symbol is used to represent the ver-
 217 tical integral from z_0 to z_c , so that Eq. 4 can be written:

$$\bar{\sigma}_{xx}(x_1) - \bar{\sigma}_{xx}(x_0) + \int_{x_0}^{x_1} \tau_{zx}(x, z_c) dx = 0 \quad (5)$$

$$\text{or, } \Delta \bar{\sigma}_{xx} + \int_{x_0}^{x_1} \tau_{zx}(x, z_c) dx = 0 \quad (6)$$

218 This equation says that on a rectangular domain aligned with the axes, and with
 219 tractions negligible along the surface, the difference in integrated horizontal normal stress,
 220 must balance the integrated shear stresses on the base. It is also commonly expressed
 221 in the differential form (Fleitout & Froidevaux, 1983). Although Eq. 6 might be regarded
 222 as the fundamental statement of the horizontal force balance, it is not particularly in-
 223 formative in terms of understanding contributions to the lithospheric force balance. We
 224 now consider an alternative form, first by expanding the normal stress (σ_{xx}) into the de-
 225 viatoric/isotropic parts ($\tau_{xx} + \sigma_I$), and then expanding the mean stress in terms of ver-
 226 tical stress quantities; $\sigma_I = \sigma_{zz} - \tau_{zz}$. Making these substitutions in the LHS of Eq.
 227 6 gives:

$$\Delta(\overline{\tau_{xx} - \tau_{zz}}) + \Delta \bar{\sigma}_{zz} + \int_{\Omega_2} \tau_{zx}(x) dx = 0 \quad (7)$$

228 The first term on LHS of Eq. 7, represents the difference in vertical integral of ($\tau_{xx} -$
 229 τ_{zz}). The term was previously defined as F_D , and represents the the in-plane differen-
 230 tial stress resultant. The second term in Eq. 7 reflects the way vertical normal stress dis-
 231 tribution impacts the integrated mean stress. In this study, we use the symbol GPE*
 232 to represent the negative of quantity $\bar{\sigma}_{zz}$. The reason for the asterisk and sign will be
 233 clarified in the following section. The final term in Eq. 7 is the basal shear force, which
 234 will be represented by F_B . Over the wavelength of the trench topography F_B is second
 235 order; it is retained in the force balance to provide a form that is also relevant for the
 236 plate scale. In symbolic form we have:

$$\Delta F_D - \Delta \text{GPE}^* + F_B = 0 \quad (8)$$

237 A useful property of this representation of the force balance, is that it separates
 238 two distinct contributions that perturb the integrated mean stress (or pressure if one prefers).

239 One of those contributions has already been identified: it arises when the horizontal principal
 240 stress is perturbed, which can be written as $\Delta\sigma_{xx}$, relative to an isostatic background
 241 state (e.g., Turcotte and Schubert (2002)). As is shown in Fig. 1, we can write the fol-
 242 lowing equivalent quantities: $\Delta\sigma_{xx} = \tau_{xx} + \Delta\sigma_I = (\tau_{xx} - \tau_{zz})$. The vertical integral
 243 (resultant) of these quantities is defined as F_D . The left hand term of Eq.8 therefore cap-
 244 tures net forces that arises from a perturbation of the integrated horizontal normal stress;
 245 in plane strain this consists of an equal perturbation of the mean stress ($\Delta\sigma_I$) and a hor-
 246 izontal deviatoric component (τ_{xx}). A separate contribution to the integrated mean stress
 247 arises from the way vertical normal stress is distributed in the lithosphere (GPE*): the
 248 net force due to this effect is represented in the second term of Eq. 8. This term cap-
 249 tures effects due to both isostatic topographic changes (like ridge push) as well as the
 250 non-isostatic topographic effects, including the trench pull force. However, if the ΔGPE^*
 251 is approximated by assuming a lithostatic vertical normal stress, it does not provide a
 252 valid description of the non-isostatic case.

253 3.3 The vertical force balance, Q and ρ^*

254 We have derived a form of the horizontal force balance that includes vertical stress
 255 terms. In order to estimate the trench pull - the ΔGPE^* that exists between the trench
 256 and an isostatic column of lithosphere - we need to develop a model for the distribution
 257 of vertical normal stress in each column. Expanding Eq. 2 for the z component, yields:

$$\frac{\partial\sigma_{zz}}{\partial z} + \frac{\partial\tau_{xz}}{\partial x} + \rho g = 0 \quad (9)$$

258 Integration of Eq. 9 from the vertical origin (z_0) to an arbitrary depth (z) yields
 259 the distribution of the vertical normal stress, where ζ is a dummy variable:

$$\sigma_{zz}(x, z) = - \int_{z_0}^z \rho(x, \zeta) g d\zeta - \int_{z_0}^z \frac{\partial\tau_{xz}}{\partial x}(x, \zeta) d\zeta \quad (10)$$

260 To compact the notation, we will use $Q(x, z)$ to symbolise the horizontal derivative of
 261 the vertical shear stress:

$$Q = \frac{\partial\tau_{xz}}{\partial x} \quad (11)$$

262 We now define a quantity which we will refer to as pseudo-density (symbolised ρ^*) and
 263 given by:

$$\rho^* = \frac{Q}{g} \quad (12)$$

264 The definition of ρ^* allows us to write:

$$\sigma_{zz}(x, z) = -g \int_{z_0}^z (\rho(x, \zeta) + \rho^*(x, \zeta)) d\zeta \quad (13)$$

265 The primary purpose for introducing ρ^* is that it: 1) makes the magnitudes as-
 266 sociated with the non-isostatic support far more intuitive; 2) allows us to take advan-
 267 tage of the existing framework for analysing the forces related to differences in gravita-
 268 tional potential energy (the symbol GPE^* is used to reflect the inclusion of ρ^*). Fun-
 269 damentally however, the concept of ρ^* is a convenience; any appearance of ρ^* in the re-
 270 mainder of the paper can be substituted for the intrinsic quantity *i.e.*, $(\frac{1}{g} \frac{\partial\tau_{xz}}{\partial x})$.

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3.4 Flexural Isostasy

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A fundamental principal of geodynamics is that beneath the Earth's strong outer layer there exists a region where vertical normal stresses are approximately equal (i.e. an isobaric compensation level). Of course, this hydrostatic approximation neglects the 'dynamic' topography that arises from variation in normal stresses, associated with flow in the mantle. However, trenches are not viewed as being 'dynamic topography' in this sense, and we therefore make the normal assumption that trench deflection is completely supported due to the presence of a vertical shear stress within the lithosphere (Turcotte & Schubert, 2002). The compensation principle requires that the LHS of Eq. 10 (or 13) is constant for all lithospheric columns extending to the compensation level. Isostatic compensation occurs when the resultant of Q is zero. For all such columns, the weight of material above the compensation level is equal. When Q has a finite resultant, the lithosphere must deflect vertically (w) from its reference level, so that the lithostatic term changes accordingly, leaving the LHS unperturbed.

For a given deflection from the isostatic level, we can approximate the change in the lithostatic term as $-(\rho_m - \rho_w)gw$. This expresses the fact that vertical motion of a column results in the exchange of between material at the compensation level, and the material above the surface of the lithosphere. For flexure of the oceanic lithosphere, this is the exchange of mantle rock with seawater. The sign is due to the fact that for a positive w there is a loss of weight in the column. We therefore have the relationships:

$$\int_{z_0}^{z_c} \frac{\partial \tau_{xz}}{\partial x}(x, z) dz = (\rho_m - \rho_w)gw(x) \quad (14)$$

$$\int_{z_0}^{z_c} \rho^*(x, z) dz = (\rho_m - \rho_w)w(x) \quad (15)$$

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Eq. 15 allows us to make a simple estimate of the magnitude of the pseudo-density that is required to support trenches. For this we use the reference parameters shown in Table 1. If ρ^* is assumed to be constant, all the way to a compensation level, we have $\rho^* \approx 80 \text{ kg m}^{-3}$. Note that this is the lower bound, where the shear stress gradients are uniformly distributed down to the compensation level. This constant distribution would also violate the free surface boundary conditions. More accurate models are developed on the next section. Note that, in general, ρ^* can be positive or negative. Around the outer rise, where there is non-isostatic uplift, ρ^* would be negative in order to compensate the excess real density in the column.

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If we exchange the order of integration and differentiation in Eq. 14, the connection with the vertical force balance as expressed in the thin plate flexure model becomes clear. In thin plate flexure, the integral of the vertical shear stress across the plate, is termed the vertical shear stress resultant, and is usually symbolised V (Turcotte & Schubert, 2002):

$$\frac{\partial}{\partial x} \int_{z_0}^{z_c} \tau_{xz}(x) dz = (\rho_m - \rho_w)gw(x) \quad (16)$$

$$\frac{\partial}{\partial x} V(x) = (\rho_m - \rho_w)gw(x) \quad (17)$$

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3.5 Connection between the vertical and horizontal force balance

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We can now define an extended concept of the 'GPE*', which is the 'potential energy' that would be associated with the distribution of real and *and* pseudo density:

$$\text{GPE}^*(x) = -\bar{\sigma}_{zz}(x) \quad (18)$$

$$= -\int_{z_0}^{z_c} \sigma_{zz}(x, z) dz \quad (19)$$

$$= g \int_{z_0}^{z_c} \left(\int_{z_0}^z (\rho(x, \zeta) + \rho^*(x, \zeta)) d\zeta \right) dz \quad (20)$$

308 This can be transformed into a single integral by reversing the order of integration, where
 309 the density distribution is weighted by the height above the compensation level ($z_c -$
 310 z):

$$\text{GPE}^*(x) = g \int_{z_0}^{z_c} (\rho(x, z) + \rho^*(x, z)) (z_c - z) dz \quad (21)$$

311 The quantity $(\rho(x, z) + \rho^*(x, z))$ can be thought of as a ‘corrected density’. As detailed
 312 in the appendices, the GPE^* represents the first moment of the corrected density around
 313 the compensation level. The first moment is equivalent to the center of mass of the dis-
 314 tribution, weighted by the integrated value. Any distribution of ρ^* (or ρ) that is sym-
 315 metric around a point, and has the same integrated value, will make the same contri-
 316 bution to the GPE^* . This relationship is emphasised in later parts of the paper.

317 3.6 Reference parameters

318 The main objective of this paper is to develop an estimate for the magnitude of the
 319 trench pull force, or the ΔGPE^*) between the lithosphere the trench compared with an
 320 isostatic reference column. The final expression (which is presented in Section 4) depends
 321 on 2 main parameters: the relative depth of the trench (w_T), and the mechanical thick-
 322 ness of the lithosphere (z'_m). Based on theoretical and empirical constraints, both of these
 323 values are positively correlated with the age of the lithosphere (Goetze & Evans, 1979;
 324 Grellet & Dubois, 1982). In discussing a typical value for trench pull, our attention will
 325 focus on capturing the behaviour of older lithosphere (> 80 Myr). This as similar to the
 326 way in which the ridge push force is usually quoted in the range of 2.5-3.5 TN m^{-1} , which
 327 is an estimate applicable to the subsidence of old lithosphere (Lister, 1975; Turcotte &
 328 Schubert, 2002; Coblenz et al., 2015). Global studies of trench bathymetry suggest that
 329 relative trench depths for older lithosphere lie in the range of about 2.5 - 5.5 km and ex-
 330 hibit a positive correlation with the age of the subducting plate (Grellet & Dubois, 1982).
 331 A value of 3.5 km is chosen as representative for old lithosphere, but clearly there are
 332 significant variations around this value (Grellet & Dubois, 1982; Zhang et al., 2014; Lemenkova,
 333 2019).

334 4 The distribution of vertical shear stress in bending plates

335 In the previous section we showed that the coupling between the non-isostatic plate
 336 deflection (downbending) and the resulting ΔGPE^* , depends on the depth distribution
 337 of the shear stress gradient (Q , or ρ^*). In this section we discuss solutions for the depth
 338 distribution of the vertical shear stress for uniform elastic plates, and consider the case
 339 of elasto-plastic flexure.

340 4.1 Elastic plates

341 Vertical shear stresses are a fundamental element of the thin plate flexure model.
 342 However, in this framework, shear stresses only appear in terms of the resultant quan-
 343 tity (V), e.g., Eq. 17. Analytic solutions that describe the vertical shear stress distri-
 344 bution can be derived through Airy’s method (or stress functions). These are detailed

Name and symbol	Explanation	Related equation / reference value
$z(x)$	surface of plate	- [km]
z_I	depth of isostatic lithosphere	- [km]
$w(x)$	deflection relative to z_I	- [km]
w_T	deflection at trench	3.5 [km]
z'_m	mechanical thickness	60 [km]
z'_{np}	neutral plane depth	30 [km]
z'_t	thermal thickness	100 [km]
z_c	compensation level	$z_I + z'_t$ [km]
$\Delta\sigma_{xx}$	in plane differential stress	$\equiv (\tau_{xx} - \tau_{zz})$ [MPa]
F_D	in-plane resultant	$(\tau_{xx} - \tau_{zz})$ [TN m ⁻¹]
$Q(x, z)$	shear stress gradient	$\equiv \frac{\partial \tau_{xz}}{\partial x}$ [N m ⁻³]
$\rho^*(x, z)$	pseudo-density	$\frac{Q}{g}$ [kg m ⁻³]
GPE*	negative vert. normal stress integral	$-\bar{\sigma}_{zz}$ [TN m ⁻¹]
g	gravity	9.8 [m/s ²]
ρ_m	mantle density	3300 [kg m ⁻³]
ρ_w	water density	1000 [kg m ⁻³]

Table 1. Symbols, definitions, reference parameters and standard units used in this paper. Overbars represent vertical integration across the lithosphere.

345 in continuum mechanics references, where simple loading examples are discussed
 346 (Goodier & Timoshenko, 1970). We can approach the solution more directly however,
 347 with only the usual assumptions for thin plate flexure (plane bending, zero shear stress
 348 on the upper and lower edge) and the stress equilibrium equations. In this section the
 349 vertical coordinate (z') has its origin at the center of the plate, the orientations are posi-
 350 tive down and to the right. As outlined in Appendix A, the distribution of vertical shear
 351 stress for an elastic plate of thickness h is parabolic:

$$\tau_{xz}(x, z') = \frac{V(x)}{I} \left(\frac{z'^2}{2} - \frac{h^2}{8} \right) \quad (22)$$

352 where I is the (2D) first moment of the area. Note that in Eq. 22, V represents the shear
 353 stress resultant, meaning the expression on the RHS (excluding V) defines a unit parabola:

$$\hat{\varphi}(z') = \frac{1}{I} \left(\frac{z'^2}{2} - \frac{h^2}{8} \right) \quad (23)$$

$$\int_{-\frac{h}{2}}^{\frac{h}{2}} \hat{\varphi}(z') dz = 1 \quad (24)$$

354 Because $\hat{\varphi}$ is independent of x , the horizontal gradient is also parabolic (e.g., Tanimoto
 355 (1957)):

$$\frac{\partial \tau_{xz}(x, z')}{\partial x} = Q(x, z') = \hat{\varphi}(z') \frac{dV(x)}{dx} \quad (25)$$

$$= -\hat{\varphi}(z') f(x) \quad (26)$$

356 where we have used $\frac{dV(x)}{dx} = -f$, i.e., the expression of vertical force balance in terms
 357 of the shear stress resultant (see Eq. 17, or Appendix A). Eq. 26 states that for a uni-
 358 form 2D elastic plate under the given boundary conditions, the vertical shear stress is
 359 always parabolic, and that along the plate, the parabola stretches with a gradient that
 360 is proportional to the load (f). Extrapolating this to a uniform elastic lithosphere, and
 361 taking f to be the isostatic restoring force, we can write:

$$Q(x, z') = \hat{\varphi}(z')(\rho_m - \rho_w)gw(x) \quad (27)$$

$$\rho^*(x, z') = \hat{\varphi}(z')(\rho_m - \rho_w)w(x) \quad (28)$$

362 where we have used the fact that a positive (downwards) deflection produces a negative
 363 (upward) restoring force.

364 When thin-plate models are applied to subduction zones, the loading pattern typi-
 365 cally consists of a combination of end loads (e.g, V_0), end moments (e.g, M_0), as well
 366 as a variable normal load due to the isostatic restoring force (Turcotte & Schubert, 2002).
 367 However, to visualise the stress distributions in plane bending, a simpler loading pat-
 368 tern is sufficient. Fig. 3 shows a schematic diagram of the deflection of an elastic plate
 369 by a uniformly distributed normal force. The right hand boundary is free, the left bound-
 370 ary is clamped. The deflection, as well as the maximum horizontal stress (σ_{xx}^{Max}) and
 371 shear stress (τ_{xz}^{Max}) have analytic solutions, as described in the Figure caption. The up-
 372 per panels of Fig. 4 show the vertical distribution of (normalised) stress quantities at
 373 2 points in the elastic domain (e1, e2). These profiles emphasise the relationships pre-
 374 viously developed in this section. Of particular importance is the parabolic distribution
 375 of $Q(z')$. This implies an identical shape for $\rho^*(z')$, which reaches its maximum at (and
 376 is symmetric around) the plate center.

377 4.2 Extension to elastic-plastic plates

378 In the trench region, the subducting plate is expected to undergo comprehensive
 379 yielding and approaches moment saturation. This behaviour is predicted from yield stress
 380 envelopes (YSEs) (Chapple & Forsyth, 1979; McNutt & Menard, 1982; Craig et al., 2014),
 381 and exhibited in numerical models which incorporate similar constitutive models (Bessat
 382 et al., 2020; D. Sandiford & Craig, 2023). Yielding has an important impact on the depth
 383 distribution of vertical shear stress (and its gradients) as has been highlighted in engi-
 384 neering literature on bending plates (Horne, 1951; Drucker, 1956). To demonstrate, we
 385 follow the approach of Horne (1951), and make the ad-hoc assumption that shear stresses
 386 in the bending elastic plate are truncated at a prescribed limit, giving rise to the plas-
 387 tic zones shown in grey in Fig. 3, and the truncated horizontal stress profiles shown on
 388 the lower left panel of Fig. 4.

389 To appreciate the impact on the vertical shear stress, consider the statement of hori-
 390 zontal stress equilibrium (Eq. 2) expanded in the horizontal coordinate:

$$\frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \tau_{zx}}{\partial z} = 0 \quad (29)$$

391 Yielding implies that the horizontal gradient of the horizontal stress is zero, which
 392 by Eq. 29, implies the vertical gradient of the *horizontal* shear stress is zero, i.e:

$$\frac{\partial \sigma_{xx}}{\partial x} = 0 \implies \frac{\partial \tau_{zx}}{\partial z} = 0 \quad (30)$$

393 The boundary condition on the shear stress is assumed to be zero, and so the *hor-*
 394 *izontal* shear stress must be zero throughout the plastic regions; by symmetry so is the
 395 vertical shear stress. In the interior of the elastic core region, the vertical shear stress
 396 will remain parabolic, as long as the horizontal stress distribution is linear. In the yield-
 397 ing region, horizontal gradients in vertical shear stress (Q) will now depend on the
 398 rate at which the elastic core is narrowing, as well as gradient in the shear stress resul-
 399 tant $\frac{dV}{dx}$ (or the normal force). Solutions to this type of problem require non-linear ap-
 400 proaches (Turcotte et al., 1978). For the distribution of vertical shear stress shown in
 401 Fig. 4, in the limit $\Delta x \rightarrow 0$, $Q(z')$ takes the form of truncated parabolas, as shown in
 402 the lower right panel. In the limit of the elastic core becoming very thin, $\rho^*(z')$ becomes
 403 very large, acting like a point force concentrated at the plate center. For the remainder
 404 of the manuscript, an important observation is that that the center of mass of $Q(z')$, (or
 405 $\rho^*(z')$) does not change with progressive yielding. Note that if an elasto-plastic plate be-
 406 gins to unbend, vertical shear stress gradients (finite Q) may re-appear where previously
 407 they were constrained (by yielding) to be zero. This may be relevant, as models of plate
 408 bending often predict that the maximum bending moment occurs slightly seaward of the
 409 trench (Turcotte et al., 1978; D. Sandiford & Craig, 2023).

410 5 Estimating the trench pull force

411 The trench pull force represents the ΔGPE^* between the trench and a isostatic ref-
 412 erence column. When it comes to expressing the *difference* in GPE^* , based on Eq. 21,
 413 we can write:

$$\Delta \text{GPE}^* = -\Delta \bar{\sigma}_{zz} \quad (31)$$

$$= g \int_{z_0}^{z_c} (\Delta \rho(z) + \Delta \rho^*(z)) (z_c - z) dz \quad (32)$$

414 which says that the ΔGPE^* is an integral function of the difference in the real and psuedo
 415 densities. In this expression $\Delta \rho(z)$ means the density of the isostatic column minus the
 416 density of the deflected column; this assumes that $\rho(z)$ and $\rho^*(z)$ for both columns are
 417 defined on the same fixed vertical coordinate (z).

418 5.1 Further assumptions

419 To make use of the models in the previous section, we need to make a choice about
 420 the thickness of the lithosphere that supports the relevant stresses (i.e., the thickness to
 421 adopt for h). There are various lithospheric length scales that may be relevant, for in-
 422 stance, the thermal thickness (z'_t), the mechanical thickness (z'_m) and the effective elas-
 423 tic thickness (z'_e). In general, ($z'_t > z'_m > z'_e$) (McAdoo et al., 1978). See Table 1 for
 424 reference values. The assumption made in the remainder of this paper is that z'_m ($\approx 2z'_{np}$)
 425 is the relevant quantity. This assumption is fundamentally tied to how the distribution
 426 of vertical shear stress behaves during elasto-plastic yielding. In the monotonic bend-
 427 ing of simple uniform plates (Section 3), the distribution of ρ^* has a vertical center of
 428 mass given by the depth of the neutral-axis of the plate (z'_{np}). Crucially, this length scale
 429 does not change with the onset of yielding. In contrast, z'_e represents an effective quan-
 430 tity - the thickness of a uniform, non-yielding, elastic plate that would support a given
 431 bending moment and a given curvature (McNutt & Menard, 1982). For a elasto-plastic
 432 plate, the inferred value of z'_e will always be smaller as the curvature grows larger. The

433 is contrary to the way in which the spatial distribution of flexural stresses behaves dur-
 434 ing progressive bending.

435 Both observational and modelling constraints suggest the depth interval that con-
 436 tributes to the flexural characteristics of the lithosphere (z'_m) is substantially less than
 437 the thermal thickness (Chapple & Forsyth, 1979; Goetze & Evans, 1979). Deeper parts
 438 of the lithosphere are effectively irrelevant in terms of the bending moment, and likewise
 439 in controlling the depth of neutral plane. While z'_m is not necessarily directly observ-
 440 able, seismological observations show that the ‘apparent’ neutral plane depth of about
 441 30 - 35 km (Chapple & Forsyth, 1979; Craig et al., 2014). Moreover, there is broad agree-
 442 ment between seismological observations, the neutral-plane depth predicted from laboratory-
 443 derived strength envelopes, and the deflection characteristics under these types of strength
 444 models. This provides confidence that concept of z'_m ($\approx 2z'_{np}$) is meaningful and reason-
 445 ably well constrained (Chapple & Forsyth, 1979; Goetze & Evans, 1979; McNutt & Menard,
 446 1982; Craig et al., 2014; Garcia et al., 2019).

447 A further assumption we make is that ρ^* is zero in the isostatic column. Strictly
 448 speaking, isostatic equilibrium requires that only the vertical integral of ρ^* is zero. In
 449 principal, a non-zero vertical distribution of ρ^* may exist in the isostatic column, while
 450 still exhibiting zero resultant. This assumption means that in order to define $\Delta\rho^*$ we
 451 need only to know the vertical distribution of ρ^* in the deflected column.

452 5.2 Expression/estimate for the trench pull force

453 The ΔGPE^* equation (Eq. 32) is a function of the difference in the distributions
 454 of the real and pseudo density, between the isostatic reference column and the deflected
 455 column. Due to the assumptions we have introduced (such as neglecting the crust, and
 456 assuming ρ^* is zero in the isostatic column) we are left with only 2, non-overlapping con-
 457 tributions to the total (corrected) density differences. The first contribution (to $\Delta\rho(z)$)
 458 comes from the real density difference between rock and water. This difference occurs
 459 in the vertical section between the isostatic level (z_I) and deflected level ($z_I + w(x)$).
 460 The second contribution (given by $\Delta\rho^*(z)$) is due to the presence of vertical shear stress
 461 gradients in the deflected column. These are non-zero only between $z_I + w(x)$ and $z_I +$
 462 $w(x) + z'_m$ (i.e., across the mechanical thickness of the deflected plate).

463 The depth distribution for three different models of $\Delta\rho^*$ is shown in the left panel
 464 of Fig. 5. Two of these models are physically motivated, corresponding the elastic and
 465 elasto-plastic distributions of ρ^* (for an arbitrary degree of yielding). The third distri-
 466 bution, which represents constant ρ^* , is shown with the solid black line. The model of
 467 constant ρ^* is not not physically consistent, as it doesn’t satisfy the boundary conditions
 468 or the equilibrium equations. However, because each of the distributions have the same
 469 integrated value, and same center of mass, the contribution to the GPE^* is identical. The
 470 ΔGPE^* is the area under the $\Delta\sigma_{zz}$ curves. The model of constant $\rho^*(z)$ is useful, as it
 471 leads to a ΔGPE^* integral that can be calculated by inspection. This is represented by
 472 the area shown in the 2 grey triangles in the middle panel of Fig. 5. The magnitude of
 473 the trench pull force is therefore:

$$\Delta\text{GPE}^* = (\rho_m - \rho_w)gw_T \left(\frac{w_T + z'_m}{2} \right) \quad (33)$$

$$\approx (\rho_m - \rho_w)gw_T \left(\frac{z'_m}{2} \right) \quad (34)$$

$$\approx (\rho_m - \rho_w)gw_T z'_{np} \quad (35)$$

474 For the reference parameters (Table 1), the trench pull force is close to 2.5 TN m^{-1} .
 475 Based on the way we have set up the problem (as discussed in Section 2.2), this estimate

476 represents the ΔGPE^* of the lithosphere and water column. For a lithosphere-only force
 477 balance, the ΔGPE^* would increase by about 0.2 TN m^{-1} , and an additional (equal and
 478 opposite) term would appear, representing the effect of the hydrostatic pressure acting
 479 on the trench slope.

480 The red lines in Fig. 5 show the lithostatic approximation, where the distribution
 481 of Q (or ρ^*) is neglected in the deflected column. In this case the vertical normal stress
 482 does not equilibrate, and the ΔGPE^* does not converge with depth. The value is mean-
 483 ingless, as it does not represent the state of stress with depth.

484 As shown in Appendix B, another way to define the length scale that appears in
 485 Eq. 33 is as a difference in the center of mass of $\Delta\rho(z)$ versus $\Delta\rho^*(z)$, weighted by the
 486 total mass anomaly (M) due to the deflection:

$$\Delta\text{GPE}^* = Mg\Delta z_{\text{cm}} \quad (36)$$

$$= (\rho_m - \rho_w)gw_T\Delta z_{\text{cm}} \quad (37)$$

487 Because M , as well as the center of mass of $\Delta\rho$ are fixed, the GPE^* problem is com-
 488 pletely determined by the center of mass of the pseudo-density: The deeper this resides
 489 in the lithosphere, the greater Δz_{cm} , the larger the ΔGPE (for a given deflection).

490 6 Discussion

491 Several previous studies have discussed the existence of a pressure deficit due to
 492 downbending and a resulting driving force (Richter et al., 1977; Bird et al., 2008; Bercovici
 493 et al., 2015). To the best of my knowledge, this study represents the first attempt to con-
 494 strain the typical magnitude of the trench pull force via mechanical analysis of the bend-
 495 ing plate. It is notable that the estimated value is similar to that predicted for the ridge
 496 push force (i.e. $2.5\text{-}3.5 \text{ TN m}^{-1}$ for older lithosphere). The implication is that the topog-
 497 raphy associated with zones of divergence and convergence contributes similar amounts
 498 of net driving force the boundary layer (e.g, Hager and O’Connell (1981); Bercovici et
 499 al. (2015)).

500 The estimates presented in this study suggest that the total ΔGPE^* , between ridges
 501 and trenches, will typically be around 5 TN m^{-1} . Is this enough to drive the plates? As-
 502 suming shear stresses beneath the oceanic lithosphere are 1 MPa , the estimated ΔGPE^*
 503 is enough to balance the basal drag force on a plate of about 5000 km , a fairly typical
 504 length scale for Earth’s subducting plates. There are many studies that infer basal shear
 505 stress of significantly less than this, in the range of $0.2\text{-}0.5 \text{ MPa}$ (Lister, 1975; Melosh,
 506 1977; Richter et al., 1977; Wiens & Stein, 1985; Chen et al., 2021). On the other hand,
 507 trench and ridge systems do not sum perfectly constructively on Earth. For the Pacific
 508 plate in the Cenozoic, there is about 50% constructive contribution to the tangential
 509 component of the torque, based on trench geometry (D. Sandiford et al., 2024). For ide-
 510 alised plate geometries, however, the total ΔGPE^* is sufficient to balance a resisting basal
 511 drag, within the uncertainties associated with the latter.

512 In developing a model for the depth distribution of the relevant stress quantities
 513 (i.e. $Q(z)$, or $\rho^*(z)$) various assumptions and simplifications have been made. For in-
 514 stance, we have adopted the standard ‘thin plate’ assumptions, including a shear stress-
 515 free basal boundary, and neglecting plate rotation due to deflection. Likewise, the anal-
 516 ysis has assumed uniform constitutive properties. These choices all preserve complete
 517 symmetry in the resulting stress distributions (e.g., Fig. 4). Some of these assumptions
 518 could be removed with a more sophisticated analytic treatment. Comparison with nu-
 519 merical models is also informative.

520 D. Sandiford and Craig (2023) analysed the force balance in a 2D finite element
 521 subduction model, comprising a 5000 km subducting plate, where flow was completely
 522 driven by the imposed density structure (D. Sandiford & Craig, 2023). Analysis showed
 523 that F_D was weakly positive (extensional) at the trench, (0.6 TN m^{-1}) and that the ΔGPE^*
 524 - defined relative to the isostatic lithosphere - was about 2.0 TN m^{-1} . Therefore, trench
 525 pull represented the dominant ($\approx 75 \%$) component of the net-slab pull. The ΔGPE^*
 526 reported in D. Sandiford and Craig (2023) is also about 75 % of the ‘typical’ value de-
 527 rived in this study. This suggests that, although based on numerous simplifying assump-
 528 tions, the expression developed in this study is applicable to the dynamics of more re-
 529 alistic, and complex, bending scenarios.

530 Bessat et al. (2020) also estimate the ‘GPE’ variation around the trench, based on
 531 retro-analysis of a numerical subduction model. They use the lithostatic stress to ap-
 532 proximate the vertical normal stress (hence theirs is an estimate of GPE, not GPE^*).
 533 The GPE variation due to the trench topography was estimated to be $> 50 \text{ TN m}^{-1}$. The
 534 very large ($\sim 20\times$) discrepancy between this value, versus the current study, is attributable
 535 to two factors in their analysis: 1) the lithostatic approximation was used for σ_{zz} ; and
 536 2) the vertical integration was extended to the base of the model domain (660 km). Note
 537 that, as shown in Fig.5, the lithostatic approximation means the $\Delta\sigma_{zz}$ does not equi-
 538 librate with depth. As a result, the ΔGPE^* does not converge. Hence, factor 2 is likely
 539 to vastly exacerbate factor 1. It is speculated that, had the true vertical normal stress
 540 from the numerical model been used, values compatible with Eq. 33 would have been
 541 obtained.

542 It should be relatively straightforward to test generality of these ideas by others
 543 in the subduction modelling community. As far as 2D numerical subduction models is
 544 concerned, the simplest way to approach an analysis of the trench pull (and its relative
 545 role in the net slab pull) is to calculate the variation of F_D seaward the trench. Func-
 546 tionally, this requires approximating the integral of the deviatoric stress difference down
 547 to a fixed compensation level (i.e., the first term in Eq. 7). The change in F_D between
 548 the trench and the point where the plate returns to the isostatic level ($\sim 100 \text{ km}$) is a
 549 proxy for the ΔGPE^* (the trench pull). This assumes that the basal shear force (F_B) is
 550 insignificant over the same horizontal lengthscale. It then remains to assess how the value
 551 of ΔF_D compares to F_D evaluated at the trench. If F_D at the trench is positive, and larger
 552 than ΔF_D , then it might be reasonable to assume the net slab pull dominated by an in-
 553 plane resultant transmitted through the slab hinge (i.e., the textbook mode of slab pull).
 554 However, if the ΔF_D is the larger term, then trench pull provides the dominate compo-
 555 nent of the net slab pull. Note that the dynamics should be very similar even if the sur-
 556 face boundary condition has a ‘free slip’ type condition. In this case, although there is
 557 no deflection, a (roughly equivalent) pressure anomaly will be present; it is through this,
 558 that the trench pull force will arise.

559 In this study, analysis of the trench pull force is developed in terms of the relative
 560 trench depth. This is a valid way of framing the problem, as the deflection can be viewed
 561 as the ‘cause’ of the pressure deficit (e.g., Richter et al. (1977)). However, downbend-
 562 ing of the subducting plate is typically linked to the presence of a vertical shear stress
 563 and/or bending moment acting on a plane beneath the trench. This means that we could
 564 also develop a relationship between the trench pull force, and some combination of these
 565 loading patterns. Davies (1983) proposed that the net slab pull is approximately equal
 566 to the vertical shear stress resultant at the trench (V_0). If we adopt the same assump-
 567 tions as that study, a uniform elastic plate, and neglecting the bending moment, then
 568 V_0 is a linear function of the trench deflection (e.g., Turcotte and Schubert (2002)):

$$V_0 = g(\rho_m - \rho_w)w_T \frac{\alpha}{2} \quad (38)$$

569 where α is the flexural parameter. Comparison with Eq. 34 shows that V_0 will be equal
 570 to the trench pull (ΔGPE^*), provided that $\alpha \sim z'_m$. Previous investigations suggest
 571 these length scales are indeed comparable (e.g., Caldwell et al. (1976); Goetze and Evans
 572 (1979); Hunter and Watts (2016)). Hence, the current study provides support for the re-
 573 lationship proposed by Davies (1983): in simple terms the vertical force due to the slab
 574 ‘pulling down’ on the trailing plate may be roughly equivalent to the horizontal force in-
 575 duced by the resulting pressure deficit (i.e., the trench pull force). However, additional
 576 coupling will occur due to the presence of a bending moment.

577 7 Conclusions

578 The purpose of this paper has been to investigate the mechanics and typical mag-
 579 nitude of the trench pull force. The description of a net horizontal force due to gravi-
 580 tational potential energy differences (ΔGPE^*) is extended to include the effect of a pseudo-
 581 density, $\rho^*(z)$, which supports the non-isostatic topography. Elastic and elasto-plastic
 582 models are used to investigate this problem, specifically the distribution of $Q(z)$ (or $\rho^*(z)$).
 583 A key conclusion is that the length scale over which trench pressure deficit acts is $\frac{z'_m}{2} \approx$
 584 z'_{np} . It is shown that this length scale represents the difference in the center of mass of
 585 the real density difference ($\Delta\rho$), versus the pseudo density difference ($\Delta\rho^*$). The result-
 586 ing estimate for a typical trench pull force is about 2.5 TN m^{-1} , similar to that associ-
 587 ated with isostatic cooling of old lithosphere. The topography that exists between ridges
 588 and the trenches ($\approx 5\text{-}6 \text{ km}$) is likely to be associated with a net force of at least 5 TN m^{-1} ,
 589 enough to balance basal drag of 1 MPa , over a plate length of 5000 km . Comparison be-
 590 tween the expression developed in this study, and results based on retro-analysis of nu-
 591 merical model, agree to about 75 %. Others in the subduction modelling community are
 592 encouraged to test the generality of these relationships, which are relatively easy to as-
 593 certain.

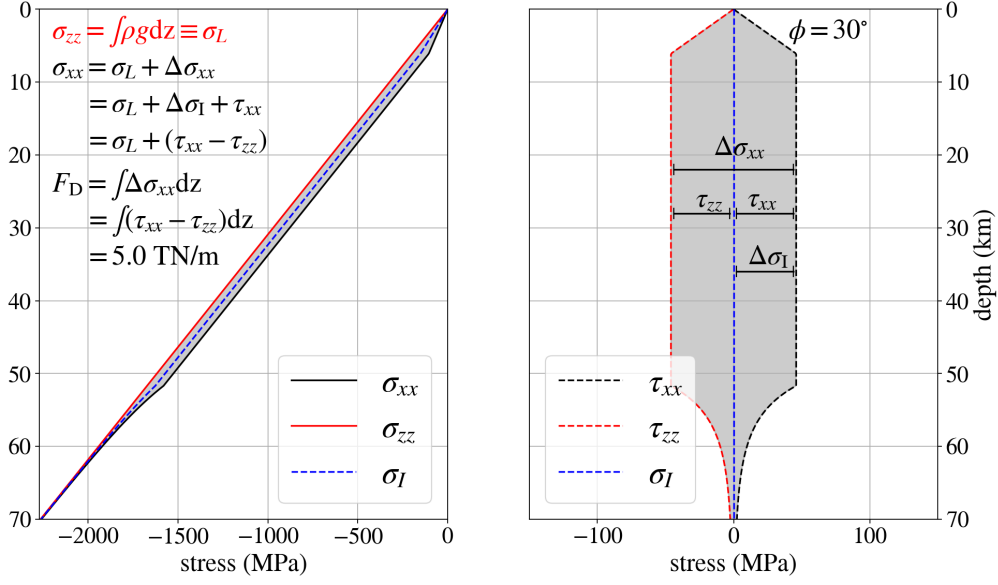


Figure 1. Horizontal and vertical stresses on a hypothetical section across the lithosphere. Compressive stresses are negative. The vertical normal stress (σ_{zz}) is a principal stress, and is assumed to be lithostatic (σ_L). The horizontal normal stress is given by the lithostatic stress, and an additional (differential) stress, symbolised $\Delta\sigma_{xx}$ (e.g., Turcotte and Schubert (2002)). The integral of the differential stress is referred to as the ‘in-plane resultant’, symbolised F_D . Positive F_D means the horizontal stress is (on average) less compressive than the vertical, i.e. an Andersonian extensional regime, or deviatoric tension. The magnitude of F_D shown in the figure is 5 TN m^{-1} . In plane strain, the differential stress can be written in several equivalent ways; $\Delta\sigma_{xx} = (\tau_{xx} - \tau_{zz})$, is an important relationship that will appear in later analysis. The strength model includes frictional behaviour as well as power law creep (Hirth & Kohlstedt, 2003).

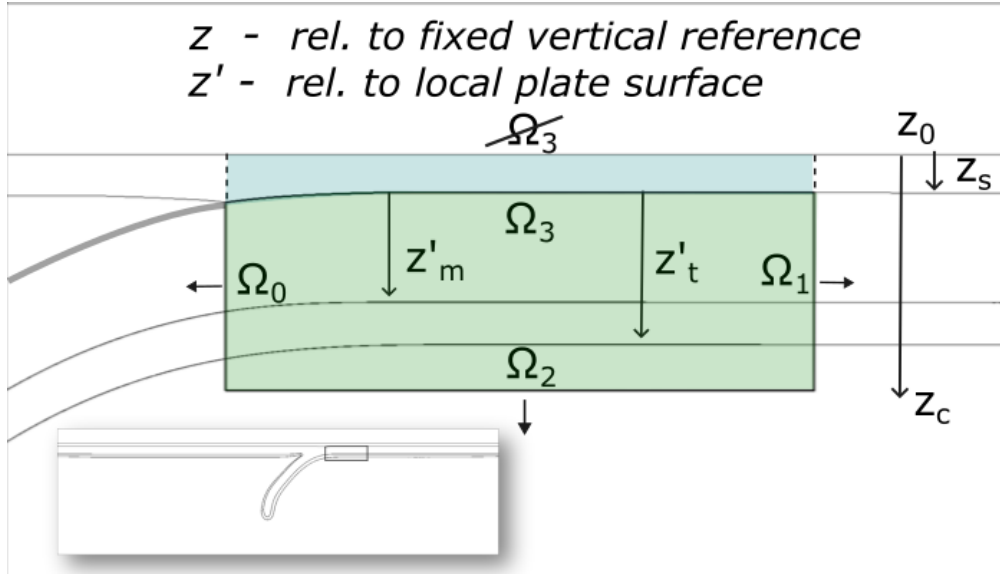


Figure 2. Schematic of a subducting plate, and a subdomain on which the ‘vertically integrated’ horizontal force balance equations are developed. The green domain represents rock, the blue domain the water column. To simplify the analysis, we combine the domains, so that Ω_3 moves to the sea surface (at z_0), but makes no contribution to the horizontal force balance. The vertical boundaries ($\Omega_{0,1}$) extend from z_0 to z_c . z'_m represents the mechanical thickness of the lithosphere, typically significantly less than the thermal thickness z'_t . The compensation level is represented by z_c . At this level, vertical normal stresses are approximately equal (exactly equal in the hydrostatic approximation). Here, z_c is taken as a fixed ($z = \text{constant}$) depth equivalent to the lithospheric thermal thickness, i.e., $z_c = z_I + z'_t$. In fact however, based on the models developed in this study, vertical normal stresses always equilibrate at (or above) the base of the mechanical lithosphere. This makes the choice of z_c irrelevant as long as it equal or greater than $z_I + z'_m$. In other words, beneath the depth $z_I + z'_m$, there is no contribution to the net horizontal force arising from the vertical integrals.

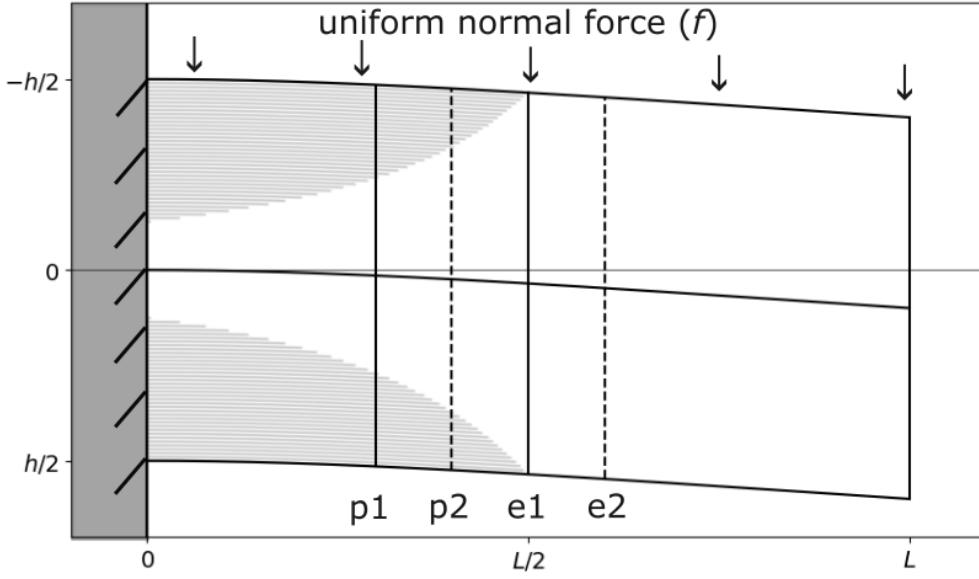


Figure 3. Deflection of a cantilever subject to uniform normal force. The dimensional deflection is: $w(x) = \frac{fx^2}{24EI} (6L^2 - 4Lx + x^2)$, where f is the normal force. In this figure, f and L are taken as 1, the aspect ratio is 2, and E is chosen to provide a dimensionless deflection $w' = \frac{w}{L}$ of 5%. The general behavior can be represented by scaling stresses by the maximum values: for the horizontal normal stress, $\sigma_{xx}^{Max} = \frac{6fL^2}{h^2}$, and for the shear stresses, $\tau_{xz}^{Max} = \frac{3fL}{2h}$. This is how the stresses along profiles (p1, p2, e1, e2) are represented in Fig. 4. The light grey region shows the zone where yielding is assumed, with the yield limit given by $\tau_{Max} \leq \frac{1}{2}\sigma_{xx}^{Max}$.

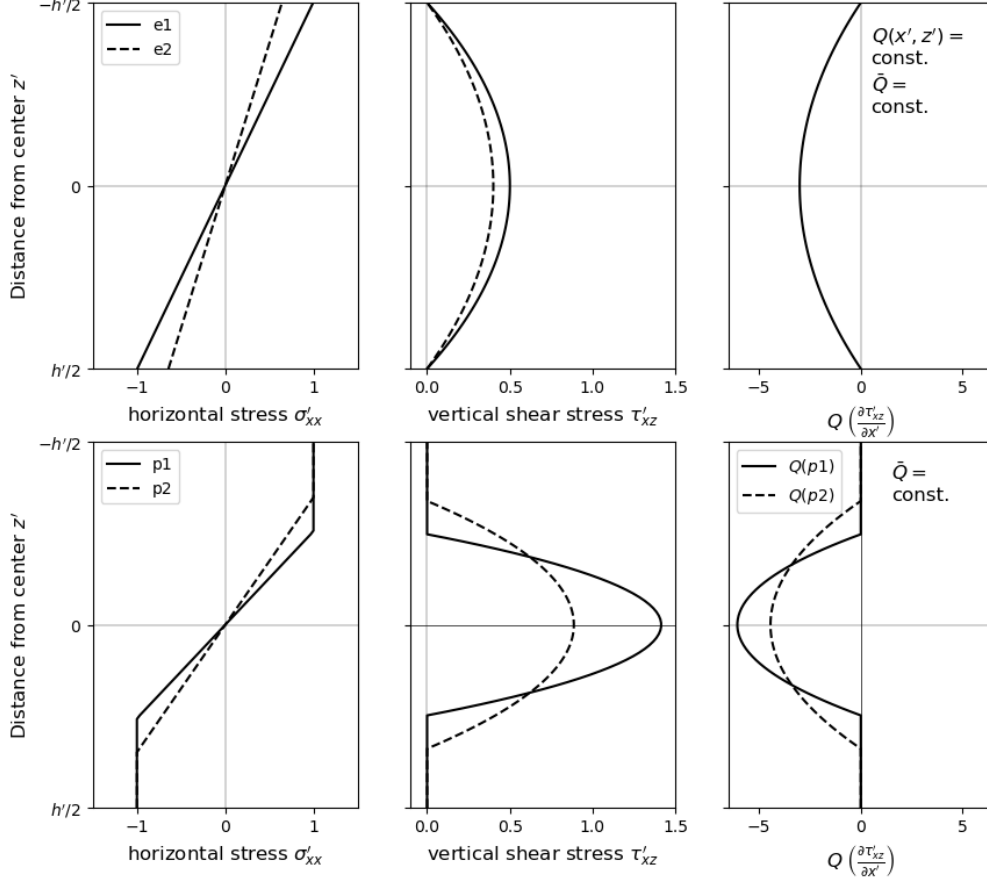


Figure 4. Distribution of stresses in elastic (upper panels), and elasto-plastic (lower panels) domains, after Horne (1951). Normal stresses are scaled using the prescribed value of the yield stress $\sigma_y = \frac{1}{4}\sigma_{xx}^{Max}$; shear stresses are scaled using $0.5\tau_{xz}^{Max}$, as discussed in the Fig. 3 caption. Lengths have been scaled using L . In terms of how the vertical distribution of stress impacts the GPE*, the key quantity is the one shown in the right hand panels - the horizontal gradient of the vertical shear stress. In this paper, we symbolise these gradients Q ; we also define $\rho^* = \frac{Q}{g}$. This setup assumes uniform loading, and hence the vertical shear stress resultant ($\bar{Q} \equiv \frac{dV}{dx}$) is constant. In the elastic domain, $Q(x', z')$ is constant everywhere, as shown in the top right hand panel. In the yielding case, $Q(x', z')$ may vary, but the resultant (\bar{Q}) remains constant. For the yielding domain, $Q(z) = 0$ in the outer yielding region, and remains parabolic in the inner elastic core region.

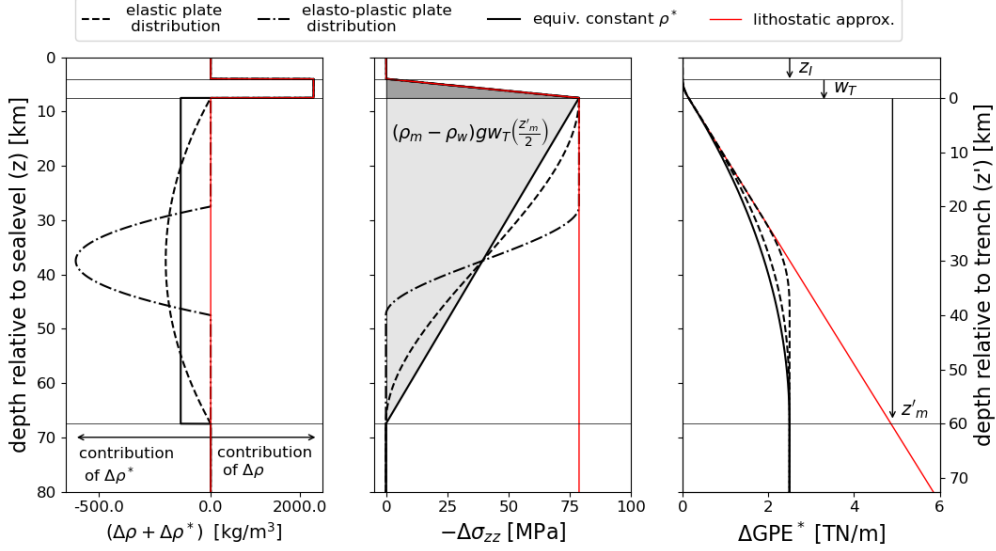


Figure 5. Each panel represents the differences in quantities between the isostatic reference column and a column beneath the trench. The figure uses reference values shown in Table 1. The left hand panel shows the difference in the corrected density between the columns. This represents, the sum of, respectively, the difference in the real density ($\Delta\rho$) and the pseudo density ($\Delta\rho^*$). The vertical integral of this quantity must be zero in order for equilibration of the vertical normal stress to occur (from Eq. 13). The first moment of this quantity, around the compensation level, gives the ΔGPE^* (from Eq. 32). The middle panel shows the (negative of) the difference in vertical normal stress. The area under the $-\Delta\sigma_{zz}$ curves is also equal to the ΔGPE^* (e.g., Eq. 31). The expression shown in the middle panel, represents the area of the light gray triangle. This is the simplified expression for the trench pull force (e.g. Eq. 34). The concept of a compensation level implies that differences in $\Delta\sigma_{zz}$ have to equilibrate exactly. When the lithostatic approximation is used for stress state beneath the trench (shown in red), there is no equilibration of the vertical normal stress. The right hand panel shows the cumulative ΔGPE^* , as a function of depth. All of the black lines converge to the same value, because they are, respectively based on density distributions ($\rho(z)$, $\rho^*(z)$), which have the same vertical center of mass. In the lithostatic approximation, the ΔGPE^* is unbounded.

Appendix A Distribution of vertical shear stress

In deriving the vertical distribution of shear stress, the assumptions are a uniform 2D plate of thickness h , which undergoes plane bending, with zero shear stress on the upper and lower edges. We retain the same coordinate convention (positive down, to the right); we use the primed coordinate system, which is relative to the plate. here, the origin of z' is the center of the plate. Neglecting any in-plane stress resultant, the balance of moments and vertical forces, for a 2D beam/plate equation are expressed as:

$$\frac{dM}{dx} = V(x), \quad \frac{dV}{dx} = -f(x) \quad (\text{A1})$$

The normal stress $\sigma_{xx}(z)$ due to bending is:

$$\sigma_{xx}(x, z') = -\frac{M(x) \cdot z'}{I} \quad (\text{A2})$$

where I is the (2D) moment of the area. From Eq. A1, the horizontal gradient of normal stress is:

$$\frac{\partial \sigma_{xx}(x, z')}{\partial x} = -\frac{z' \cdot V(x)}{I} \quad (\text{A3})$$

For the horizontal direction, the stress equilibrium equation is:

$$\frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{zx}}{\partial z'} = 0 \quad (\text{A4})$$

meaning:

$$\frac{\partial \tau_{xz}}{\partial z} = \frac{z' \cdot V(x)}{I} \quad (\text{A5})$$

Integrating with respect to z' :

$$\tau_{xz}(x, z') = \int \frac{z' \cdot V(x)}{I} dz = \frac{V(x)}{I} \left(\frac{z'^2}{2} \right) + C(x) \quad (\text{A6})$$

Given $\tau_{xz}(x, \pm \frac{h}{2}) = 0$, we can solve for $C(x)$. Substituting $C(x)$ back, we get:

$$\tau_{xz}(x, z') = \frac{V(x)}{I} \left(\frac{z^2}{2} - \frac{h^2}{8} \right) \quad (\text{A7})$$

There are few brief points to note. The maximum value of the vertical shear stress occurs at the center of the plate (or more generally, at the neutral plane), where the horizontal stress is zero. Across the plate, the principal stresses rotate: they are only truly vertically aligned (Andersonian) at the free surface. At the center of the plate, the principal stresses are oriented at 45° from the horizontal: the differential stress is not zero at the middle of the plate, although the quantity σ_{xx} is. For the lithosphere, where gravitational forces obviously contribute to the mean stress, it would be τ_{xz} , or $\Delta\sigma_{xx}$ that are zero at the neutral plane.

616 **Appendix B ΔGPE^* as a difference in center of mass**

617 In the manuscript, the ΔGPE^* between an isostatic reference column, and a de-
618 flected column, is given by:

$$\Delta\text{GPE}^* = g \int_{z_0}^{z_c} (\Delta\rho(z) + \Delta\rho^*(z)) (z_c - z) dz \quad (\text{B1})$$

619 From Eq. 15, we know that total integral (mass anomaly) of each density distribution
620 is equal:

$$M = \int_{z_0}^{z_c} \Delta\rho(z) dz = \int_{z_0}^{z_c} \Delta\rho^*(z) dz = (\rho_m - \rho_w)w(x) \quad (\text{B2})$$

621 where $w(x)$ is the deflection. The difference in the center of mass of each of these dis-
622 tributions (around z_c) can be written as:

$$\begin{aligned} \Delta z_{\text{cm}} &= \frac{1}{M} \int_{z_0}^{z_c} (\Delta\rho(z)(z_c - z)) dz \\ &\quad - \\ &\quad - \frac{1}{M} \int_{z_0}^{z_c} (\Delta\rho^*(z)(z_c - z)) dz \end{aligned} \quad (\text{B3})$$

623 the negative sign on the last line reflects the fact that $\Delta\rho^*(z)$ is a negative quantity, and
624 we wish to define a positive center of mass. Which means we can write Eq. B1 as:

$$\Delta\text{GPE}^* = gM\Delta z_{\text{cm}} \quad (\text{B4})$$

625 The center of mass of $\Delta\rho(z)$ is given by $z_I - \frac{1}{2}w_T$. Based on the models and as-
626 sumptions developed in this paper, the center of mass of $\rho^*(z)$ occurs at $z_I + w + \frac{1}{2}z'_m$.
627 The difference is $\frac{1}{2}(w + z'_m)$, as in Eq. 33.

628 This relationship also allows us to examine the approximation we used in neglect-
629 ing the crust. Because we neglected the crust, and instead treated the entire column of
630 lithosphere as having background mantle density, we introduced an error in the distri-
631 bution of $\Delta\rho$. We overestimated the $\Delta\rho$ in the section of lithosphere between z_I and w_T ,
632 because we took the density difference as $\rho_m - \rho_m$, whereas the actual density differ-
633 ence is $\rho_m - \rho_c$ (assuming the mocho depth (z'_m) is greater than w_T , which is usually cor-
634 rect). This overestimate is balanced by an equal underestimate between the depths $z_I +$
635 z'_m and $z_I + z'_m + w_T$, where the isostatic column contains mantle rock and the deflected
636 column contains crust. The error in the GPE^* can be estimated from Eq. B4, and is \approx
637 0.04 TN m^{-1} .

638 **Open Research Section**

639 This manuscript does not contain any new data.

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