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We welcome helpful feedback.

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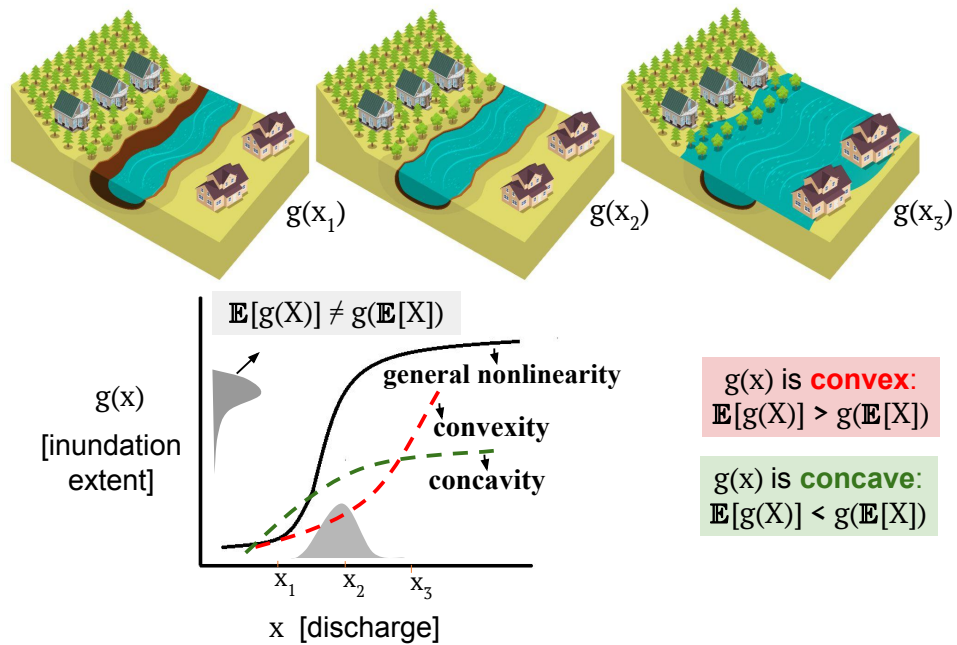
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Graphical Abstract

Flooding, nonlinear dynamics and Jensen's inequality: Analyzing the damping and amplification of inundation extent with river discharge nonstationarity

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Highlights

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- The relationship between inundation extent and river discharge is nonlinear.
- We introduce Jensen's Inundation Factor (JIF) to characterize this nonlinearity.
- We discuss the damping and amplification in inundation with shifts in discharge.
- JIF provides insights on critical thresholds related to flood inundation risk.

Flooding, nonlinear dynamics and Jensen's inequality: Analyzing the damping and amplification of inundation extent with river discharge nonstationarity

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Abstract

Nonlinear relationships between river discharge and flood inundation complicate effective flood risk assessments. In this study, we characterize the behavior of these nonlinearities. We explore the nature of the expected shifts in mean and variance of inundation due to various kinds of river discharge nonstationarities. Viewing flood inundations through the lens of Jensen's inequality, we show that the shifts in flood discharge do not result in proportionate shifts in inundation extent. We introduce a Jensen's Inundation Factor (JIF), which is an aggregate index dependent on the river-reach nonlinearity and the parameters of the discharge distribution. We highlight the implications of Jensen's inequality by running an operational NOAA OWP HAND flood inundation model across six catchments in the United States. Our results confirm a variety of nonlinear relationships across all basins, with critical discharge thresholds - providing insights that allow for more reliable flood risk estimation. We use these examples as a basis to highlight the need to understand river-reach level nonlinearities for evaluating climate nonstationarities - as global shifts in rainfall will not translate to proportionate shifts in inundation extent.

Keywords: Flooding, inundation extent, Jensen's inequality, nonlinearity, nonstationarity

1. Introduction

Flooding remains a formidable natural disaster, disrupting hydro-ecological systems and causing substantial socio-economic losses[1, 2]. In recent years, there has been a noticeable increase in the frequency and severity of fluvial, pluvial, and compound flooding events in many parts of the world[3, 4, 5, 6]. This surge in flooding events is characterized by the rise in floodwater levels that breach levees and inundate floodplains, causing flooding and damage in areas that were previously safe[7]. At the heart of this growing crisis is a confluence of factors: shifting climate, changing land use patterns, and rapid population growth[8, 9]. These factors exacerbate the impact of extreme weather events by altering the local hydrological cycle, leading to increased river discharge and heightened flood risks[7, 10]. Thus, gaining insight into the extent and patterns of the escalation in current flood levels is imperative for formulating resilient strategies in areas prone to vulnerability.

Traditionally, flood models have relied on stationary assumptions about input precipitation distributions, where past hydrological patterns/return period events are used to predict future flood risks[11]. However, this approach is becoming increasingly unreliable in the face of nonstationarity - the idea that river discharge patterns are evolving due to climate variability, land use changes, and human interventions [12, 13, 14]. Under nonstationarity, small changes in river discharge can result in various responses in inundation distributions, leading to amplification or damping of shifts [12]. Non-stationary conditions complicate flood predictions and challenge the effectiveness of current flood hazard maps because meaningfully assigning exceedance probabilities to various events becomes challenging[13]. Notably, in recent years, significant portions of flood insurance claims have originated outside the confines of regulatory flood hazard boundaries, drawing attention to the limitations of existing flood hazard maps. These maps have faced widespread criticism for presenting flood hazards as a binary process—within or outside inundation probability—while neglecting the inherent uncertainties in model estimates[15].

The present study aims to bridge this gap in understanding the combined effect of flood inundation nonlinearity and nonstationarity in river discharge. Armed with the mathematical concept of Jensen’s inequality[16], this study explores the relationships that characterize this nonlinear behavior. Jensen’s inequality, a fundamental principle in probability theory[17], provides insight into how the average behavior of a dependent random variable is in relation to the behavior induced by the average independent variable. The dependent random variable, in this case, is the flood inundation, while the stream flow is the independent random variable. We use systematic shifts in the streamflow distribution to see its influence on flood inundations. Through a suite of carefully designed simulation experiments, this investigation seeks to decipher the factors governing flood inundation expansion and intensity.

39 In particular, this study introduces the concept of the Jensen Inundation Factor (JIF),
 40 a numerical parameter that quantifies the damping and amplification in the flood inun-
 41 dation shifts relative to the shifts in the streamflow. By investigating how shifts in the
 42 mean of the discharge distribution impact inundation behavior using the National Oceanic
 43 and Atmospheric Administration - Office of Water Prediction (NOAA-OWP) operational
 44 Height Above the Nearest Drainage (HAND) based flood inundation model (FIM), the
 45 study offers a methodology with the potential to significantly enhance the precision and
 46 reliability of flood forecasting. This approach promises not only practical benefits for
 47 flood management but also contributes to the broader scientific discourse surrounding
 48 flood modeling, adaptation, and the intricate relationship between river discharge and
 49 inundation extent. Understanding these dynamics will help improve flood risk assess-
 50 ments, particularly in the face of increasingly unpredictable hydrological patterns driven
 51 by climate change.

52 2. Methods and material

53 2.1. Conceptual overview

54 2.1.1. Nonlinear transformations and Jensen's inequality

55 Nonlinear transformations are crucial for understanding how random variables are af-
 56 fected by nonlinear functions. Consider a random variable X representing river discharge.
 57 When this variable is transformed by a nonlinear function $f(x)$, the expected value of
 58 $f(X)$ is generally not equal to $f(\mathbb{E}[X])$. This discrepancy arises because nonlinear func-
 59 tions alter the distribution of X in ways that can either amplify or dampen the effect of
 changes in X on the transformed variable.

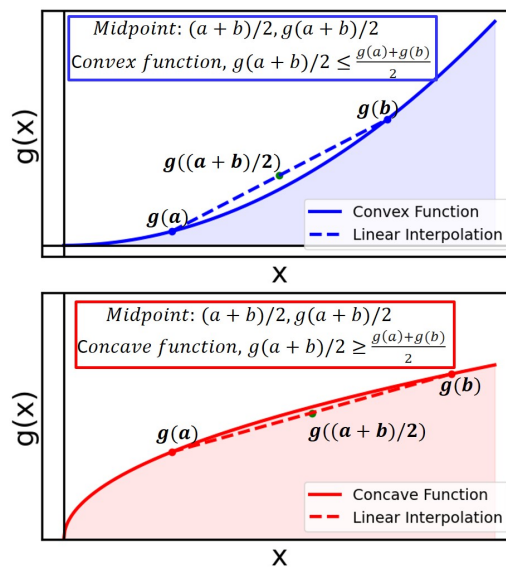


Figure 1: Convex and concave behavior of functions.

61 A function $g(x)$ is defined as convex if, for any two points a and b , the function value
 62 at the average of a and b is less than or equal to the average of the function values at a
 63 and b [18] (Figure 1) :

$$g\left(\frac{a+b}{2}\right) \leq \frac{g(a)+g(b)}{2} \quad (1)$$

64 Conversely, a function $g(x)$ is concave if the function value at the average of a and b is
 65 greater than or equal to the average of the function values at a and b (Figure 1) :

$$g\left(\frac{a+b}{2}\right) \geq \frac{g(a)+g(b)}{2} \quad (2)$$

66 Jensen's inequality relates to these definitions by stating that for a convex function
 67 $g(x)$ and a random variable X , the expected value of the function is greater than or equal
 68 to the function evaluated at the expected value of X [17]:

$$\mathbb{E}[g(X)] \geq g(\mathbb{E}[X]) \quad (3)$$

69 For concave functions $g(x)$, the inequality reverses:

$$\mathbb{E}[g(X)] \leq g(\mathbb{E}[X]) \quad (4)$$

70 2.1.2. Jensen's Inundation Factor - damping and amplification

71 Here we introduce Jensen Inundation Factor (JIF) to quantify the nonlinear rela-
 72 tionship between inundation and river discharge. To account for non-stationarity in
 73 streamflow, USGS gauge discharge data is fitted with a log-normal distribution and then
 74 adjusted with systematic multipliers. The JIF is calculated as follows:

$$\text{JIF} = \frac{\mathbb{E}[g(X)]}{g(\mathbb{E}[X])} \quad (5)$$

75 where $g(\mathbb{E}[X])$ is the value of the function g evaluated at the expected value of X and
 76 $\mathbb{E}[g(X)]$ is the expected value of the function $g(X)$ when applied to X .

77 The curve in Figure 2 represents the nonlinear relationship between river discharge and
 78 flood inundation extent. At lower discharge (within bankfull), the relationship appears
 79 relatively linear, with a gradual increase in inundation. As discharge approaches and
 80 exceeds bankfull, the curve bends more sharply, reflecting a nonlinear response where
 81 inundation increases more dramatically. In reality, the shape of the curve varies, and
 82 for different topographies, it bends with different shapes and slopes. Convex sections
 83 (red dashed line) indicate a rapid increase in inundation, while concave sections (green
 84 dashed line) show a slower rise in inundation extent. The curve highlights how different
 85 landscapes and discharge scenarios influence flood risk. This concept aligns with Jensen's
 86 Inundation Factor, which quantifies the nonlinear relationship between inundation and

87 streamflow.

88 Various values of JIF, which signify damping and amplification, are used to indicate
 89 the disproportionality of the change in inundation with respect to the change in stream-
 90 flow. Specifically, damping occurs when the shift in inundation is less than the relative
 91 shifts in streamflow, indicating a lower inundation response to discharge shifts. This
 92 damping phenomenon is a result of a sublinear response to the shifts in streamflow. In
 93 contrast, amplification reflects that the shifts in inundation is higher relative to the shift
 94 in the streamflow, signifying a more pronounced flood response. The amplification phe-
 95 nomenon is a result of the superlinear response to the shifts in streamflow. For sublinear
 response, JIF is less than 1 and for superlinear response JIF is greater than 1.

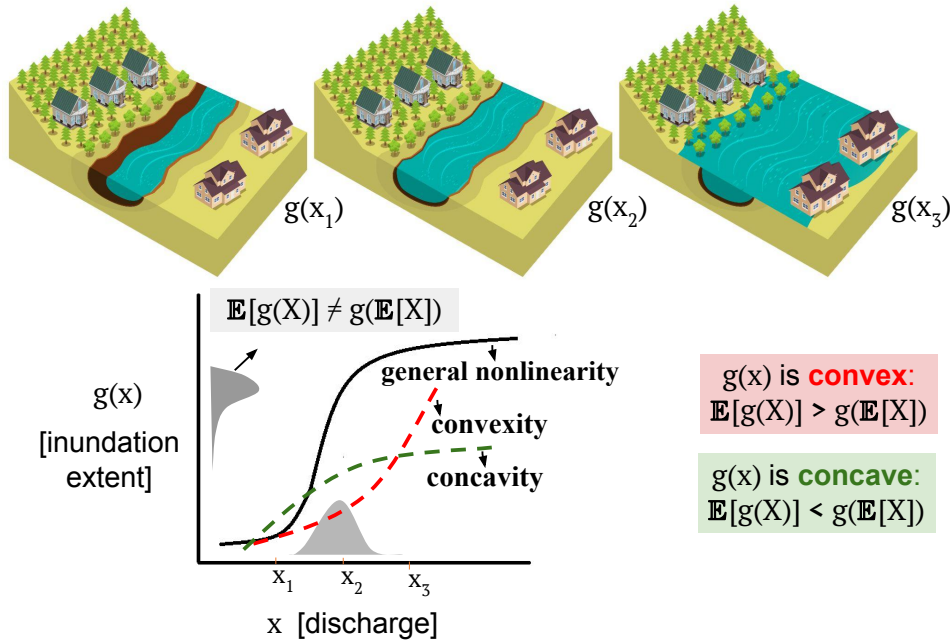


Figure 2: Conceptualization of Jensen's inequality for different discharge-inundation nonlinearities

96

97 2.1.3. Proof of concept - analytical derivation

98 The simplest way to express the discharge-inundation nonlinearity in fluvial flooding
 99 is by using Manning's equation. It is used to estimate the discharge at a given time
 100 based on the hydraulic geometry and river slope. Although it is for uniform flow, it is
 101 representative of the nonlinear relationship between the stream flow and flood inundation.

102 The Manning's-Stickler equation in open channel flow is expressed as:

$$Q = A \cdot \left(\frac{1}{n} R^{2/3} S^{1/2} \right) \quad (6)$$

103 where $R = \frac{A}{P}$. P is the wetted perimeter. Therefore:

$$Q = k \cdot A^{5/3} P^{2/3} \quad (7)$$

104 For a triangular section, $A = \frac{Ih}{2}$, where I is the top width of the section and h is the
 105 flow depth from the bottom.

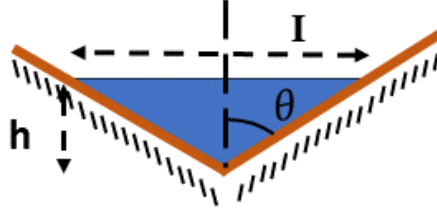


Figure 3: Simple triangular section used for geometric illustration in the derivation of Manning's equation

106 From simple geometric considerations for a triangular channel (Figure 3) :

$$h = I \tan\left(\frac{\theta}{2}\right) \quad (8)$$

$$107 \quad A = \frac{I^2 \tan^2\left(\frac{\theta}{2}\right)}{2} \quad (9)$$

$$108 \quad P = I \sin(\theta) \quad (10)$$

109 Substituting A and P into the equation for Q :

$$Q = k \cdot (\sin(\theta))^{2/3} \cdot \left(\frac{\tan^2\left(\frac{\theta}{2}\right)}{2}\right)^{5/3} \cdot \left(I \cdot \frac{\sin(\theta)}{\tan\left(\frac{\theta}{2}\right)}\right)^{2/3} \quad (11)$$

110 Simplifying, we get:

$$I^{8/3} = \frac{Q}{K'} \quad (12)$$

$$111 \quad I = K \cdot Q^{3/8} \quad (13)$$

112 Let X be a random variable representing discharge, following a uniform distribution
 113 between 0 and b , where b is the highest flow :

$$X \sim U_{[0,b]} \quad (14)$$

114 The probability density function is:

$$p_X(x) = \frac{1}{b} \quad (15)$$

115 Now, let $Y = g(X)$. The nonlinear transform of Y is:

$$p_Y(y) = p_X(x) \cdot \left| \frac{d(g^{-1}(y))}{dy} \right| \quad (16)$$

116 Substituting:

$$g^{-1}(y) = \left(\frac{y}{c}\right)^{\frac{3}{8}} \quad (17)$$

$$\frac{d\left(\frac{y}{c}\right)^{\frac{3}{8}}}{dy} = \frac{8}{3} \left(\frac{y}{c}\right)^{\frac{5}{8}} \quad (18)$$

117 From equations (15) and (16), we get:

$$p_Y(y) = \frac{1}{b} \cdot \frac{8}{3} \cdot \left(\frac{y}{c}\right)^{\frac{5}{8}} \quad (19)$$

118 Now, calculating the expectation:

$$\mathbb{E}[X] = \int_0^b x \cdot p_X(x) dx = \int_0^b \frac{x}{b} dx = \frac{b}{2} \quad (20)$$

119 Now:

$$\mathbb{E}[g(X)] = \mathbb{E}[Y] = \int_0^{cb^{\frac{3}{8}}} y \cdot p_Y(y) dy \quad (21)$$

120 Further integrating, we get:

$$\mathbb{E}[g(X)] = \frac{8}{11} \cdot \frac{c}{b^{\frac{3}{8}}} \quad (22)$$

121 Using equation (13), which represents the nonlinearity in a triangular channel:

$$g(\mathbb{E}[X]) = c \left(\frac{b}{2}\right)^{\frac{3}{8}} = \frac{c}{2^{\frac{3}{8}}} \cdot b^{\frac{3}{8}} \quad (23)$$

122 Dividing equations (22) and (23):

$$\frac{\mathbb{E}[g(X)]}{g(\mathbb{E}[X])} = \frac{\frac{8}{11} \cdot c \cdot b^{\frac{3}{8}}}{\frac{c}{2^{\frac{3}{8}}} \cdot b^{\frac{3}{8}}} = 0.943 \quad (24)$$

123 This is one of the simplest analytical demonstrations of Jensen's inequality for flood
124 inundation with increasing discharge. The $JIF < 1$ represents the damping effect in inun-
125 dation shifts as the discharge distribution shifts towards the right. As the water level rises,
126 the velocity in the channel also increases, allowing more water to pass through the same
127 cross-sectional area. As a result, the inundation extent does not increase proportionately
128 to the increase in discharge.

129 2.1.4. Climatic and topographical influence on Jensen's Inundation Factor

130 Let X be the input random variable with probability distribution function $p_X(x | \theta)$,
131 where θ is the set of parameters determining its location, shape, and scale.

132 Given $g(x, \Psi)$ is the non-linearity, with parameters Ψ , the expectation is given by:

$$\mathbb{E}[g(X, \Psi)] = \int_{-\infty}^{\infty} g(x, \Psi) p_X(x | \theta) dx = f_1(\Psi, \theta) \quad (25)$$

133 where $f : \mathbb{R}^n \rightarrow \mathbb{R}$ represents any general functional relationship from the n -dimensional
134 parameter space to real line.

135 Similarly:

$$g(\mathbb{E}[X]) = f_2(\Psi, \theta) \quad (26)$$

136 which leads to

$$\text{JIF} = \frac{\mathbb{E}[g(X)]}{g(\mathbb{E}[X])} = \frac{f_1(\Psi, \theta)}{f_2(\Psi, \theta)} = f_3(\Psi, \theta) \quad (27)$$

137 This equation shows that the JIF factor is dependent on both the parameters of the
138 nonlinear relationship and the parameters of the distribution of the discharge. While
139 the function $g(x)$ remains constant for a given catchment property, the parameters θ can
140 change due to non-stationarity in the input time series. This non-stationarity may arise
141 from factors such as climate variability, land use changes, and alterations in hydrological
142 processes, leading to complex interactions within the hydrological system.

143 Climate significantly affects the parameters θ within the distribution of X . Variations
144 in precipitation patterns and extreme weather events can alter the flow characteristics of
145 a catchment area. For example, increased rainfall intensity may lead to higher peak flows,
146 thereby impacting the relationship between discharge and inundation. Conversely, pro-
147 longed drought conditions can reduce soil moisture and modify runoff patterns, affecting
148 the frequency and distribution of flood events.

149 Topographical features, including slope, elevation, and drainage density, also play a
150 crucial role in shaping hydrological responses within a catchment. The land surface's
151 geometry influences how water moves across the landscape, which affects both the timing
152 and magnitude of runoff. Steeper slopes may result in faster runoff and reduced infil-
153 tration, while flatter areas may facilitate greater water retention and slower flow. This
154 interplay between climatic and topographical factors can create complex responses in
155 flood behavior. In regions with diverse topography, the spatial distribution of rainfall
156 can lead to heterogeneous flood responses. Steep terrain might experience rapid runoff
157 and localized flooding, whereas low-lying regions may be more susceptible to prolonged
158 inundation.

159 In summary, while $g(x)$, the inundation response for a given stream flow, is primar-
160 ily governed by the inherent properties of the catchment, like its morphology, Jensen's
161 Inundation Factor is influenced by both climatic and topographical variables. The in-
162 troduction of non-stationarity due to changing land use and climate significantly alters

163 the hydrological dynamics of a region. Understanding these influences is crucial for effec-
 164 tively modeling flood inundation, particularly in the context of ongoing climate change
 165 and urbanization.

166 *2.2. Simulation experiments*

The overall methodology for the case studies is shown in Figure 4.

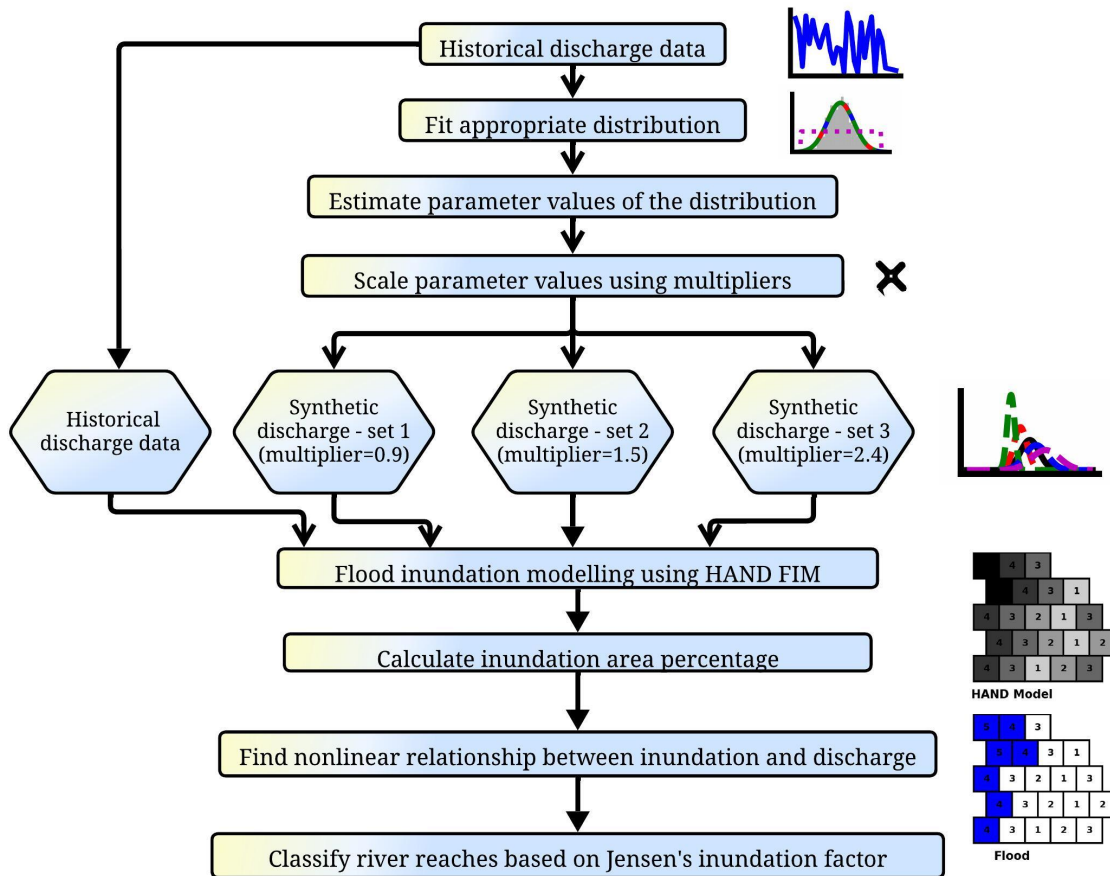


Figure 4: Overall methodology for the Case studies

167

168 *2.2.1. Study area*

169 The study area includes six catchments located across the United States: Upper
 170 Neuse, Middle Neuse, Upper Saline, Lake Conway, Rio-Granade Santa Fe, and Ouachita
 171 Headwaters (Figure 5). These catchments were selected to represent diverse hydrolog-
 172 ical and geographical characteristics. The Upper Neuse and Middle Neuse catchments,
 173 located in North Carolina, are characterized by steep topography and mixed land use, in-
 174 cluding forest, urban, and agricultural areas[1]. The Upper Saline catchment in Arkansas
 175 is characterized by predominantly rural land use and varied topography, including moun-
 176 tainous terrains. Lake Conway, in Arkansas, is situated in a low-lying coastal plain with
 177 significant urbanization around the lake area. The Ouachita Headwaters catchment,

178 spanning Arkansas and Oklahoma, is characterized by diverse land use and relatively
 179 steep topography. This selection of catchments allows for a comprehensive analysis of
 180 the impact of river discharge nonstationarity on flood inundation extents across different
 geographical and hydrological settings.

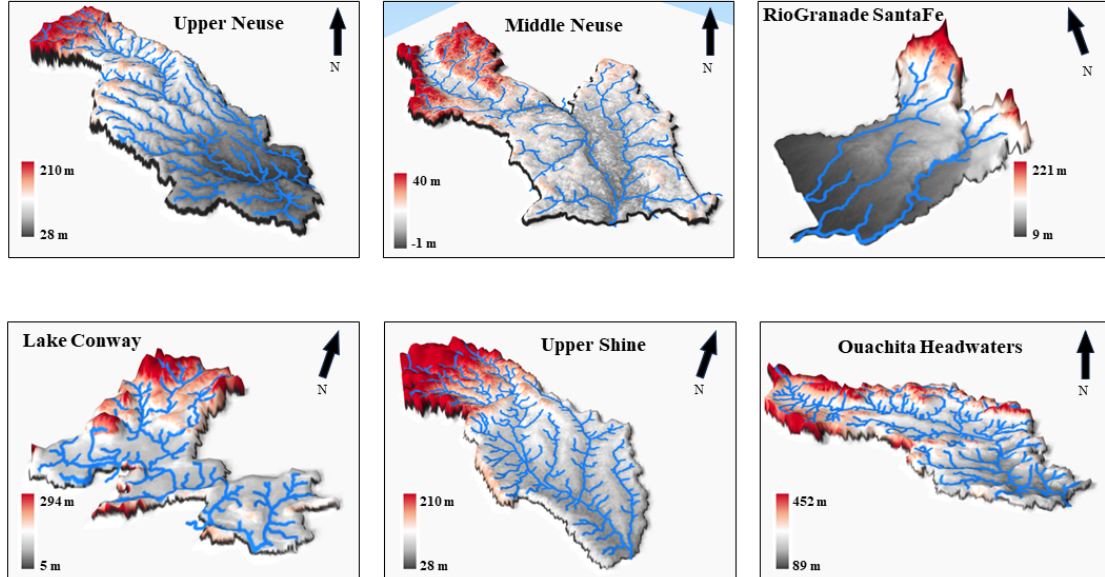


Figure 5: Study area showing the six catchments across the United States.

181

182 2.2.2. Discharge data

183 United States Geological Survey (USGS) gauge discharge data for 20 years are used for
 184 these six selected catchments(<https://waterdata.usgs.gov/nwis/>). This hydrological data
 185 set included long-term historical river discharge records available on daily interval. These
 186 records were crucial for understanding the variability and characteristics of river flow in
 187 each catchment.

188 2.2.3. Synthetic discharge generation

189 Statistical analysis was performed to determine the best probability distribution that
 190 fits with the discharge data collected for each catchment. For all six catchments, we
 191 used the lognormal distribution as the best fit. This choice was guided by the statis-
 192 tical properties of river discharge data, which often exhibit skewness and heavy tail —
 193 characteristics well-captured by the Lognormal distribution [19, 20]. The Lognormal
 194 distribution is defined as follows:

$$f(x | \mu, \sigma) = \frac{1}{x\sigma\sqrt{2\pi}} \exp\left(-\frac{(\ln x - \mu)^2}{2\sigma^2}\right) \quad (28)$$

195 where μ is the location parameter of and σ is the shape parameter of the log-normal
 196 distribution

197 For discharge shifts, we generated synthetic samples with the following process:

198 Step 1: Calculation of parameters: For each catchment, the parameters of the stream
199 flow time series were estimated using maximum likelihood estimation. In log-normal
200 distributed stationary time series, the mean and standard deviations are the best repre-
201 sentative of the location μ and scale σ parameters. These parameters provide a statistical
202 basis for subsequent simulations.

203 Step 2: Discharge shift:The calculated parameters from the gauge data (μ and σ) are
204 multiplied with different factors to implicate various discharge shifting scenarios. These
205 factors are random (0.9,1.5, and 2.4), which adjusted the original distribution parameters
206 to reflect different discharge scenarios. These shifts are implemented to generate various
207 discharge scenarios, a potential indicator of climate change impacts, land use alterations,
208 urbanization, and hydrological modifications. By applying these shifts, we generated
209 synthetic discharge values that allowed for a comprehensive evaluation of how changes
210 in river flow affect flood inundation across different catchments. These new parameters
211 represent shifts in the variability and mean of the discharge.

212 Step 3: Generation of synthetic data: For each case study, we calculated new param-
213 eters (μ_{new} ,and σ_{new}) as shown below:

$$\mu_{\text{new}} = \text{factor} \times \mu \quad (29)$$

$$\sigma_{\text{new}} = \text{factor} \times \sigma \quad (30)$$

214 where μ and σ are the original location and scale parameters, while μ_{new} and σ_{new}
215 are the new parameters used to generate the synthetic discharge data. These synthetic
216 discharge time series are then used to simulate various flood conditions and assess the
217 impact of different discharge scenarios on flood inundation.

218 *2.2.4. Flood inundation mapping and percent flooding calculation*

219 The NOAA-OWP HAND FIM (Height Above the Nearest Drainage - Flood Inunda-
220 tion Mapping) is used to simulate flood inundation extents for various scenarios across all
221 six catchments[21].OWP HAND-based FIM is a continental-scale flood inundation model
222 capable of producing flood depth and inundation extents at high spatial and temporal res-
223 olution. The model runs at 10m resolution 3DEP elevation products with hourly national
224 water model (NWM) retrospective and forecasted streamflow.The model uses Manning’s
225 equation to construct reach-averaged synthetic rating curves for the entire watershed
226 (HUC-8) using the reach-averaged cross-sectional parameters. The HAND-derived syn-
227 thetic rating curves (SRCs) provide the key piece of information needed to convert an
228 input streamflow value (NWM retrospective/forecast streamflow) into the corresponding
229 HAND stage value. This stage-discharge relationship facilitates the production of flood

230 inundation maps (FIMs) . If the stage for a given discharge is higher than the relative
 231 elevation, water spills into the floodplains or vice versa. Although this framework does
 232 not account for mass and momentum conservation or solve the Saint-Venant Equations,
 233 its scalability and low computational cost make it suitable for large-scale simulations [22]
 234 Manning’s equation is given by:

$$Q = A \cdot \frac{1}{n} \cdot R^{2/3} \cdot S_0^{1/2} \quad (31)$$

235 where Q is the discharge, A is the cross-sectional area, n is the Manning’s roughness
 236 coefficient, R is the hydraulic radius, and S_0 is the slope of the flow.

237 The OWP HAND FIM flood model runs at the watershed scale, including the head-
 238 water streams. Being computationally efficient, the model can produce 10m resolution
 239 flood maps at HUC-8 scale watersheds in less than a minute. The model was config-
 240 ured to run automatic simulations for all six basins under four different scenarios: one
 241 using gauge discharge data and three with shifted discharge datasets (0.9, 1.5, and 2.4).
 242 Each scenario produced 100 binary flood rasters, resulting in a total of 2,400 samples. In
 243 this case, we ran the model multiple times, and to make the computation less tedious,
 244 we focused on major streams having a stream order higher than 4. Using these simu-
 245 lated flood inundation extents, we calculated the percentage of flooding for each scenario
 246 based on the binary flood maps. For each scenario i , the flood inundation percentage (P_i)
 247 is calculated as:

$$P_i = \frac{\text{Number of Flooded Pixels}}{\text{Total Number of Pixels}} \times 100 \quad (32)$$

248 Percent inundation extents are plotted against the river discharge to analyze the rela-
 249 tionship between flood percentage and discharge. This analysis aimed to identify whether
 250 the relationship was linear, sub-linear, or super-linear across the different catchments.

251 *2.2.5. Application of Jensen’s inequality and JIF*

252 Jensen’s inequality and Jensen’s Inundation Factor (JIF) are crucial for analyzing
 253 the nonlinear relationships between river discharge and flood inundations. These con-
 254 cepts, thoroughly explained in Sections 2.1 and 2.2, are applied to assess how nonlinear
 255 transformations impact flood predictions.

256 Jensen’s inequality provides a framework to understand how the expected value of a
 257 nonlinear function of a random variable, such as river discharge, differs from the non-
 258 linear function of the expected value. This is particularly relevant for flood modeling
 259 because river discharge often exhibits complex, nonlinear behavior. By applying Jensen’s
 260 inequality, it is possible to evaluate whether flood inundation is more or less sensitive to
 261 changes in discharge than would be predicted by a simple linear model.

262 The Jensen’s Flood Inundation Factor (JIF) extends this analysis by quantifying the

263 degree of nonlinearity in the relationship between streamflow and flood extent. JIF is
264 calculated by comparing the expected value of the nonlinear function of discharge to
265 the function evaluated at the expected value of discharge. This factor provides insights
266 into whether flood inundation indicates damping or amplification effects in response to
267 variations in river discharge.

268 **3. Results and discussion**

269 *3.1. Discharge and inundation distributions across various basins*

270 To analyze the relationship between river discharge and flood inundation, we employed
271 a series of parameter shifts as described in the methodology. .

272 Figure 6 shows the frequency of discharge and inundation across various basins using
273 observed and synthetic discharge values. A significant change in discharge values was ob-
274 served after shifting the distribution parameters. For instance, in the Upper Neuse, the
275 maximum observed discharge is approximately 254 m³/s, while with the synthetic dis-
276 charge values, the maximum value from our sample set reaches 600 m³/s. The percent of
277 inundation increases as we move from the gauge discharge towards the 2.4 shifted scenar-
278 ios by an amount of 40%. From this scenario analysis, we found that even after there is a
279 substantial change in the discharge (in this case, different synthetic discharge scenarios)
280 potentially attributed to climate change, land use alterations, urbanization, or hydro-
281 logical modifications, there is no significant flood inundation we observed for the Upper
282 Neuse basin. The relationship we observed is sublinear between the discharge-inundation,
283 where even the extreme shifts in discharge lead to a minor increase in inundation from
284 the actual inundation(0.1% to 0.17%).

285 In the Middle Neuse catchment, gauged discharge values range from 0.17 to 257 m³/s,
286 with maximum shifting we get the maximum discharge of 800 m³/s. Similar to the Up-
287 per Neuse, the frequency of inundation shows a considerable increase across synthetic
288 discharge scenarios, where inundation percentages increase from 4% to nearly 24%. This
289 indicates that the Middle Neuse catchment is also sensitive to changes in discharge values
290 and shows a nonlinear relationship between discharge and inundation extent. Conversely,
291 the Rio Grande Santa Fe shows a consistently lower discharge distribution, with gauged
292 discharge ranging from 2 to 70 m³/s. The response of inundation to synthetic discharge
293 shifts is less pronounced here, with inundation percentages increasing only from 5% to
294 25%. This suggests that the Rio Grande Santa Fe catchment may have a lower sensi-
295 tivity to discharge changes, indicating a potentially more resilient hydrological response
296 compared to the other basins analyzed while still showing nonlinear dynamics in the
297 relationship between discharge and inundation.

298 The Lake Conway catchment presents a broad discharge distribution, with observed
299 values from 0.25 to 110 m³/s, and the maximum shift in synthetic discharges is 311%.

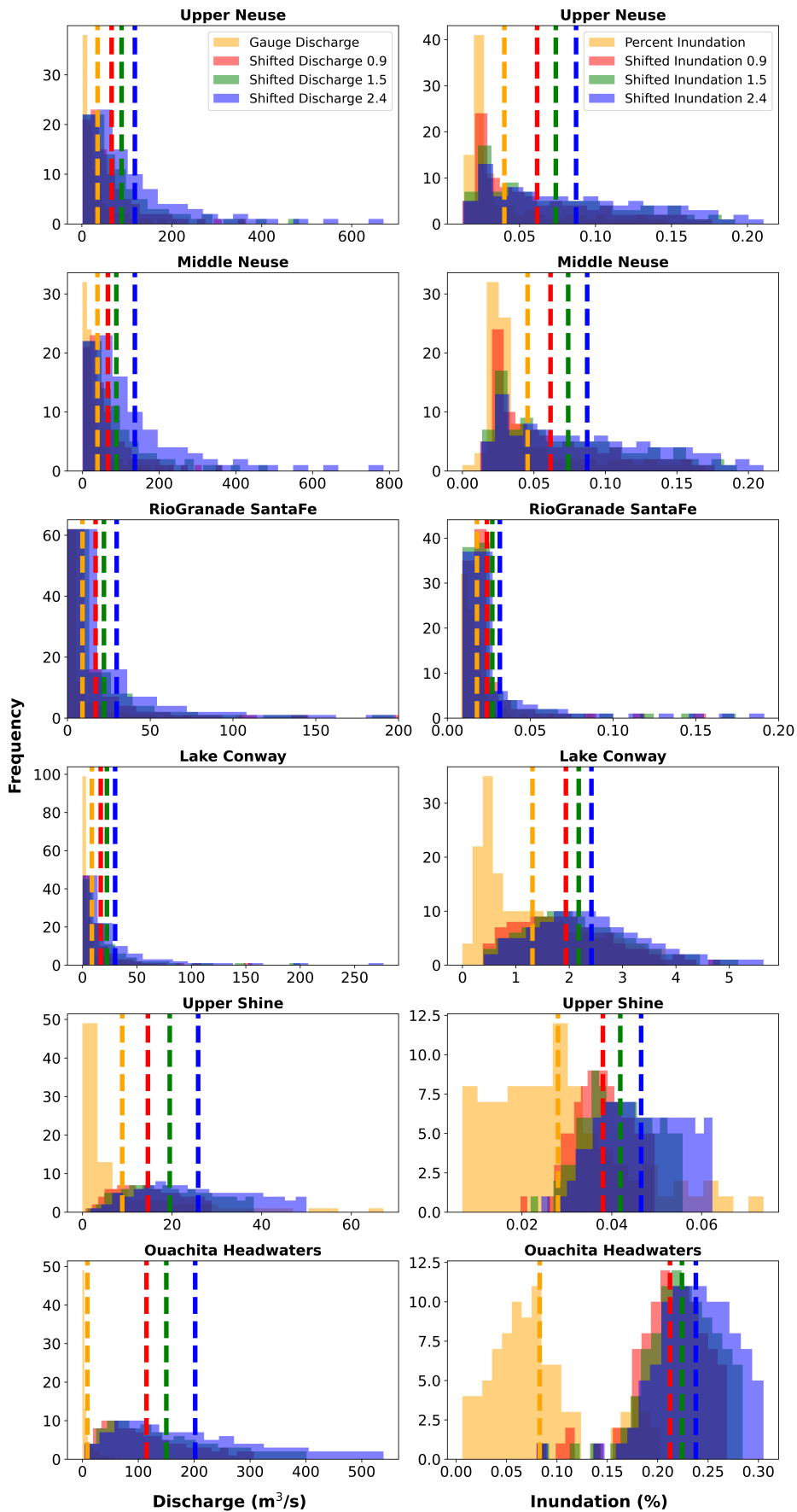


Figure 6: Comparison of actual and synthetic discharge frequency across various basins and scenarios

300 In this basin, the frequency of inundation dramatically increases, particularly with 2.4
301 scenario, where inundation percentages increase to over 45%. The Upper Shine basin
302 shows a narrower discharge distribution, with observed values ranging from 0.009 to 65
303 m^3/s , and after shifting, the maximum discharge value is $121\text{m}^3/\text{s}$. Despite this increase
304 in discharge, the inundation percentage increased to 35%.

305 Lastly, the Ouachita Headwaters show a diverse discharge distribution, with observed
306 values from $0.25 \text{ m}^3/\text{s}$ to $117 \text{ m}^3/\text{s}$ and synthetic discharges reaching up to a maximum
307 value of $550 \text{ m}^3/\text{s}$. Notable increases in inundation frequency are evident, particularly at
308 the highest discharge values, with inundation percentages rising to 51%. This observation
309 underscores the non-linear relationship between discharge and inundation, consistent with
310 the patterns observed in all the other five basins.

311 *3.2. Discharge-inundation dynamics across river basins*

312 Building on the observations from Section 4.1, discharge versus inundation is plotted
313 to further clarify how the relationships between discharge and inundation extents across
314 different basins based on 100 simulations for each basin and each scenario, presented
315 in both normal and semi-logarithmic scales, as shown in Figure 7. It can be seen that
316 overall, all catchments show a nonlinear and predominantly sub-linear trend. However,
317 some catchments initially display a linear response to increases in discharge, transitioning
318 to a sub-linear trend after reaching a specific threshold. Others show a super-linear trend
319 at lower discharge values before shifting to a sub-linear trend.

320 In the Upper Neuse Basin, a significant increase in inundation occurs at discharge
321 levels of approximately $150\text{--}200 \text{ m}^3/\text{s}$, marking a critical threshold where small increases
322 beyond this point can lead to much greater flooding. The Middle Neuse Basin also
323 shows a notable rise in inundation once discharge levels exceed $200 \text{ m}^3/\text{s}$, emphasizing
324 its sensitivity beyond this discharge value.

325 In contrast, the Rio Grande Santa Fe Basin presents a more linear trend, appearing
326 relatively straight throughout its range, indicating a consistent relationship between dis-
327 charge and inundation levels. This pattern is echoed in the Lake Conway Basin, which
328 similarly exhibits a gradual rise starting at comparable discharge levels. The Upper Shine
329 Basin displays a steady increase in inundation up to $30\text{--}40 \text{ m}^3/\text{s}$, followed by a sharper
330 rise, suggesting that minor changes in discharge can have a considerable impact. Fi-
331 nally, the Ouachita Headwaters Basin experiences a gradual rise in inundation up to 200
332 m^3/s , after which the trend shifts, particularly indicating a transition from sub-linear to
333 super-linear behavior as discharge crosses $80 \text{ m}^3/\text{s}$.

334 The observed variability across the basins reveals the intricate dynamics of how each
335 basin responds to fluctuations in discharge levels. Understanding these distinct trends is
336 crucial for developing effective flood risk management strategies, as it allows for tailored
337 approaches that account for the unique hydrological behaviors of each basin.

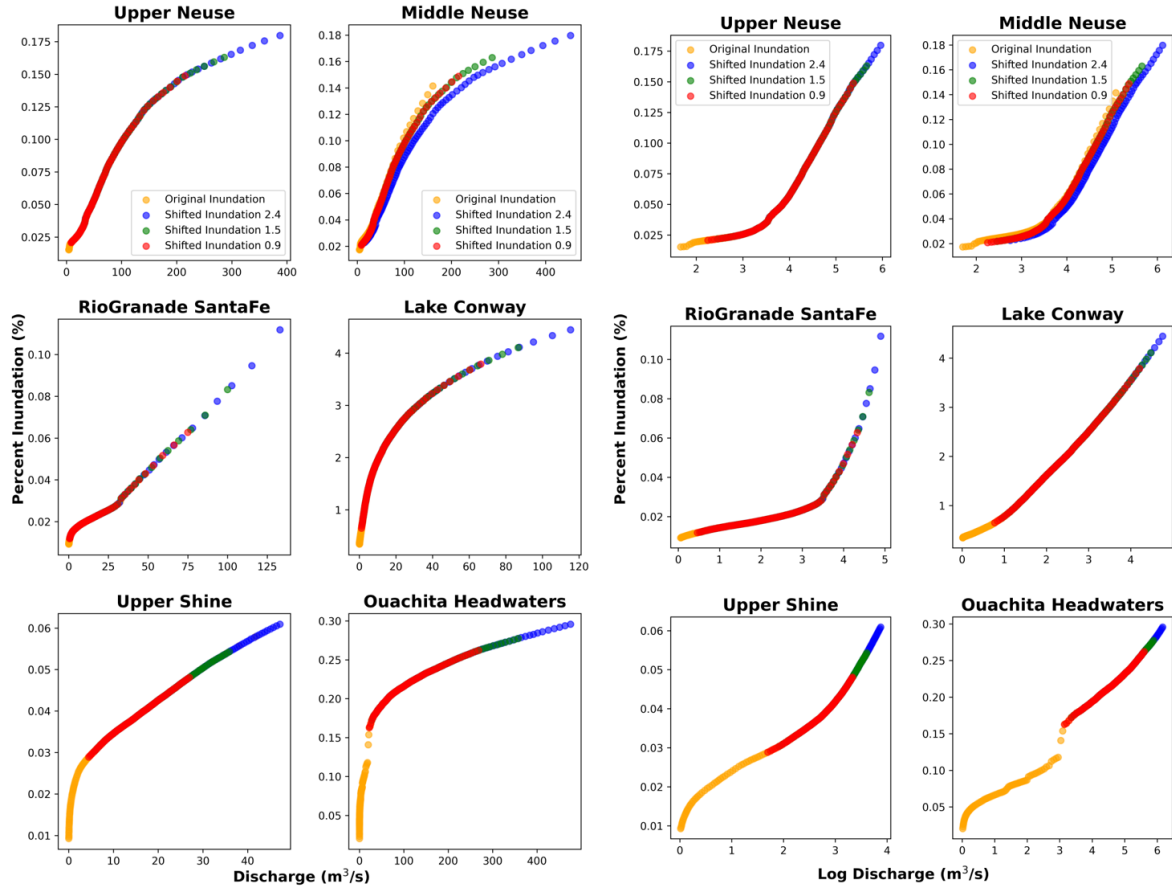


Figure 7: Relationship between discharge (m^3/s) and percent inundation for various river basins, illustrated through linear scales (first and second columns) and semi-logarithmic scales (third and fourth columns)

338 3.3. Analyzing JIF

339 JIF for the different basins under various discharge scenarios as shown in Table 1 reveal
 340 important patterns regarding flood inundation dynamics. It provides insights into the
 341 non-linear responses of each basin to changes in discharge and highlights the differential
 342 impact of flood events across different basins.

343 The Upper Neuse and Middle Neuse basins were found to have higher JIF values
 344 across all scenarios, particularly under the original and shifted 0.9 discharge scenarios,
 345 with values of 0.98 and 0.97, respectively, in the original scenario and 0.85 and 0.86 in
 346 the shifted 0.9 scenario. However, as the discharge shifts further to 1.5 and 2.4 scenarios,
 347 the JIF values gradually decrease, indicating a sub-linear flood response. This trend
 348 indicates that, despite increasing discharge values, the flood extent in these basins does
 349 not increase proportionally, likely due to the presence of better drainage systems or
 350 resilient topographical features.

351 In contrast, the Rio Grande Santa Fe basin is more sensitive to discharge increases. Its
 352 JIF value drops from 0.86 in the original scenario to 0.62 under the shifted 2.4 discharge
 353 scenario. A JIF value below 1 typically reflects a sublinear relationship between discharge

Table 1: Jensen’s Inundation Factor for different basins.

Basin	Scenarios			
	Original	Shifted_0.9	Shifted_1.5	Shifted_2.4
Upper Neuse	0.98	0.85	0.80	0.78
Middle Neuse	0.97	0.86	0.81	0.78
RioGranade SantaFe	0.86	0.84	0.76	0.62
Lake Conway	0.71	0.67	0.75	0.72
Upper Shine	0.83	0.99	0.99	0.98
Ouachita Headwaters	0.85	0.96	0.96	0.96

354 and inundation extent, meaning that as discharge increases, the extent of flooding is
355 increasing very gradually. However, this substantial drop signifies a shift in how the basin
356 manages excess water, suggesting that extreme discharge scenarios may lead to less severe
357 flooding than expected. The decline indicates a tipping point in the basin’s capacity,
358 implying that while incremental increases in discharge may not cause significant flooding,
359 the overall effectiveness in handling larger discharges is declining. The vulnerability of
360 the Rio Grande Santa Fe basin may also stem from its topographical characteristics, as
361 flatter areas often have less capacity to convey water away, leading to greater inundation
362 extents when discharge increases significantly. With elevations ranging from 9 to 221
363 meters, the presence of only a small higher elevation area may limit the basin’s ability to
364 effectively manage excessive runoff, especially during extreme discharge events.

365 The Lake Conway basin shows a distinct characteristic compared to the others, with
366 relatively lower JIF values across all scenarios, beginning at 0.71 in the original discharge
367 scenario. Interestingly, the JIF increases slightly to 0.75 under the shifted 1.5 scenario
368 before slightly decreasing again to 0.72 under the shifted 2.4 scenario. This fluctuating
369 pattern could be due to Lake Conway’s urban or semi-urban characteristics, where even
370 small shifts in discharge can trigger significant changes in flood extent. The basin’s
371 elevation ranges from 5 to 224 meters, with a majority of areas at higher elevations,
372 suggesting that topographical features may influence its flood response. However, despite
373 these higher elevations, certain flatter regions may limit the efficient movement of excess
374 water during flood events, indicating a vulnerability to flooding.

375 The Upper Shine and Ouachita Headwaters basins show relatively stable JIF values
376 across all discharge scenarios. Both basins consistently show JIF values above 0.96 but
377 below 1, indicating a nearly linear or slightly sub-linear relationship between discharge
378 and flood extents. This suggests that increasing discharge does not significantly impact
379 flooding in these areas. The Upper Shine basin, with elevations ranging from 28 to 210
380 meters and predominantly higher elevation areas, along with the Ouachita Headwaters
381 basin, which ranges from 89 to 452 meters and has significant portions at higher elevations,
382 likely benefits from their topography. This higher elevation landscape enhances their

383 capacity to manage excess discharge effectively, resulting in a stable flood extent even as
384 discharge values increase.

385 Thus, while the flood inundation extent consistently increases with discharge, the
386 rate of increase and the mean inundation, given the distribution of flow, are influenced
387 by flow probability parameters and nonlinearity factors. The introduction of the novel
388 Jensen's Inundation Factor (JIF) in this paper offers valuable insights into the flood re-
389 sponse of different basins. Changes in JIF values indicate that as discharge increases,
390 the relationship between discharge and flood inundation becomes more complex. Thus,
391 JIF introduced here can provide a clearer understanding of the dynamics of flood in-
392 undation across various basins. By identifying basins that may be more susceptible to
393 flooding under increased discharge scenarios using JIF, policymakers can prioritize areas
394 for infrastructure improvements and apply mitigation strategies.

395 *3.4. Insights and implications for future studies*

396 A major insight from this study is the nonlinear relationship between shifts in precipi-
397 tation and the associated inundation extent, primarily influenced by complex interactions
398 within diverse landscapes. This finding underscores the necessity of considering shifts in
399 precipitation when assessing flood risks as well as critical factors such as the Jensen Inun-
400 dation Factor (JIF). The JIF provides a nuanced understanding of inundation dynamics,
401 indicating that inundation extent or flood risk does not scale directly with precipitation;
402 instead, landscape features act as amplifiers or dampers, significantly affecting the extent
403 and intensity of flooding.

404 Incorporating the JIF into our model allows us to account for variable responses of
405 different basins to changes in precipitation volume, particularly with different basins
406 characterized by diverse topographic features. The observed nonlinear dynamics chal-
407 lenge conventional flood risk estimates that typically assume a proportional relationship
408 between precipitation and flooding. Our findings indicate that regions experiencing simi-
409 lar precipitation changes may face disparate flood risks shaped by their unique landscape
410 characteristics.

411 The implications of this study advocate for a transformative shift in flood risk mod-
412 eling and urban development strategies within flood-prone regions. Resilience planning
413 must incorporate these nonlinear flood responses to better anticipate the impacts of even
414 modest increases in precipitation. By employing the JIF in risk assessments, urban de-
415 velopers and policymakers can effectively identify areas vulnerable to heightened flooding
416 and prioritize adaptive infrastructure or natural buffers.

417 In the context of a rapidly changing climate, where precipitation patterns are increas-
418 ingly erratic, integrating the JIF or similar metrics into flood risk assessments fosters
419 a more targeted approach to flood resilience. Rather than treating flood risk as a di-
420 rect function of precipitation alone, this framework accounts for localized landscape re-

421 sponses, guiding the formulation of policies that fortify vulnerable regions. Through this
422 lens, flood risk management evolves into a more adaptive and nuanced process, congru-
423 ent with the intricate nature of natural landscapes, ultimately enabling the development
424 of more effective flood protection strategies for communities facing climate-driven flood
425 intensification.

426 **4. Conclusion**

427 Our study highlights the effect of nonlinearity between flood inundation extent and
428 river discharge on flood risk assessments. Our findings suggest that an increase in dis-
429 charge due to increased precipitation alone will not necessarily lead to a proportional
430 increase in inundation extent. Instead, the introduction of Jensen’s Inundation Factor
431 reveals that various nonlinear dynamics, including damping and amplification mecha-
432 nisms, significantly influence the flood behavior of a catchment, influenced by factors
433 such as topography, climatology, and hydrodynamics.

434 Our analysis emphasizes that the anticipated increase in flood inundation is usually
435 not a linear function of flood discharge. Local hydrological conditions and catchment
436 characteristics can significantly attenuate or amplify the relationship between precipita-
437 tion and flood extent in these catchments. While, for the six analyzed catchments, we
438 observed only damping effects in inundation shifts, other catchments around the world
439 may exhibit either damping or amplification depending on specific local conditions. This
440 variability emphasizes the necessity for context-specific evaluations, as the interplay be-
441 tween topography and hydrodynamics can influence flood inundation outcomes and the
442 severity of damage.

443 Thus, future studies should focus on systematically identifying and classifying these
444 nonlinearities across all populated reaches of major rivers and identifying critical thresh-
445 olds related to flood risk. Understanding these thresholds will be essential for determining
446 when specific catchments are likely to experience significant flooding under varying con-
447 ditions. This classification will be crucial for informing policy decisions related to flood
448 management and insurance claims, ultimately aiming to enhance community resilience
449 against flooding.

450 This work, particularly the introduction of JIF, provides a theoretical framework for
451 a desirable global analysis that would characterize flooding nonlinearities and summarize
452 the interplay of nonlinear hydrological responses and climatic conditions by using a single
453 index. As populations continue to grow in flood-prone areas, quantifying Jensen’s Inun-
454 dation Factor will be valuable for risk assessment and, in turn, for improving community
455 preparedness.

456

457

458 **Authorship contribution statement**

459 **Anupal Baruah:** Ideas, simulation, writing, reviewing, and editing. **Gilbert Hinge:**
460 Ideas, simulation, writing, reviewing, and editing. **Omar Wani:** Primary conceptualiza-
461 tion and supervision. Statistical analysis, writing, reviewing, and editing

462

463 **Data Availability Statement:** The USGS gauge data can be accessed from
464 <https://waterdata.usgs.gov/nwis>. The OWP HAND model can be accessed from
465 <https://github.com/NOAA-OWP/inundation-mapping>. The Digital Elevation Model (DEM)
466 used in this study is sourced from USGS.

467

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471 **Conflicts of Interest:** The authors declare no conflict of interest.

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