

1 **Challenging the turbidity current maximum run-up height**
2 **paradigm**

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22 **ABSTRACT**

23 Turbidity currents are a primary mechanism for transporting sediments, pollutants, and organic
24 carbon into the deep ocean. They are strongly influenced by seafloor topography because of
25 their relative bulk density and associated gravitational influence being 3-4 orders of magnitude
26 smaller than in terrestrial systems. Marked run-up of turbidity currents on slopes poses a hazard
27 to seafloor infrastructure, and leads to distinctive depositional patterns, yet the prediction of
28 run-up heights remains poorly understood because the present calculations are derived from
29 2D experimental configurations and/or numerical modelling and merely limited to scenarios in
30 which the flow strikes the topographic barriers orthogonally.

31 Here we present the results of 3D experiments in unconfined settings that are used to develop
32 a new analytical model that improves the prediction of maximum run-up heights of turbidity
33 currents that encounter topographic slopes of varying gradients and flow incidence angles. We
34 show that existing predictive models based on 2D confined flows focusing on frontal
35 topographic configurations underestimate the run-up heights of turbidity currents by
36 approximately 15-40%. Experimental results highlight the importance of considering the
37 energy contribution from internal pressure in the fluid, cross-stream and/or vertical velocities
38 and lateral flow expansion and divergence in unconfined flows. Our findings reveal that
39 intermediate slope gradients (ca. 30°) and (near-)perpendicular flow incidence angles generate
40 the highest run-up heights, up to 3.3 times the flow thickness. Novel analytical models are
41 presented subsequently for predicting maximum run-up height as a function of both the
42 gradient and incidence angle, comparing the models to the newly observed data. Such models
43 provide relatively more realistic estimates of run-up heights for flows on three-dimensional
44 slopes typical of natural systems.

45 These findings are critical for improving sediment transport models, predicting the distribution
46 of sediments, pollutants, and organic carbon in deep-sea environments, assessing seafloor
47 geohazards, and reconstructing ancient deep-water basin palaeogeographies.

48

49 **INTRODUCTION**

50 Turbidity currents are subaqueous, gravity-driven turbulent flows and are the primary
51 mechanism for transporting clastic sediments, microplastics, organic carbon, and dissolved
52 nutrients and pollutants from continental shelves to the deep ocean (e.g., Kuenen and
53 Migliorini, 1950; Dzulynski et al., 1959; Sestini, 1970; Normark et al., 1993; Kneller and
54 Buckee, 2000). These flows also present a significant geohazard and can cause catastrophic
55 damage to seafloor infrastructure, such as pipelines and communication cables (Carter et al.,
56 2014). Turbidity currents, along with other sediment gravity flows, commonly traverse seafloor
57 terrain characterized by substantial topography (e.g., Normark, 1985; Apps et al., 1994; Kneller
58 and McCaffrey, 1999), which is typically generated related to mass transport deposits, volcanic
59 features, tectonic deformation, salt or mud diapirism, and even abyssal plain mountains. Such
60 topography exerts a strong influence on the behaviour of turbidity currents, notably altering
61 their velocity and sediment concentration profiles, which in turn affects their sediment transport
62 capacity (e.g., Kneller et al., 1991; Edwards et al., 1994; Patacci et al., 2015; Tinterri et al.,
63 2016, 2022; Dorrell et al., 2018; Soutter et al., 2021; Keavney et al., 2024; Reece et al.,
64 2024a,b). Where the relief of the seafloor is sufficiently elevated, the turbidity current may be
65 blocked entirely. However, due to their reduced excess density—2–3 orders of magnitude
66 smaller than rivers in terrestrial systems—turbidity currents, especially the dilute upper portion
67 of the fluid, can ascend topographic barriers several times their flow thickness via a process
68 known as superelevation (e.g., Rottman et al., 1985; Muck and Underwood, 1990; Lane-Serff
69 et al., 1995; Kneller and McCaffrey, 1999; Keavney et al., 2024). In submarine canyon systems,

70 the Coriolis and centrifugal forces may further enhance the vertical run-up by generating lateral
71 pressure gradient forces in a canyon bend (e.g., Komar, 1969; Pirmez and Imran, 2003; Straub
72 et al., 2008; Lamb et al., 2008).

73 The phenomenon of turbidity current run-up on topographic slopes and the resultant deposition
74 of turbidites at elevations higher than the initial flow base has been documented in laboratory
75 experiments (e.g., Muck and Underwood, 1990; Soutter et al., 2021; Keavney et al., 2024),
76 modern and ancient field studies (e.g., Damuth and Embley, 1979; Cita et al., 1984; Dolan et
77 al., 1989; Wynn et al., 2010; Al A'Jaidi et al. 2004; Soutter et al., 2019). For example, turbidites
78 collected from the Ceara Rise, western equatorial Atlantic, indicate a run-up flow on slopes of
79 a horizontal distance of 40 km and a vertical distance of 150 ~ 400 m (Damuth and Embley,
80 1979). Ocean drilling core samples on the Tiburon Rise near the Barbados subduction zone
81 demonstrate a transport distance of ca. 1000 km of the terrigenous turbidite sands from South
82 America and a speculative vertical transfer of minimum 1,000 m (Dolan et al., 1989). The
83 ability of turbidity currents to run up slopes and deposit materials at higher elevations
84 exacerbates their threat as a geohazard to deep-sea infrastructure (e.g., Bruschi et al., 2006;
85 Carter et al., 2014) and presents challenges for modern human-made water reservoir de-risking
86 management (e.g., Wei et al., 2013). A comprehensive understanding of the factors governing
87 these processes is crucial for predicting the distribution of plastic and other pollutants on
88 seafloor topographic slopes (e.g., Haward et al., 2018; Kane et al., 2020), assessing the impacts
89 on deep-sea oxygenation, and reconstructing the paleogeography of ancient deep-water basin-
90 fills (e.g., Sinclair, 1994; Lomas and Joseph, 2004; Bell et al., 2018).

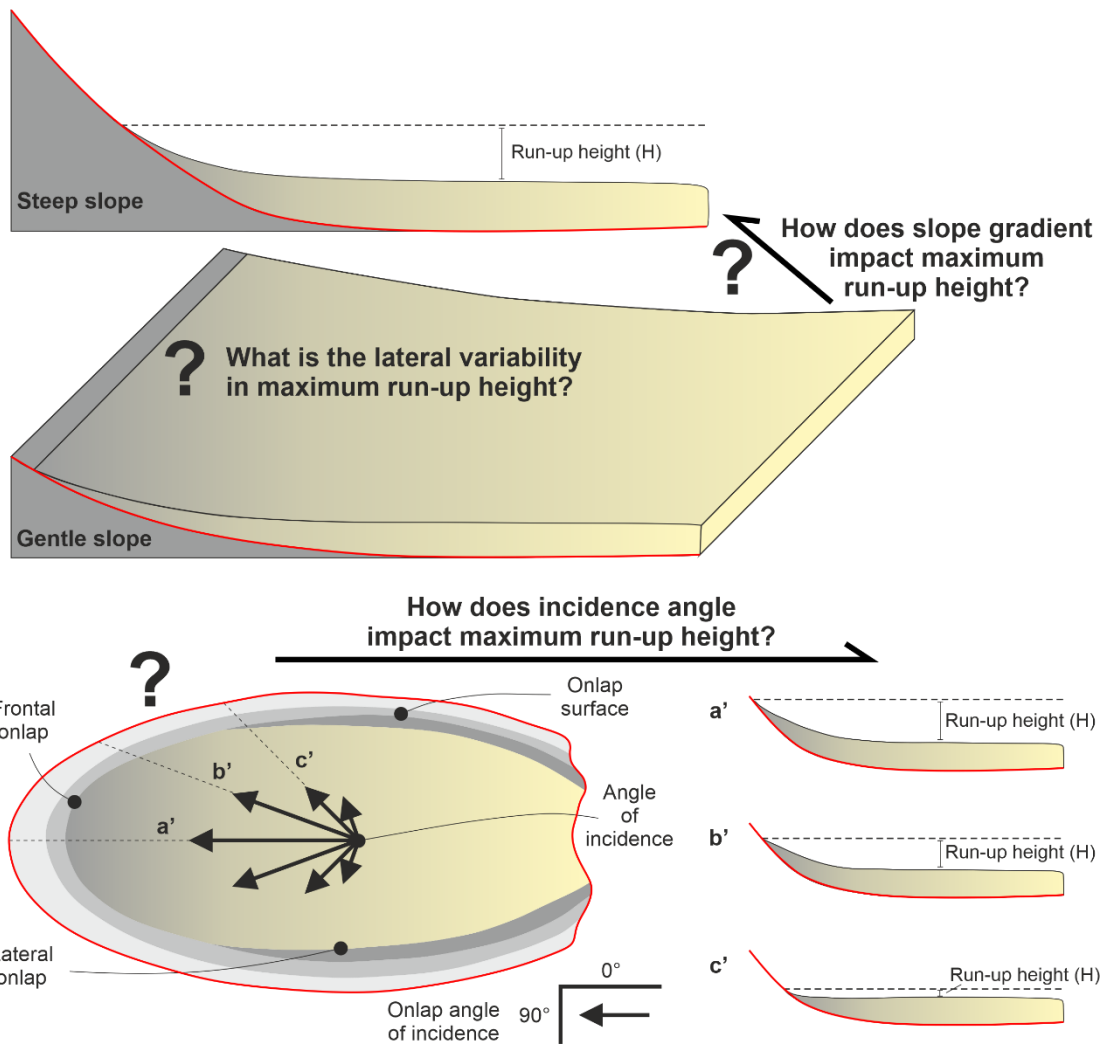
91 Despite their global importance, development of the theory on run-up elevation has mainly
92 relied on numerical and physical modelling, possibly due to a lack of direct measurements of
93 the interaction of unconfined turbidity currents and seafloor topography in the field. The
94 existing models to predict maximum run-up elevation are primarily either based on scaled-

95 down 2D narrow flume experiments and/or numerical modelling (Rottman et al., 1985; Muck
96 and Underwood, 1990; Lane-Serff et al., 1995; Kneller and McCaffrey, 1999) and have been
97 limited to scenarios in which the flow strikes the topographic barriers orthogonally. 2D flume-
98 tank experiments suggest that the body of density currents striking a frontal topography may
99 rise up the topography to a height 1.5 to 2.5 times the flow thickness (Rottman et al., 1985).
100 Muck and Underwood (1990) noted the maximum run-up height of a subcritical turbidity
101 current is approximately 1.53 times the flow thickness, based on a simple numerical analysis
102 assuming a full conversion of kinetic energy to potential energy and frictional heat loss, with
103 further validation from 2D laboratory experiments using saline density currents. However, the
104 applicability of this predictive model into the natural world is questioned as it assumes a
105 uniform flow density (i.e., neglecting lateral and vertical density gradients) and flow velocity.
106 To date, Kneller and McCaffrey's (1999) analytical model represents the most common method
107 for estimating the maximum run-up height of overflows of turbidity currents on obstacles,
108 taking into consideration not only the energy balance from the kinetic energy (KE) of a current
109 to the potential energy gained as it moves up a topographic slope (Chow, 1959; Hungr et al.,
110 1984; Kirkgoz, 1983), but also the impact of flow density stratification and vertical flow
111 velocity variations over height, typical of natural systems in the field. This model assumes that
112 between the time of first encountering the topography and reaching its maximal elevation, a
113 fluid parcel in the current at initial height z within the flow transfers all its kinetic energy into
114 gravitational potential energy along with some energy lost to friction (cf. Allen, 1985; Muck
115 and Underwood, 1990). Hence, the maximum run-up elevation h_{max} of any parcel of the
116 density current is the sum of its initial elevation h and its height gain, and is given by $h_{max} =$
117 $z + \frac{\rho_z u_z^2 (1-E)}{2g\Delta\rho_z}$, where u_z is the component of velocity at initial height z that is normal to the
118 topography, g is gravitational acceleration, ρ_z is the density of the fluid at initial height z , $\Delta\rho_z$
119 is the density difference between ρ_z and the ambient fluid and E is the frictional energy loss

120 relative to the initial KE. The predicted maximum run-up height H_{max} of the overall flow is
121 then the maximum value of this estimated h_{max} for all the fluid parcels.

122 However, the applicability of these existing models, especially the Kneller and McCaffrey
123 (1999) method, into 3D, unconfined settings is yet untested. The present paradigms do not
124 account for the effect of different configurations of topographic slopes, i.e., incidence angle of
125 the flow onto the slope and slope gradient (**Fig. 1**). Furthermore, the lateral variability in
126 maximum run-up height potential on slopes in a strike direction, its characteristics as a function
127 of slope orientations and slope angles and therefore their potential impact on the magnitude of
128 maximum upslope run-up height is rarely explored (**Fig. 1**).

129 Here we develop a generic framework to predict the maximum run-up height up slopes when
130 3D, unconfined turbidity currents encounter topographic slopes with different configurations,
131 including incidence angle of the flow and slope gradient. First, we present the first experimental
132 measurements of run-up height notably in unconfined settings under controlled laboratory
133 conditions, using sustained, saline density currents, where the flow interacts with a planar
134 topographic slope of varying gradients, at a range of flow incidence angles. The flows were
135 designed to be unable to overtop the topographic slope but could flow downstream around the
136 slope. We utilised dissolved salt to represent fine mud in suspension that does not easily settle
137 out, and moves in bypass mode, and therefore flows herein can be considered to model low-
138 density turbidity currents (Sequeiros et al., 2010; Keavney et al., 2024; Reece et al., 2024a, b).
139 Second, we present a novel analytical model to predict maximum run-up height as a function
140 of both the slope gradient and incidence angle, comparing the models to the newly observed
141 experimental data. These newly developed numerical models afford relatively more realistic
142 estimates of run-up heights for unconfined flows on 3D slopes.



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144 Fig. 1. Schematic diagram illustrating the existing knowledge gaps in the understanding of the
 145 characteristics of run-up height potential of turbidity currents interacting with topographic
 146 slopes.

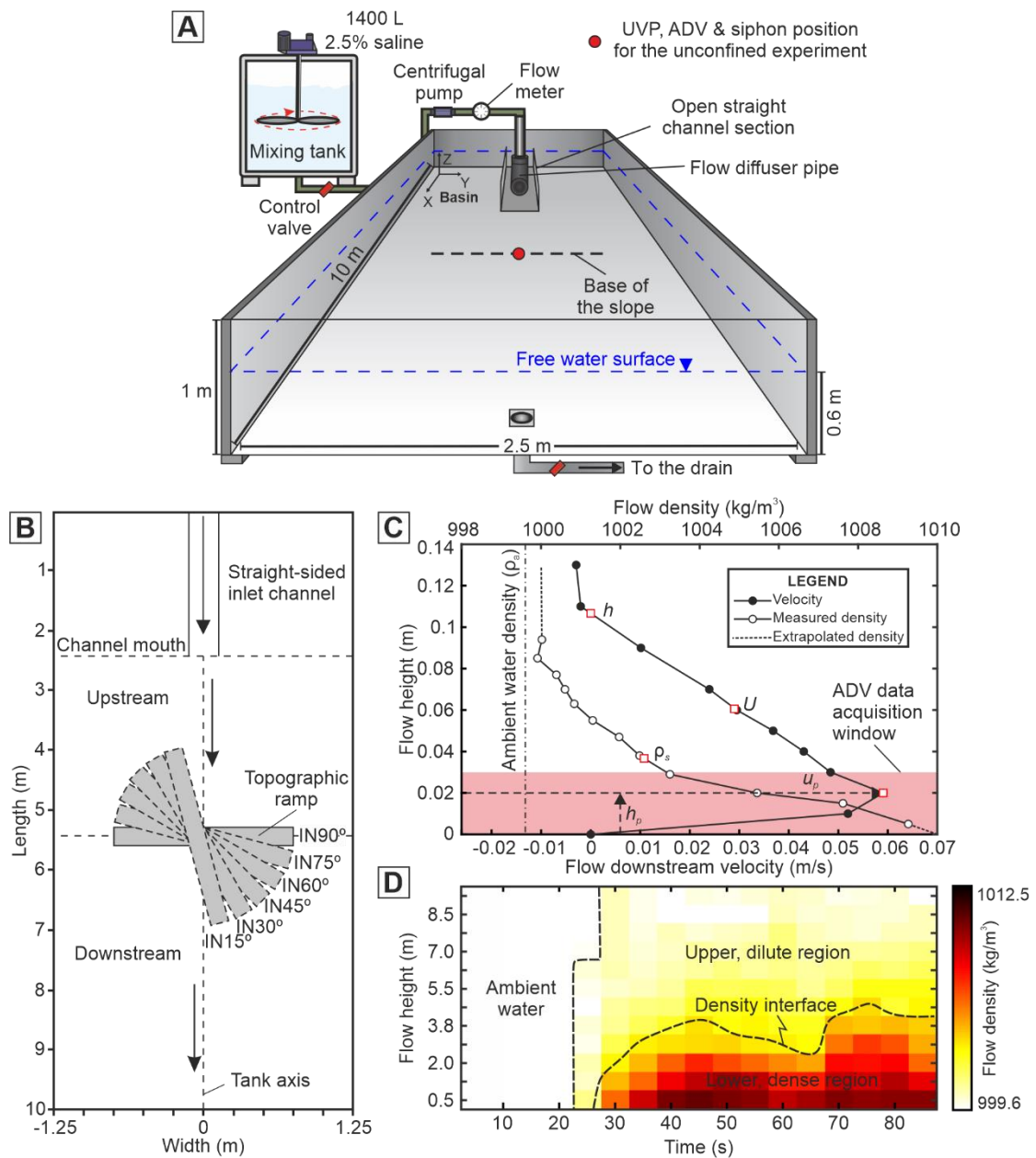
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148 **RESULTS**

149 **General flow behaviour of the ramp experiments**

150 A summary of the general flow behaviour in the ramp experiments and a representative video
 151 of Experiment S40°IN75° is given in the **Supporting Information (Video S1)**, with details
 152 documented in Keavney et al. (2024) and Wang et al. (2024). Notably, when the flow exits the
 153 channel, it moves as an unconfined density flow along the basin floor (**Fig. 2**). Once the flow

154 encounters the ramp, it decelerates and strikes the topographic slope with a strong flow
155 divergence character on the slope surface. Subsequently, the flow stratifies into a denser lower
156 part and a more dilute upper part. The dilute, upper part can run up the slope, thinning until it
157 reaches its maximum height H_{max} ('maximum run-up height', hereafter; cf. Pantin and Leeder,
158 1987; Edwards et al., 1994). The lower part collapses back down, either deflecting parallel to
159 the slope or reflecting towards the inlet. The flow stripping zone on the slope can be quantified
160 by the height of the initial reversal of the dense lower flow H_{min} and the maximum run-up
161 height H_{max} (Keavney et al., 2024). The initial reversal of the flow can undercut the outbound
162 flow and migrate upstream, leading to a thickening of the entire body of the density current,
163 known as an unsteady flow inflation phase. Eventually, as the parental flow re-establishes, the
164 thickening body of the density current in the basin becomes flat-topped, a quasi-stable flow
165 front develops on the slope surface. Finally, as the inlet flow wanes, the entire body of the
166 turbidity current collapses.



167

168 Fig. 2. (A) Schematic sketch of the experimental facility. The base of the topographic ramp is
 169 shown as a black dashed line. The positions of the Ultrasonic Velocity Profiler (UVP), Acoustic
 170 Doppler Velocimeter (ADV) and siphoning system for the unconfined experiment is also
 171 indicated. (B) Topographic configurations for ramps with different incidence angles relative to
 172 the incoming flow shown in a plan view. (C) Profiles of time-averaged flow downstream
 173 velocity and density for the experimental density current recorded at 3 m downstream from the
 174 channel mouth along the channel-basin centreline in the unconfined reference experiment. Both
 175 measurements were initiated 5 s after the current head passed and lasted for 30 s. The flow

176 depth h , maximum downstream velocity u_p , its height above the basin floor h_p , depth-
177 averaged downstream velocity U and depth-averaged density ρ_s are shown as red squares. The
178 ambient water density ρ_a was measured at 12 °C. (D) Time-series profiles of flow density
179 measured at 3 m downstream of the channel mouth along the channel-basin centreline.
180 Modified after Keavney et al. (2024).

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182 **Maximum run-up distance and height on slopes in the ramp experiments**

183 *Global distribution and comparison with existing models*

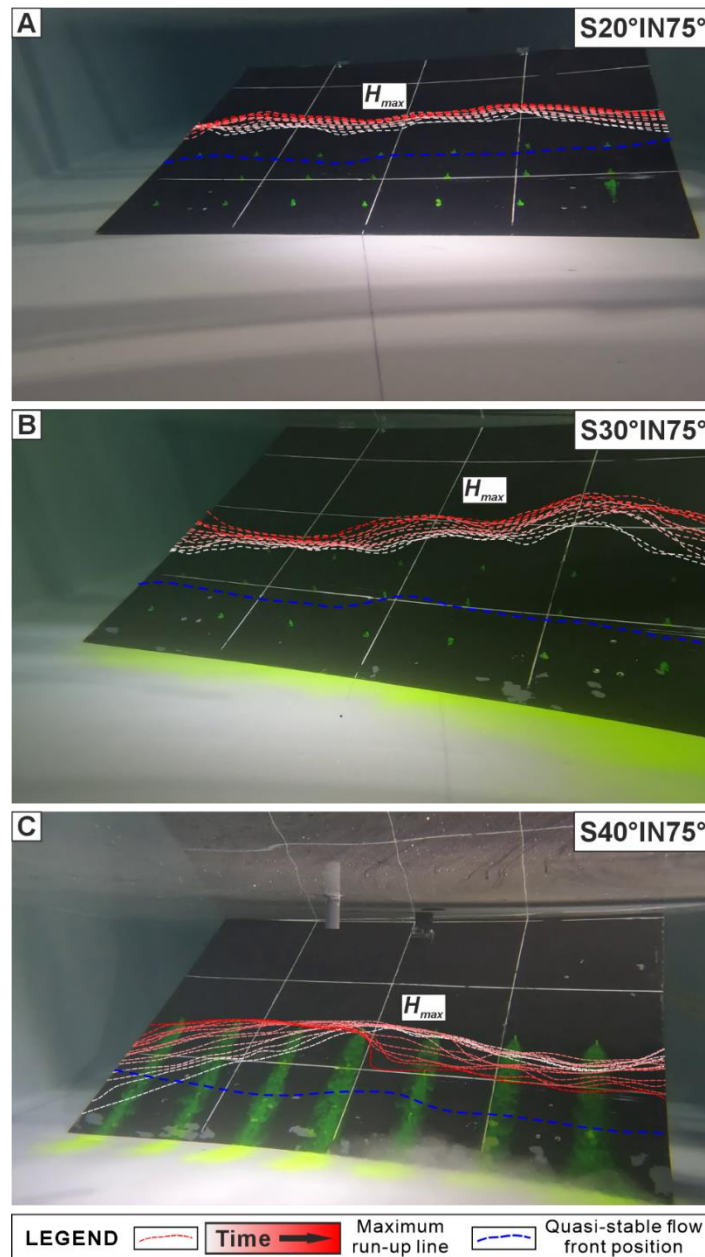
184 The measured maximum run-up distance L_{max} and height H_{max} for all the ramp experiments
185 ranges from approximately 2.3 to 7.9 times flow thickness and 1.3 to 3.3 times flow thickness,
186 respectively (**Fig. 3**). The measured maximum run-up height in our frontal experiments
187 (Experiments S20°IN90°, S30°IN90° and S40°IN90°) is shown to be approximately 2.5 ~ 2.7
188 times flow thickness, which is relatively higher than those predicted by the Kneller and
189 McCaffrey (1990) (see **Supporting Information 2** for the details on the modelled estimation)
190 and the Muck and Underwood (1990) methods and marginally higher than the upper limit of
191 the prediction interval by Rottman et al. (1985).

192 *Variation of incidence angles of the current onto the slope*

193 Maximum run-up distance and height on slopes as a function of incidence angles of the current
194 with the slope are examined for experiments with the same slope gradient (**Fig. 3G, 3I**).
195 Notably, for lower incidence angles, the maximum run-up distance on slopes decreases
196 markedly ($L_{max} = 0.44$ m to 0.87 m for experiments of slope gradient of 20°; $L_{max} = 0.28$ m
197 to 0.72 m for experiments of slope gradient of 30°; $L_{max} = 0.25$ m to 0.45 m for experiments
198 of slope gradient of 40°; **Fig. 3G**). However, in Experiment S30°IN90°, the maximum run-up

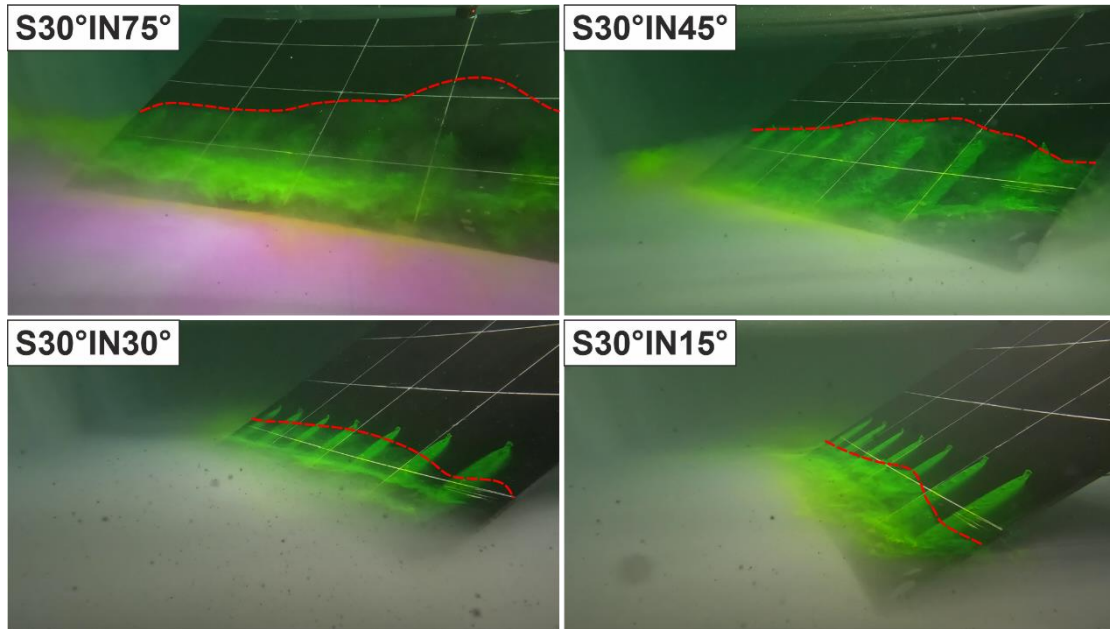
199 distance on slopes reaches ca. 0.55 m, which is unexpectedly shorter than that documented in
200 experiments S30°IN75°, S30°IN60° and S30°IN45° ($L_{max} = 0.72, 0.59$ and 0.58 m,

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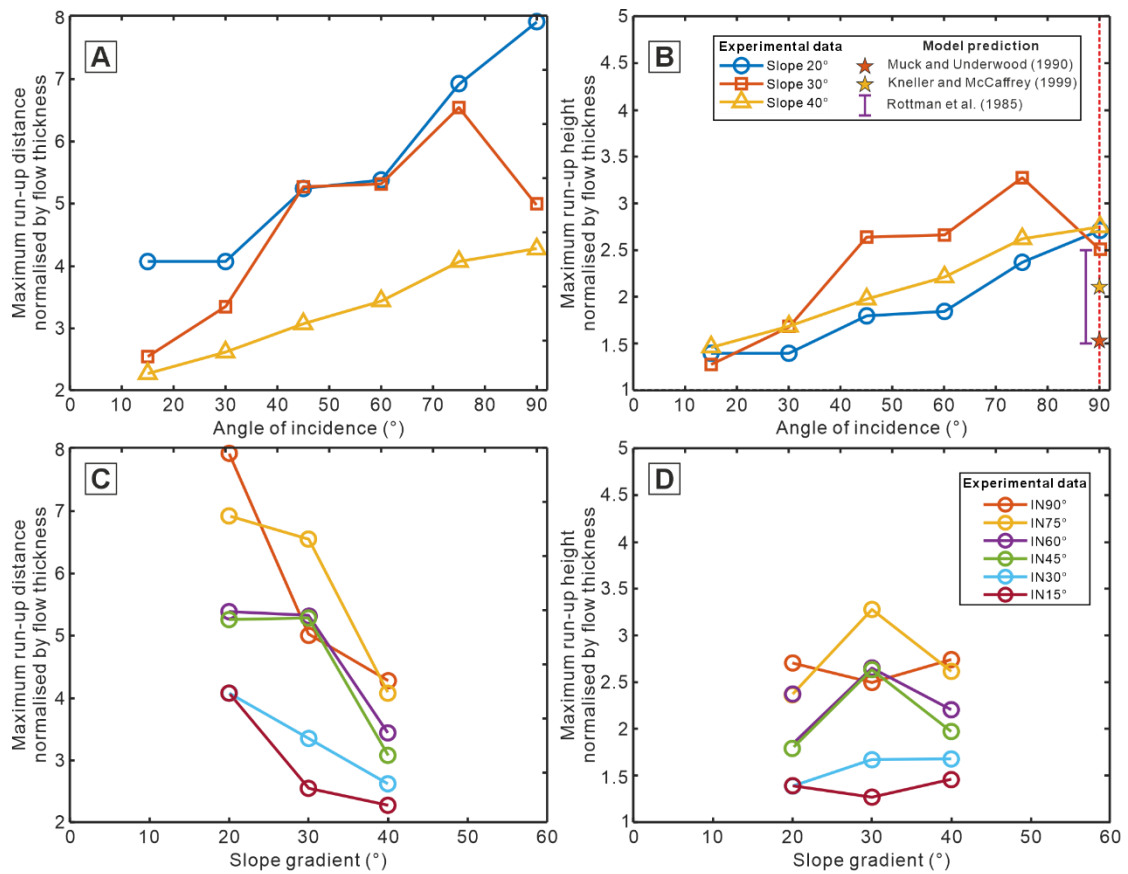
203 Fig. 3a. Representative front-view photographs depicting the fluctuations of maximum run-up
204 line on the topographic slope for experiments with different slope angles of topographic slope
205 (Experiments S20°IN75°, S30°IN75° and S40°IN75°). H_{max} denotes the maximum height that
206 the dilute, upper part of the flow can run up on the slope surface. Red dashed line indicates the

207 maximum run-up line, colour-coded according to time. Here, a 15 s time window (1 s time
 208 interval) after the achievement of the maximum run-up point is chosen to demonstrate the
 209 fluctuations of the maximum run-up line. Blue dashed line represents the location of quasi-
 210 stable flow front.



211
 212 Fig. 3b. Representative front-view photographs depicting the geometry of the maximum run-
 213 up line on the topographic slope for experiments with different incidence angles of the current
 214 onto the slope (Experiments S30°IN75°, S30°IN45°, S30°IN30° and S30°IN15°). H_{max} denotes
 215 the maximum height that the dilute, upper part of the flow can run up on the slope surface.
 216
 217 respectively). A similar positive relationship is identified between maximum run-up height on
 218 slopes versus incidence angles of the current onto the slope ($H_{max} = 0.15 \sim 0.30$ m, ca. 1.4 to
 219 2.7 times the flow thickness for experiments of slope gradient of 20°; $H_{max} = 0.14 \sim 0.36$ m,
 220 ca. 1.3 to 3.3 times the flow thickness for experiments of slope gradient of 30°; $H_{max} = 0.16 \sim$
 221 0.30 m, ca. 1.5 to 2.7 times the flow thickness for experiments of slope gradient of 40°; **Fig.**
 222 **3I**). However, in Experiment S30°IN90°, the maximum run-up height on slopes reaches ca.
 223 0.28 m (ca. 2.5 times flow thickness), which is lower than that observed in experiments

224 S30°IN75°, S30°IN60° and S30°IN45° ($H_{max} = 0.36, 0.29, \text{ and } 0.29 \text{ m}$, respectively, ca. 3.3,
 225 2.6 and 2.6 times flow thickness).



226

227 Fig. 3c. (A-B) Plots of non-dimensional maximum run-up distance and height of the density
 228 currents on the barrier ramp versus the incidence angle of the current onto the slope. (C-D)
 229 Plots of non-dimensional maximum run-up distance and height on the barrier ramp versus the
 230 angle of the topographic slopes. In panel B, the predictive values and/or interval of maximum
 231 run-up elevation in frontal experimental setting (i.e., the flow incidence angle onto the slope is
 232 90°) based on previous models are indicated. The yellow and red stars represent the values
 233 predicted by Kneller and McCaffrey (1999) and Muck and Underwood (1990) methods,
 234 respectively. The purple range symbol is the 1.5 to 2.5 range, predicted by the model of
 235 Rottman et al. (1985).

236

237 *Variation of slope gradients*

238 Maximum run-up distance and height on slopes as a function of the slope angles of the
239 topographic slopes are examined for experiments with a single incidence angle of the current
240 onto the slope (**Fig. 3H, 3J**). An inverse relationship is seen between maximum run-up distance
241 on slopes versus slope gradient, i.e., a gentler topographic slope corresponds to a longer
242 maximum run-up distance on slopes (e.g., $L_{max} = 0.44 \sim 0.87$ m for experiments of incidence
243 angle of 90° ; $L_{max} = 0.38 \sim 0.59$ m for experiments of incidence angle of 60° ; $L_{max} = 0.25 \sim$
244 0.45 m for experiments of incidence angle of 15° ; **Fig. 3H**). The relationship between
245 maximum run-up height versus the gradient angle of the topographic slope is more complicated
246 (**Fig. 3J**). In an oblique experimental configuration with the same incidence angle of the flow
247 to the slope, the maximum run-up height for a slope of 30° is higher than that for a 20° slope,
248 which paradoxically occurs higher than a 40° slope. In a highly oblique and frontal
249 experimental configuration (e.g., Experiments IN 15° and IN 90°), only a slight difference is
250 documented in maximum run-up height across experiments with different slope gradients (e.g.,
251 $H_{max} = 0.15$ m, 0.14 m and 0.16 m, respectively in Experiment S 20° IN 15° , Experiment
252 S 30° IN 15° and Experiment S 40° IN 15° , ca. 1.3 ~ 1.5 times flow thickness). In a frontal
253 configuration, the maximum run-up height for a slope of 30° is marginally lower than that of a
254 slope of 20° and 40° ($H_{max} = 0.28$ m, ca. 2.5 times flow thickness in Experiment S 30° IN 90°
255 versus $H_{max} = 0.30$ m in Experiment S 20° IN 90° and Experiment S 40° IN 90° , ca. 2.7 times flow
256 thickness).

257 *Revisiting the existing paradigm of maximum run-up height estimation*

258 The large superelevation of the density currents observed in our laboratory experiments
259 compared to the existing predictive models on the maximum run-up height of sediment gravity
260 flow interaction with slopes (Rottman et al., 1985; Muck and Underwood, 1990; Lane-Serff et

261 al., 1995; Kneller and McCaffrey, 1999) challenge the validity of these commonly used
262 methods, especially Kneller and McCaffrey's (1999) method, into the 3D unconfined settings.
263 This might be ascribed to the fact that (i) the fluid parcel that reaches H_{max} is pushed forward
264 by the flow behind it (and the pressure gradient due to the density gradient between the saline
265 and ambient water); (ii) in an unconfined turbidity current setting, not only the downstream
266 velocity, but also the vertical and/or cross-stream velocity components contribute to the initial
267 kinetic energy, which therefore tends to transfer into higher potential energy and consequently
268 comparatively higher upslope run-up height.

269 Crucially, our experimental observations show that the maximum run-up height of turbidity
270 currents on slopes is a function of both slope gradient and flow incidence angle onto the
271 topographic slopes (**Fig. 3I and 3J**), which were not incorporated in the existing predictive
272 models. A noticeable decrease in maximum run-up height on slopes with a lower flow
273 incidence angle onto the slope (**Fig. 3I**) is mainly due to a decreased degree in topographic
274 containment, and reduced velocity component normal to the topography which could
275 contribute to run-up. However, our experiments herein also show an anonymously lower value
276 in H_{max} for Experiment S30°IN90° compared to its counterparts with incidence angles of 75°
277 and 60°. This might be attributed to the diverse dominant flow behaviour across experiments
278 with different slope gradients (further discussion on this is presented later). Based on
279 experimental work on unconfined turbidity currents interacting with orthogonal topography,
280 Keavney et al. (2024) highlighted that the dominant flow processes transition from laterally
281 divergence-dominated, through reflection-dominated, to deflection-dominated as the slope
282 gradient changes from 20° to 40°. Here, as the flow process is dominated by flow reflection at
283 the base of the slope in Experiment S30°IN90°, we interpret that much less the initial kinetic
284 energy is contributed to transfer into potential energy, leading to the anonymously low value
285 in H_{max} (**Fig. 3I**).

286 Without energy dissipation, the maximum run-up height would be independent of slope angle
287 (Allen, 1985; Pantin and Leeder, 1987; Simpson, 1987). Taking frictional heat loss and
288 turbulent dissipation into consideration, reducing the slope angle of the topography should lead
289 to a lower maximum run-up height because at a lower slope gradient a larger horizontal flow
290 distance is required to reach a given elevation (**Fig. 3H**), and in travelling this greater distance
291 more energy is dissipated. The lack of dependence of maximum run-up height from slope angle
292 in our frontal experiments (**Fig. 3J**) might be ascribed to a relatively short flow travel distance
293 on the slope and therefore a negligible variation in the effects of energy dissipation on the
294 ultimate run-up elevation. However, in the experimental results herein we found that in oblique
295 experimental configurations, H_{max} exhibited a significant dependence on slope gradient; when
296 the incidence angle of the current onto the slope is kept uniform and lying in the range from
297 30° to 85° , H_{max} for a 20° slope was consistently lower than that for a 40° slope, which in turn
298 was surprisingly lower than that for a 30° slope (**Fig. 3I**).

299 Another big difference between our experimental observations and the modelled prediction is
300 that pronounced lateral flow expansion (i.e., transverse to the flow direction) and flow
301 divergence phenomenon occurs in our 3D unconfined experiments (**Video S1**; Keavney et al.,
302 2024 and Wang et al., 2024). This lateral and diverging flow component is overlooked in
303 previous predictive models. It is hypothesised that the strength of the lateral flow expansion
304 and the resultant different levels of rugosities of the geometry of the maximum run-up line on
305 the slope surface along strike direction (**Supporting Information 3; Fig. 3**) would affect the
306 ultimate maximum run-up height. In oblique experimental configurations, the observed greater
307 value of H_{max} for a 30° slope compared to those for a 20° and 40° slope might be attributed to
308 the lower amount of lateral flow expansion for a 30° slope (Keavney et al., 2024 and Wang et
309 al., 2024) and a resultant higher rugosity in the geometry of the lateral maximum run-up line
310 on the slope surface along the strike direction, compared to its counterparts for a 20° and 40°

311 slope (**Supporting Information 3; Fig. 3**). A high rugosity in maximum run-up line geometry
312 on the slope surface tends to contribute to a higher H_{max} value than modelled predictions, on
313 the basis of 2D confined turbidity current settings. It appears that a fluid parcel H_{max} must
314 receive additional energy from the force of the flow behind it, and from the pressure gradient
315 at the boundary between the saline and ambient water, allowing it to reach a greater elevation
316 than the Kneller and McCaffrey (1999) approach predicts.

317 The above-mentioned information adds complexity to the existing paradigms and highlights
318 the need for updated predictive models that can capture these multidimensional interactions in
319 3D environments.

320

321 **Numerical modelling**

322 Our experimental data on observed maximum run-up height (**Fig. 3I**) challenges the application
323 of existing methods for estimating upslope run-up elevation when turbidity currents encounter
324 a frontal topographic slope in an unconfined setting. In this section, to address this issue, we
325 introduce a novel numerical model that incorporates the effects of slope gradient angle (θ) and
326 flow incidence angle (φ). The model is a further development of the Kneller and McCaffrey
327 (1999) approach based on energy balance principles, accounting for kinetic energy, potential
328 energy, work done by pressure, as well as frictional and turbulent dissipation (Allen, 1985; see
329 **Supporting Information 4** for the details on the derivation of the predictive model). Like the
330 Kneller-McCaffrey method, this model considers a fluid parcel at initial height z upon reaching
331 the slope, approximating the parcel as retaining its density and structure throughout its journey
332 up the ramp, effectively modelling it as a classical point particle. These broad approximations
333 circumvent the need to solve any nonlinear hydrodynamic equations.

334 For simplicity, we assume an initial velocity $\vec{U} = (u, 0, 0)$ for each parcel of fluid meeting the
 335 ramp, with the x , y and z axes aligned with the downstream, cross-stream and vertical
 336 directions, respectively (**Fig. 2A**). The flow velocity u_z is averaged over all horizontal
 337 locations, with the subscript denoting its remaining dependence on vertical position z . The
 338 predicted run-up height for a fluid parcel at initial height (see **Supporting Information 4** for
 339 the details on the derivation of the predictive model) is

$$340 \quad h_{max}(z) = z + \frac{\frac{1}{2}\rho_z u_z^2 \sin^2 \varphi (\cos^2 \theta + S \sin^2 \theta) + \Delta E_{gain}}{\Delta \rho_z g + \frac{F_{ave}}{\sin \alpha}} \quad (1)$$

341 with, again, the z subscripts referring to the density and velocity at height z . In the above
 342 equation, ΔE_{gain} is the energy gained from internal pressure and interactions of the fluid parcel
 343 with neighbouring fluid parcels; S is a dimensionless collision factor ranging from 0 to 1,
 344 characterising the fraction of kinetic energy associated with the normal component of the initial
 345 velocity that contributes to H_{max} ; F_{ave} is the average dissipative force per unit volume, acting
 346 in the direction opposed to the fluid parcel's velocity; and the angle $\alpha =$
 347 $\tan^{-1}(\sin \varphi \tan \theta)$ represents the 'effective slope' of the ramp in the downstream (x) direction
 348 in the vertical (x, z) plane (**Fig. 4D**).

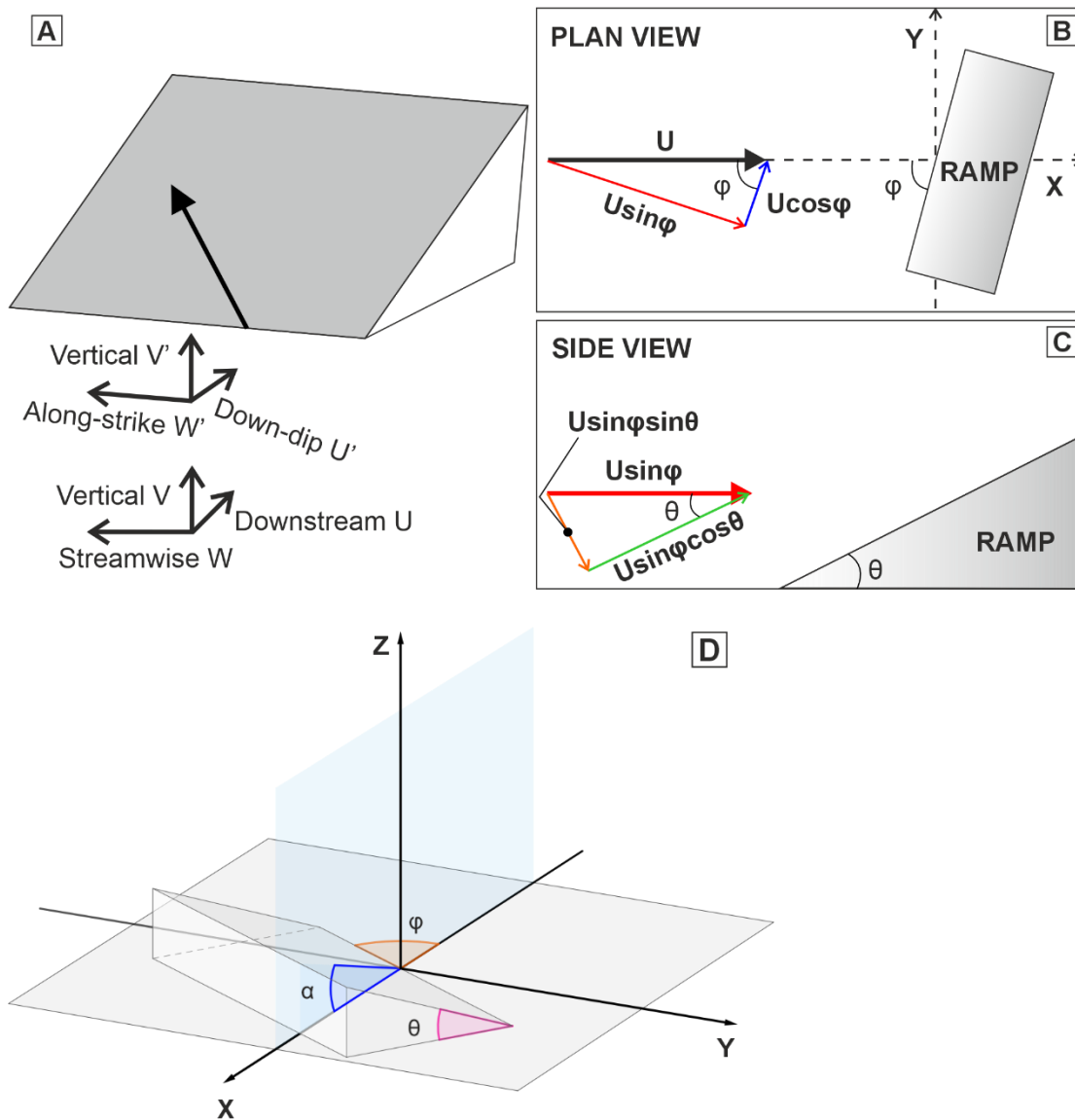
349 The overall H_{max} is the maximum h_{max} for all fluid parcels, and thus occurs when

$$350 \quad \frac{d}{dz} \left(z + \frac{\frac{1}{2}\rho_z u_z^2 \sin^2 \varphi (\cos^2 \theta + S \sin^2 \theta) + \Delta E_{gain}}{\Delta \rho_z g + \frac{F_{ave}}{\sin \alpha}} \right) = 0 \quad (2)$$

351 To facilitate comparison to natural turbidity currents in the field, we normalise $h_{max}(z)$ by the
 352 flow thickness of the current body h and therefore Equation 1 changes into:

$$353 \quad h_{max}(z)/h = z/h + \frac{\frac{1}{2}\rho_z u_z^2 \sin^2 \varphi (\cos^2 \theta + S \sin^2 \theta) + \Delta E_{gain}}{(\Delta \rho_z g + \frac{F_{ave}}{\sin \alpha}) h} \quad (3)$$

354



355

356

357 Fig. 4. Model conceptualisation of the numerical modelling work. (A) Model
 358 conceptualisation, showing an unconfined turbidity current interacts with topographic slope
 359 with a specific slope gradient and flow incidence angle. (B-C) Definition sketch illustrating the
 360 trigonometric relationships between the decomposed components of the initial velocity U in
 361 plan-view (B) and side view (C). (D) Schematic diagram demonstrating the 'effective slope'
 362 (α) between the flow path and the horizontal (x, z) plane, which is dependent on the flow
 363 incidence angle against the ramp and slope gradient. Here, $\sin \alpha = \sin(\tan^{-1}(\sin \phi \tan \theta))$.

364

365 Note that the collision factor S , average dissipative force F_{ave} and energy gain from the
366 surrounding fluid ΔE_{gain} are at this stage unknown variables, each requiring their own
367 estimation, and are likely themselves to depend on the initial velocity, density and the angles
368 φ and θ . However, in the present paper they will be approximated at zeroth order and treated
369 as constant parameters. Finding more realistic estimates of these three unknown variables and
370 their dependences on the initial parameters will be the subject of future research. In a relaxed
371 way, F_{ave} can be approximated as $F_{ave} = \mu N$, where μ is the frictional coefficient and N is the
372 normal contact force from the ramp, which should be equal to the component of the weight
373 normal to the ramp: $N = \rho g' \cos \theta$. S is a dimensionless collision factor and determined by the
374 properties of the inlet flow and the material of the slope surface.

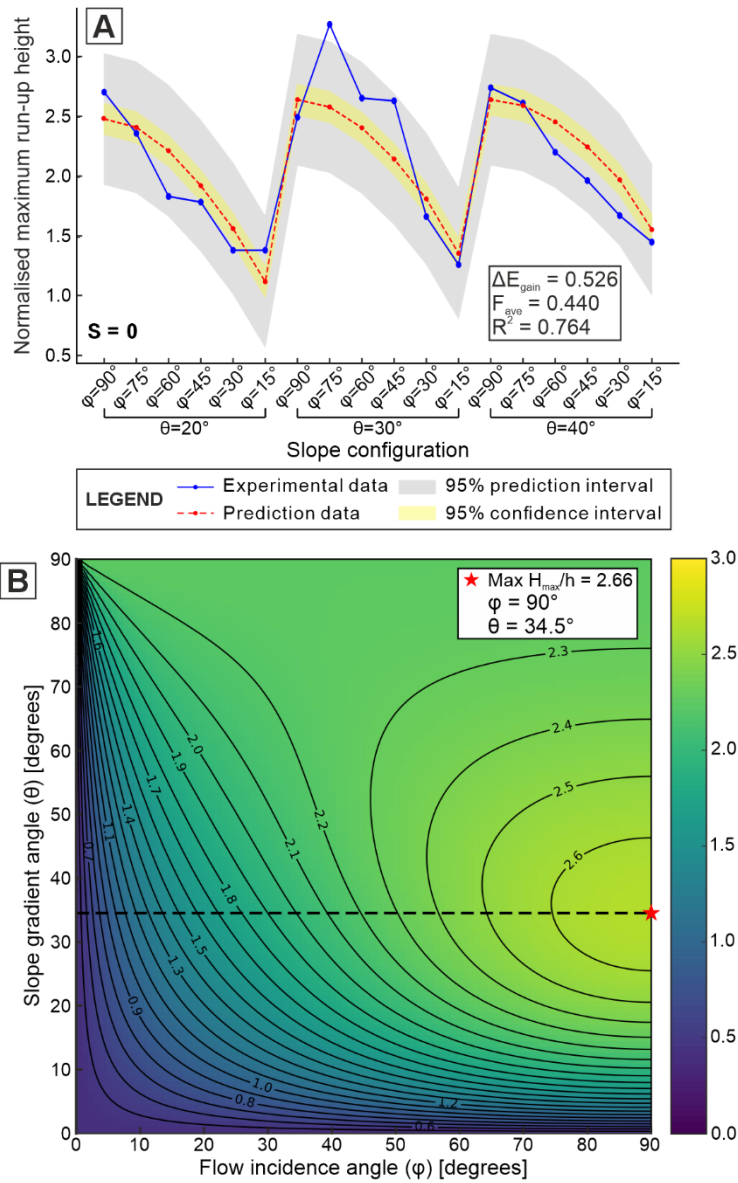
375

376 *Comparison of the numerical-model predictions with the observed experimental data*

377 To test the validity of the numerical model above, its general run-up height predictions are
378 compared to the observed values for each of the 18 ramp experiments herein, with the aim of
379 approximating the overall dependence of (normalised) maximum run-up height on flow
380 incidence angle and slope gradient. Due to the broad approximations made in the model and
381 the turbulent nature of the flow ($Re \approx 3000$), only an approximate fit to the data is to be
382 expected. To be fully realistic would require a Computational Fluid Dynamics simulation, but
383 the purpose here is to provide a method of estimation that can be calculated quickly for practical
384 purposes. Values of the input quantities representative of those measured in the current physical
385 experiments in the first 5 s after the current head were substituted into the model ($z = 0.045$ m,
386 $\rho_z = 999.8$ kg/m³, $\Delta\rho_z = 0.22$ kg/m³, $h = 0.11$ m, $u_z = 0.0243$ m/s) as it is the dilute head of the
387 density current that appears to mainly contribute to the maximum value of the run-up elevation
388 over the course of the experiment (**Video S1; Fig. S2**). However, accurate values for the energy

389 gain ΔE_{gain} , averaged dissipative force F_{ave} and collision factor S were not available from the
390 data gathered in the physical experiments. Here, for simplicity S is assumed to be 0, i.e., none
391 of the kinetic energy associated with the component of the velocity normal to the ramp was
392 converted into gravitational potential energy, while optimised values for the remaining two
393 parameters giving the best fit with the observed experimental data points were found to
394 be $\Delta E_{gain} = 0.526 \text{ Jm}^{-3}$ and $F_{ave} = 0.440 \text{ Nm}^{-3}$ (**Fig. 5A**). A contour map of the modelled
395 normalised H_{max}/h , as a function of the flow incidence angle onto the slope (φ) and the angle
396 of slope gradient (θ), using the input values above is given in **Figure 5B**.

397 Overall, our numerical model captured the first-order dependence of normalised maximum run-
398 up height as a function of flow incidence angle and slope gradient (**Fig. 5**), with an R^2 value
399 of 0.764. A critical slope gradient θ exists ($\theta = 34.5^\circ$), where H_{max}/h reaches its maximum
400 value of 2.66. This is approximately consistent with the experimental observations of higher
401 H_{max}/h for a slope of 30° compared to that of 20° and 40° in an oblique setting (**Fig. 3J**).
402 Additionally, when the slope gradient is set to a constant, the normalised maximum run-up
403 height increases with a higher flow incidence angle, consistent with the observed positive
404 relationship between values H_{max}/h versus the flow incidence angle in our physical
405 experiments (**Fig. 3I**).



406

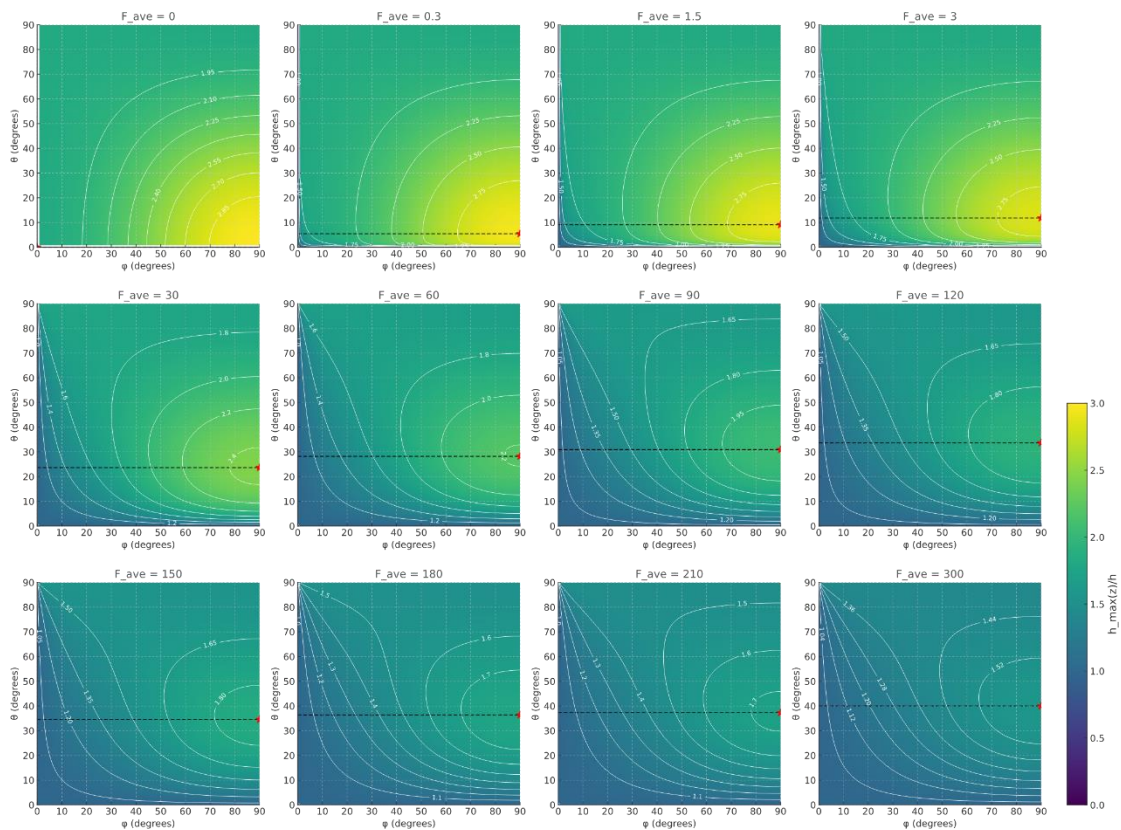
407 Fig. 5. (A) Comparison between observed and predicted values of normalised maximum run-up
 408 up height upslope for our 18 ramp experiments. The best-fit values of ΔE_{gain} and F_{ave} in
 409 Equation 3 optimised for the observed experimental data points and the fit accuracy (R^2) are
 410 given in the bottom right box. The input quantities in the model are set to constant,
 411 representative for the current physical experiments in the first 5 s after the current head ($z =$
 412 0.045 m, $\rho_z = 999.8$ kg/m³, $\Delta\rho_z = 0.22$ kg/m³, $h = 0.11$ m, $u = 0.0243$ m/s). (B) Contour
 413 map of modelled normalised maximum run-up height, H_{max}/h , as a function of the flow
 414 incidence angle onto the slope (ϕ) and slope gradient (θ), with the input variables set to
 415 constant values typical of current physical experiments and the optimised ΔE_{gain} and F_{ave} .

416 *Evaluation of numerical model by field data*

417 To simulate the normalised maximum run-up height, we assume that the turbidity current is
418 relatively dilute ($\rho_s = 1,060 \text{ kg/m}^3$; $\Delta\rho_s = 30 \text{ kg/m}^3$) and has an initial downstream velocity of
419 5 m/s and flow height of 39 m. The energy gain from the internal pressure of the nearby fluid
420 parcels is poorly known, $\Delta E_{gain} = 9600 \text{ Jm}^{-3}$, is chosen as it is tested to yield an approximately
421 realistic output of (normalised) maximum run-up height. The averaged dissipative force can be
422 approximated as $F_{ave} = \mu N$, where μ is the coefficient of friction and N is the normal contact
423 force from the ramp, which should be equal to the component of the weight normal to the ramp:
424 $N = \rho g' \cos \theta$. Here F_{ave} varies from 0 to 300 Nm^{-3} , approximately corresponding to the case
425 whereby the frictional coefficient $\mu = 0, 0.001, 0.005$ (base-case), 0.01, 0.1, 0.2, 0.3, 0.4, 0.5,
426 0.6, 0.7 and 1, respectively. These input quantities are chosen arbitrarily but ensuring that they
427 are within ranges of observations from field-scale turbidity currents (e.g., Sinclair, 2000;
428 Mohrig and Buttles, 2007; Symons et al., 2017; Azpiroz-Zabala et al., 2017; Straub et al., 2008;
429 Lamb et al., 2008).

430 We first conducted sensitivity analysis to explore the effect of different variables incorporated
431 in Equation 3 on the normalised maximum run-up height for a specific topographic
432 configuration (take $\theta = 45^\circ$ and $\varphi = 90^\circ$ for an example) with the above-mentioned input
433 variables set as a base case and S equal to 0.5 (**Fig. S4**). Results indicate that initial downstream
434 flow velocity U and excess density difference $\Delta\rho_s/\rho_s$ are the most influential factors affecting
435 H_{max}/h , collision factor S and energy gain from internal pressure of nearby fluid parcels
436 ΔE_{gain} are moderately sensitive while the averaged dissipative force F_{ave} has the least impact.
437 An inverse relationship is identified between excess density difference or averaged dissipative
438 force F_{ave} versus H_{max}/h , as one would expect, whereas a positive relationship is seen
439 between other input parameters versus H_{max}/h .

440 We then explored the effect of the flow incidence angle onto the slope (φ) and the angle of
 441 slope gradient (θ) on (normalised) maximum run-up height with covarying averaged
 442 dissipative force F_{ave} (**Fig. 6**). Taking $F_{ave} = 3 \text{ Nm}^{-3}$ for example, results indicate that an
 443 increase in flow incidence angle with the same slope gradient notably contributes to a higher
 444 H_{max}/h . This is because higher incidence angles correspond to better alignment between the
 445 average flow velocity and the updip direction. However, the impact of slope gradient is more
 446 complicated. A critical angle of slope gradient exists where the normalised H_{max}/h achieves
 447 its maximum. For a given incidence angle, increasing the slope gradient will first lead to an
 448 increase in the normalised H_{max}/h , which is followed by an ultimate decrease.



449
 450 Fig. 6. Numerical model results for the normalised maximum run-up height of turbidity
 451 currents interacting with a topographic slope with varying averaged dissipative force F_{ave} . In
 452 each panel map, contours of normalised maximum run-up height, H_{max}/h , as a function of the
 453 flow incidence angle onto the slope (φ) and the angle of slope (θ), with other variables set to

454 constant values typical of field-scale turbidity currents ($z = 39$ m, $\rho_z = 1,060$ kg/m³; $\Delta\rho_z = 30$
455 kg/m³; $h = 39$ m; $u_z = 5$ m/s). For all panel maps, $S = 0$ assuming a maximal collision model
456 and $\Delta E_{gain} = 9600$ Jm⁻³. Across the panel maps, the given value of the averaged dissipative
457 force F_{ave} varies from 0 to 300 Nm⁻³, approximately corresponding to the case whereby the
458 frictional coefficient $\mu = 0, 0.001, 0.005, 0.01, 0.1, 0.2, 0.3, 0.4, 0.5, 0.6, 0.7$ and 1,
459 respectively. Red star denotes the position whereby H_{max}/h reaches its maximum value for a
460 specific topographic configuration with a critical angle of θ and φ .

461

462 **DISCUSSION**

463 **Comparison with existing numerical models**

464 Our experimental results clearly demonstrate that the existing predictive models, including
465 those by Kneller and McCaffrey (1999) and Muck and Underwood (1990), based on simplified
466 2D settings and orthogonal interactions, underestimate the run-up height for unconfined
467 turbidity currents (**Fig. 3I**). The existing models neglects key factors in 3D unconfined settings,
468 such as lateral flow expansion and divergence on the slope surface, vertical and cross-stream
469 velocity components' contribution to initial kinetic energy and the energy gain from the internal
470 pressure of nearby fluid parcels. Crucially, our experimental data show that maximum run-up
471 heights are influenced not just by the gradient of the topographic slope but also by the angle of
472 incidence of the flow against the slope (see subsection below). These factors are integral to the
473 behaviour of 3D unconfined turbidity currents in natural submarine environments.

474

475 **Influence of slope gradient and flow incidence angle on the magnitude of maximum run-**
476 **up height**

477 A key finding from our laboratory experiments is the nonlinear relationship between slope
478 gradient and maximum run-up height (**Fig. 3**). While conventional wisdom suggests that
479 steeper slopes should yield higher run-up heights, we observed that intermediate slopes (around
480 30°) often exhibited the highest run-up heights compared to both gentler and steeper slopes,
481 particularly in oblique flow configurations. This is further supported by our numerical model
482 predictions (**Fig. 5-6**), which reveal a critical slope gradient θ_c for non-zero average dissipative
483 force F_{ave} , with θ_c increasing with rising F_{ave} (**Fig. 6**). This complicated θ dependence is
484 ascribed to the competition between the following opposite effects: (1) a greater θ means less
485 alignment between the average flow direction and the up-dip direction, lowering the run-up
486 height (**Fig. 4C**); (2) a greater θ also means less overall distance to travel on the slope surface
487 to achieve the same vertical run-up height, which means less energy lost to friction or turbulent
488 dissipation, increasing the run-up height. In the regime $\theta < \theta_c$ with the same flow incidence
489 angle relative to the slope, the influence of the average dissipative force dominates and thus a
490 steeper slope gradient is associated with higher H_{max}/h . In the regime $\theta > \theta_c$ with the same
491 flow incidence angle relative the slope, the influence of the collision factor dominates and thus
492 a steeper slope gradient is associated with lower H_{max}/h .

493 The flow incidence angle impacts the maximum run-up height markedly. Notably more oblique
494 flow interactions with the topographic slope (e.g., 15° – 45° incidence angles) tend to have lower
495 maximum run-up heights due to a lower degree in topographic containment and reduced up-
496 dip velocity component, while near-perpendicular interactions (e.g., 75° – 90°) allow more
497 initial kinetic energy to be converted into gravitational potential energy. Importantly,
498 experimental results (**Fig. 3I**) indicate that occasionally, a critical flow incidence angle φ_c
499 exists near the frontal setting, that is, an incidence angle less than 90° at which there is a

500 pronounced boost in superelevation, leading to the greatest value of all the vertical run-ups.
501 This is likely a consequence of variations in the cross-stream velocity component, which
502 effectively change the local incidence angle of the fluid parcel relative to the ramp, locally
503 increasing the incidence angle by a shift $\Delta\varphi$ so that its maximum occurs when $\varphi + \Delta\varphi = 90^\circ$,
504 instead of when $\varphi = 90^\circ$ (the value at which the H_{max}/h would usually reach its maximum).
505 With sufficient data, a more sophisticated version of the predictive model could take into
506 account the cross-stream velocity component and its lateral profile, to facilitate a prediction of
507 this shift and the resulting critical angle φ_c .

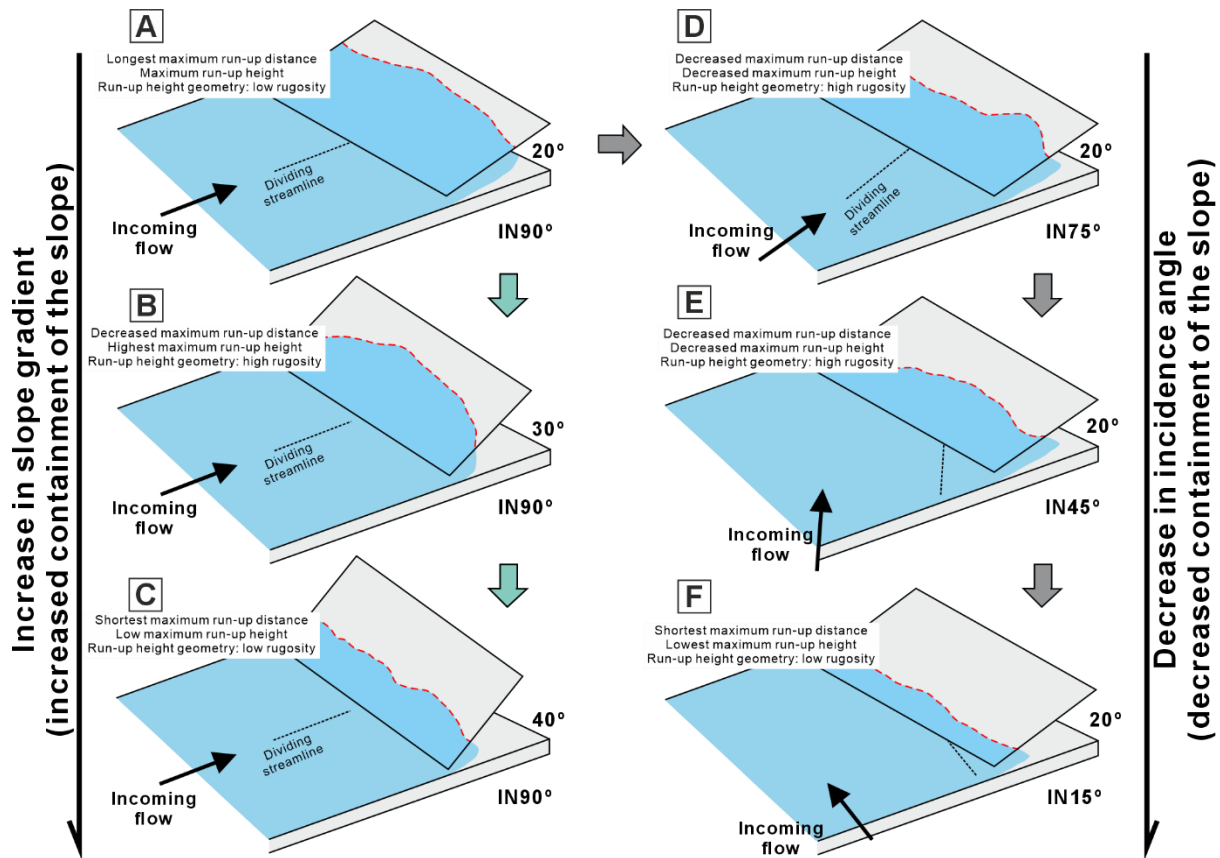
508

509 **Implications for the stratigraphic record**

510 Our results can be used to inform the position of deposition along intrabasinal slopes, and the
511 style of onlap, in the ancient rock record. A lower flow incidence angle onto the slope leads to
512 a lower maximum run-up distance, and an initial increase and subsequently a decrease in the
513 rugosity of the maximum run-up line. A steeper slope results in a shorter maximum run-up
514 distance.

515 Superelevation of turbidity currents and deposition higher on topographic slopes have been
516 recognized in laboratory (e.g., Muck and Underwood, 1990; Soutter et al., 2021; Keavney et
517 al., 2024), field (e.g., Damuth and Embley, 1979; Cita et al., 1984; Dolan et al., 1989) and
518 outcrop investigations (e.g., Al A’Jaidi et al. 2004; Soutter et al., 2019). Based on experimental
519 observations of unconfined turbidity currents interacting with orthogonal topography, Keavney
520 et al. (2024) pointed out that once the flow encounters the topography, the initial flow
521 decouples: a basal dense region and an upper dilute region. The basal dense part of the flow
522 decelerates quickly at the base of the slope and leads to coarse-grained sediment deposition
523 lower on the slope, and therefore contributes to abrupt pinchouts. The upper dilute part would

524 travel further and deposit finer-grained sediments higher on slopes, and therefore contributes
 525 to draping pinchouts. Here, our results support this model and further expand on the
 526 characteristics of run-up elevation on slopes, including the magnitude of run-up height and
 527 lateral variability in strike direction (**Fig. 7**), which would provide key insights into the 3D
 528 stratal onlap termination styles in the ancient rock record.



529
 530 Fig. 7. Schematic diagram illustrating the characteristics of maximum run-up height potential
 531 of turbidity currents that interact with different configurations of topographic slopes, including
 532 incidence angle of the flow onto the slope (A, D-F) and slope gradient (A-C). The red dashed
 533 line indicates the outline of the run-up line on the slope surface. The red filled circle denotes
 534 the position of the maximum run-up point.

535
 536 In a low-gradient and nearly frontal intrabasinal slope, the onlap style in dip section is
 537 consistent with the model proposed by Keavney et al. (2024). The upper deposit limit of finer-

538 grained sediments is evenly distributed. In a low-gradient and oblique intrabasinal slope, the
539 upper limit of finer-grained sediments is more laterally variable, but the highest point on slope
540 surface is lower on slopes in a dip direction. In a low-gradient and nearly parallel intrabasinal
541 slope, weak flow stripping would lead to the deposition of a limited zone of draped fines, which
542 abruptly terminates lower on the slope. The upper limit line of the abrupt pinch out in strike
543 direction would exhibit minimal lateral variability. In a steep-gradient and nearly frontal
544 intrabasinal slope, strong topographic containment would lead to a rapid deceleration of the
545 flow. This could lead to the deposition of thick coarser-grained sediments, abruptly terminating
546 lower on slopes, and evenly distributed in a strike direction. Crucially, the nonlinear
547 relationship between slope gradients and flow incidence angles versus maximum run-up
548 heights, i.e., critical slope gradients and flow incidence angles exist which can generate the
549 highest run-up heights, means that a specific topographic configuration exists where the upper
550 limit of the finer-grained sediment reaches its maximum.

551 The depositional model herein suggests that distinct 3D onlap styles on slopes correspond to
552 different topographic configurations, which can be used to reconstruct the orientation and slope
553 gradient of the topographic slopes in the modern field and ancient rock record. Notably, the
554 maximum run-up distance is a good indicator of topographic configurations; however, the
555 maximum run-up elevation and the geometry of the run-up line do not exhibit the same level
556 of indicative reliability.

557

558 **Implications for hazard management in natural submarine systems**

559 These findings have important implications for predicting sediment transport and deposition
560 patterns in natural submarine environments. The ability of turbidity currents to climb
561 topographic barriers and deposit material at elevated locations is critical for understanding the

562 distribution of sediments, microplastics, and pollutants on the seafloor. Our results suggest that
563 in regions with varied topography, sediment deposition could occur at higher elevations than
564 previously anticipated, especially when turbidity currents interact with slopes at specific
565 incidence angles and slope gradients.

566 In the context of deep-sea infrastructure, such as pipelines and communication cables, our
567 findings raise concerns about the potential for greater-than-expected sediment deposition on
568 elevated terrain, which could pose a hazard to these structures. Understanding the dynamics of
569 turbidity current run-up and deposition is therefore crucial for risk assessment and mitigation
570 strategies in such environments.

571

572 **CONCLUSIONS**

573 This study advances our understanding of the characteristics of maximum run-up height
574 potential (with a focus on the magnitude) of turbidity currents interacting with topographic
575 slopes with varying slope gradients and flow incidence angles onto the slope, in unconfined,
576 3D settings. Our experimental results show that existing predictive models based on 2D
577 confined flows and frontal topographic configurations markedly underestimate the run-up
578 heights of turbidity currents, highlighting the importance of considering lateral flow expansion
579 and divergence, vertical and/or downstream velocity components and the energy gain from the
580 internal pressure of nearby fluid parcels. Experimental results also highlight the importance of
581 slope gradient and flow incidence angle in controlling the magnitude of maximum run-up
582 height, revealing that intermediate slope gradients (ca. 30°) and (near-)perpendicular flow
583 incidence angles generate the highest run-up heights. Our newly developed numerical model
584 captures the key dynamics of turbidity current interaction with topography in 3D, unconfined
585 settings and provides relatively more accurate predictive framework for run-up heights. These

586 findings are critical for improving sediment transport models, predicting the distribution of
587 sediments, pollutants, and organic carbon in deep-sea environments, assessing seafloor
588 geohazards, and reconstructing ancient deep-water basin palaeogeographies.

589

590 **MATERIALS AND METHODS**

591 **Experimental design and data collection**

592 Details and video of our experiments, performed at the Sorby Environmental Fluid Dynamics
593 Laboratory, University of Leeds, using a large flume tank (10 m long, 2.5 m wide and 1 m deep;
594 **Fig. 2A**) are presented in the **Supporting Information 1** and are summarized here. The tank
595 configuration mirrored previous studies (e.g., Keavney et al., 2024; Wang et al., 2024) and
596 included a 1.8 m long straight input channel section centred at the upstream end of the main
597 tank and a flat basin floor (**Fig. 2A**). Nineteen experiments were performed in total: the first,
598 an unconfined experiment, served as a base case for scaling, while the other eighteen involved
599 a non-erodible smooth planar ramp (1.5 m wide, 1.2 m long) centrally placed in the tank with
600 the ramp's leading edge positioned 3 m downstream from the channel mouth (**Fig. 2A**). Each
601 ramp experiment used a different combination of ramp slope gradient (20°, 30° and 40°) and
602 incidence angle relative to the incoming flow (90°, 75°, 60°, 45°, 30° and 15°; **Fig. 2B**). The
603 tank was filled up to 0.6 m water level with fresh tap water prior to each experiment. During
604 each experimental run, a saline solution of excess density 2.5% (1,025 kg m⁻³) was pumped at
605 a constant discharge rate of 3.6 L s⁻¹ from the mixing tank (**Fig. 2A**). This setup could better
606 constrain the flow thickness, vertical velocity profile and concentration profile of the density
607 current at the base of the barrier ramp (**Fig. 2C**) and hence ensured subcritical, fully turbulent
608 flow conditions (Densimetric Froude number $Fr_d = 0.50$; Reynolds number $Re = 3203$; see
609 details in Keavney et al., 2024 and Wang et al., 2024) at the base of the barrier ramp. This can
610 better approximate basin-floor flows in the field which have passed through the channel-lobe

611 transition zone, experiencing a loss in flow confinement (Komar, 1971; Hodgson et al., 2022;
612 Keavney et al., 2024). Each experimental run lasted 130 seconds in total.

613 In the unconfined experiment, velocity and density profiles over height were recorded for flows
614 at 3 m downstream from the channel mouth along the channel-basin centreline (i.e., the position
615 of the central base of the barrier ramp in subsequent ramp experiments; **Fig. 2A**). These were
616 achieved through the measurements by an Ultrasonic Velocity Profiler (UVP), Acoustic
617 Doppler Velocimeter (ADV) and siphoning system, respectively (see Keavney et al., 2024 and
618 Wang et al., 2024 for details of the UVP, ADV and siphon set-ups; **Fig. S1**). Time-averaged
619 UVP downstream velocity profile and density profile for the flow body at this point (**Fig. 2C-**
620 **D**) were obtained by averaging measurements over 30 seconds, starting 5 seconds after the
621 current head passed. In the ramp experiments, Pliolite and a small amount of white paint were
622 added to the inlet flow to better visualise the internal fluid motion within the current (cf.
623 Edwards et al., 1994), while fluorescent yellow dye was injected via a series of tubes mounted
624 from the rear of the ramp and flush with its surface, to aid in the visualisation of the density
625 current interacting with the ramp. We used four high-resolution video cameras (GoPro HERO
626 10; GoPro Inc., USA) to record the flow process in each ramp experiment and finally captured
627 the maximum run-up elevation and the outlines of the maximum run-up geometry on the slope
628 surface from the video stills.

629 **Numerical simulations**

630 To better capture the multidirectional flow-topographic slope interactions in 3D, unconfined
631 settings observed in our physical experiments, we developed a novel numerical model
632 (**Supporting Information 4**) that incorporates the effects of slope gradient angle (θ) and flow
633 incidence angle (φ). The model is also a further development of the Kneller and McCaffrey
634 (1999)'s approach based on energy balance principles, accounting for kinetic energy, potential

635 energy, work done by internal pressure from nearby fluid parcels, as well as frictional and
636 turbulent dissipation.

637

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644 for providing some inspirations for the designing of the analytical model shown in this work.

645

646 **NOMENCLATURE**

647 *E*: Frictional coefficient

648 *Fr*: Froude number

649 *Fr_d*: Densimetric Froude number

650 *g*: Acceleration due to gravity (m s^{-2})

651 *H_{max}*: Maximum run-up height on slopes (m)

652 *h_{max}*: Maximum run-up height on slopes for a specific parcel of the fluid (m)

653 *h*: Flow height (m)

654 *h_p*: Height of the maximum downstream velocity above the basin floor (m)

655 *L_{max}*: Maximum run-up distance on slopes (m)

656 *Re*: Reynolds number

657 *U*: Mean depth-averaged downstream velocity (m s^{-1})

658 *u_p*: Maximum downstream velocity (m s^{-1})

659 u_z : Velocity component at initial height z that is normal to the topography (m s^{-1})
660 z : Initial height of a specific parcel of the fluid (m)
661 ρ_s : Mean depth-averaged density of the current (kg m^{-3})
662 $\Delta\rho_z$: Density difference between the fluid at initial height z and the ambient fluid (kg m^{-3})

663

664 **DATA AVAILABILITY STATEMENT**

665 The data that support the findings of this study are available from the corresponding author
666 upon reasonable request.

667

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782

783 **SUPPLEMENTARY TEXT**

784 **Supporting Information 1: Physical experiments**

785 Experiments were carried out in the Sorby Environmental Fluid Dynamics Laboratory,
786 University of Leeds, using a large flume tank (10 m long, 2.5 m wide and 1 m deep; **Fig. 2A**).
787 The tank configuration mirrored previous studies (e.g., Keavney et al., 2024; Wang et al., 2024)
788 and included a 1.8 m long straight input channel section centred at the upstream end of the
789 main tank and a flat basin floor (**Fig. 2A**). Nineteen experiments were performed in total: the
790 first, an unconfined experiment, served as a base case for scaling, while the other eighteen
791 involved a non-erodible smooth planar ramp (1.5 m wide, 1.2 m long) centrally placed in the
792 tank with the ramp's leading edge positioned 3 m downstream from the channel mouth (**Fig.**
793 **2A**). Each ramp experiment utilised a different combination of incidence angle relative to the

794 incoming flow (i.e., 90°, 75°, 60°, 45°, 30° and 15°; **Fig. 2B**) and ramp slope gradient (i.e., 20°,
795 30° and 40°).

796 The tank was filled up to 0.6 m water level with fresh tap water prior to each experiment.
797 During each experimental run, a saline solution of excess density 2.5% (1,025 kg m⁻³) was
798 pumped at a constant discharge rate of 3.6 L s⁻¹, flowing from the mixing tank where the saline
799 solution was mixed, through the straight channel section onto the basin floor. This setup tightly
800 controlled the flow thickness, vertical velocity profile and concentration profile of the density
801 current at the base of the barrier ramp (**Fig. 2C**) and hence ensured subcritical, fully turbulent
802 flow conditions (Densimetric Froude number $Fr_d = 0.50$; Reynolds number $Re = 3203$; see
803 details in Keavney et al., 2024 and Wang et al., 2024) at the base of the barrier ramp, in order
804 to better approximate basin-floor flows in the field which have passed through the channel-
805 lobe transition zone, experiencing a loss in flow confinement (Komar, 1971; Hodgson et al.,
806 2022). Each experimental run lasted 130 seconds in total.

807 In the unconfined experiment, the inlet flow was dyed purple to aid flow visualisation. Velocity
808 and density profiles over height were recorded for flows at 3 m downstream from the channel
809 mouth along the channel-basin centreline (i.e., the position of the central base of the barrier
810 ramp in subsequent experiments; **Fig. 2A**). These were achieved through the measurements by
811 the Ultrasonic Velocity Profiler (UVP), Acoustic Doppler Velocimeter (ADV) and siphoning
812 system, respectively (see Keavney et al., 2024 and Wang et al., 2024 for details of the UVP,
813 ADV and siphon set-ups; **Fig. S1**). Time-averaged UVP downstream velocity profile and
814 density profile for the flow body at this point (**Fig. 2C**) were obtained by averaging
815 measurements over 30 seconds, starting 5 seconds after the current head passed. In the ramp
816 experiments, Pliolite, a low density and highly reflective polymer (subspherical, mean grain
817 size of 1.5 mm, density of 1050 kg m⁻³), and a small amount of white paint were added to the
818 inlet flow to better visualise the internal fluid motion within the current (cf. Edwards et al.,

819 1994). Fluorescent yellow dye was injected via a series of tubes mounted from the rear of the
820 ramp and flush with its surface, to aid the visualisation of the density current interacting with
821 the barrier ramp. Each ramp experiment was recorded using up to four high-resolution video
822 cameras (GoPro HERO 10; GoPro Inc., USA) to capture front, side, and top views of the
823 experiment. For each ramp experiment, measurements of the maximum run-up elevation and
824 the outline of the run-up geometry on the barrier ramp were made from the video stills.

825

826 **Supporting Information 2: Details on the estimation of maximum run-up height based on** 827 **the Kneller and McCaffrey (1990) model**

828 Based on the unconfined control experiment, the maximum run-up height H_{max} of the
829 experimental flows modelled by the Kneller and McCaffrey (1990) method were estimated
830 using the time-averaged downstream velocity profile of UVP and density profile of the
831 experimental density currents at 3 m downstream from the channel mouth along the channel-
832 basin centreline for the first 5 s after the current head. The first 5 s time window was chosen as
833 it is the dilute head of the density current that appears to mainly contribute to the maximum
834 value of the run-up elevation over the course of the experiment (**Video S1; Fig. S2**). Assuming
835 no frictional energy loss and that the flow strikes the topographic slope with a right incidence
836 angle, results indicate an estimated maximum run-up height of the experimental flows of ca.
837 0.23 m on the barrier ramp, ca. 2.1 times flow thickness (**Fig. 2C**).

838

839 **Supporting Information 3: Geometry of maximum run-up line on slopes in the ramp** 840 **experiments**

841 In contrast to previous studies, these unconfined experiments permit the geometry of the flow
842 front at the maximum run-up height to be documented using video stills, and how this geometry
843 changes through time.

844 *Variation of incidence angles of the current onto the slope*

845 Characteristics on the geometry of the maximum run-up line on slopes as a function of the
846 incidence angle onto the slope are examined for experiments S30°IN90°, S30°IN75°,
847 S30°IN45°, S30°IN30° and S30°IN15° (**Fig. 3B, D-F**). Notably, a lower incidence angle
848 experimental configuration leads to an initial increase in the rugosity of the maximum run-up
849 line on slopes and subsequently a decrease in the rugosity.

850 *Variation of slope gradients*

851 Characteristics on the geometry of the maximum run-up line on slopes as a function of the
852 slope angle of the topographic slope are examined for experiments S20°IN75°, S30°IN75° and
853 S40°IN75° (**Fig. 3A-C**). Notably, the maximum run-up line on the topographic slope for
854 experiment S20°IN75° exhibits no pronounced lateral variability across the slope. In
855 Experiment S30°IN75°, the maximum run-up point resides laterally at ca. 0.37 m away from
856 the right edge of the ramp and the maximum run-up line on the slope displays a high rugosity.
857 Comparatively, the rugosity of the maximum run-up line on a slope of 30° is higher than for
858 20° and 40° slopes.

859 *Fluctuations of maximum run-up line on slopes*

860 To characterise the fluctuations of maximum run-up line on slopes, we tracked and outlined
861 the run-up line on slopes using a 15 s time window (1 s time interval) after reaching the
862 maximum run-up height for experiments S20°IN75°, S30°IN75° and S40°IN75° (**Fig. 3A-C**).
863 Results indicate that notably, the maximum run-up line on the topographic slope is not stable
864 and fluctuates both vertically and laterally. For instance, in Experiment S40°IN75°, the

865 maximum run-up line can travel up to 0.5 m distance on the slope on the left edge of the ramp
866 (Fig. 3C).

867

868 **Supporting Information 4: Derivation of the numerical model for the estimation of** 869 **maximum run-up height in an unconfined setting**

870 Our experimental data on observed maximum run-up height (Fig. 3I) challenges the application
871 of existing methods for estimating upslope run-up elevation when turbidity currents encounter
872 a frontal topographic slope in an unconfined setting. In this section, we address this issue and
873 introduce a novel numerical model that also incorporates the effects of slope gradient (θ) and
874 flow incidence angle (φ). The model is based on energy balance principles, accounting for
875 kinetic energy, potential energy, work done by pressure, and frictional heat loss in the
876 sedimentary system (Allen, 1985). Between first meeting the ramp and reaching its maximum
877 height, the energy balance equation for a fluid parcel (per unit volume) is expressed as:

$$878 \quad KE_{initial} + \Delta E_{gain} = KE_{final} + \rho g' \Delta z + \Delta E_{loss} \quad (S1)$$

879 where $KE_{initial}$ is the initial kinetic energy of the fluid parcel, ΔE_{gain} is the energy contribution
880 from pressure gradients in the fluid, KE_{final} is the kinetic energy the fluid parcel retains due to
881 its along-strike motion, ρ is the density of the fluid, $g' = g \Delta\rho/\rho$ is the reduced gravitational
882 field strength, Δz is the vertical height gain and ΔE_{loss} is the energy lost to friction and/or
883 turbulent dissipation. The fluid parcel here is approximated as retaining its density throughout
884 the run-up, so that the excess density $\Delta\rho/\rho$ remains constant.

885 The initial kinetic energy can be estimated using trigonometric relationships between velocity
886 components. For simplicity, we work with a horizontally averaged initial downstream velocity
887 $\vec{U} = (u_z, 0, 0)$ for each parcel of fluid meeting the ramp, where the x , y and z axes are aligned
888 with the downstream, cross-stream and vertical directions, respectively (Fig. 2A). The

889 velocity can be decomposed into two horizontal components: the horizontal updip component
 890 \vec{U}_{hu} with a magnitude of $U \sin \varphi$ and the along-strike component \vec{U}_{as} with a magnitude of
 891 $U \cos \varphi$ (**Fig. 4B**). The along-strike component does not contribute to the maximum upslope
 892 height H_{max} , since it remains approximately constant throughout the run-up process, meaning
 893 its associated kinetic energy $KE_{final} = \frac{1}{2}\rho U^2 \cos^2 \varphi$ can be subtracted from both sides of
 894 Equation S1, with any losses absorbed into the ΔE_{loss} term. While the kinetic energy associated
 895 with the horizontal updip component is the primary source of the fluid parcel's ultimate
 896 gravitational potential energy, some energy dissipation will occur during the initial collision
 897 with the ramp surface (**Video S1**). The component of \vec{U}_{hu} that is normal to the ramp, denoted
 898 as \vec{U}_n , must change direction upon collision with the ramp, so only an unknown fraction of the
 899 corresponding kinetic energy will be available to contribute to the maximum run-up elevation.
 900 Thus, we further decompose \vec{U}_{hu} into the updip and the normal component: $\vec{U}_{hu} = \vec{U}_u + \vec{U}_n$
 901 (**Fig. 4C**). A dimensionless collision factor S (ranging from 0 to 1) characterises the fraction of
 902 the normal component's kinetic energy that contributes to H_{max} . Thus, the kinetic energy from
 903 the updip velocity ($\frac{1}{2}\rho U^2 \sin^2 \varphi \cos^2 \theta$) and a fraction S of that from the normal velocity
 904 ($\frac{1}{2}\rho U^2 \sin^2 \varphi \sin^2 \theta$) represent the total kinetic energy available for conversion to potential
 905 energy. The energy loss ΔE_{loss} is expected to increase with distance and so for simplicity, is
 906 herein approximated as work done by an effective average dissipative force, $F_{ave}\Delta D$, where
 907 F_{ave} is the average dissipative force per unit volume, acting in the direction opposed to the fluid
 908 parcel's velocity and ΔD is the total distance travelled up the slope. Substituting into Equation
 909 S1 and subtracting KE_{final} from each side yields:

$$910 \quad \frac{1}{2}\rho U^2 \sin^2 \varphi \cos^2 \theta + S \left(\frac{1}{2}\rho U^2 \sin^2 \varphi \sin^2 \theta\right) + \Delta E_{gain} = \rho g' \Delta z + \frac{F_{ave}\Delta z}{\sin \alpha} \quad (S2)$$

911 where ΔD has been approximated by $\frac{\Delta z}{\sin \alpha}$, with $\alpha = \tan^{-1}(\sin \varphi \tan \theta)$ representing the
 912 ‘effective slope’ of the ramp in the downstream (x) direction in the vertical (x, z) plane (**Fig.**
 913 **4D**).

914 The maximum run-up height for a fluid parcel at initial height z is then given by:

$$915 \quad h_{max}(z) = z + \frac{\frac{1}{2}\rho_z u_z^2 \sin^2 \varphi (\cos^2 \theta + S \sin^2 \theta) + \Delta E_{gain}}{\Delta \rho_z g + \frac{F_{ave}}{\sin \alpha}} \quad (S3)$$

916 with the z subscripts referring to the density and velocity at height z . The overall H_{max} is the
 917 maximum h_{max} for all fluid parcels, and thus occurs when

$$918 \quad \frac{d}{dz} \left(z + \frac{\frac{1}{2}\rho_z u_z^2 \sin^2 \varphi (\cos^2 \theta + S \sin^2 \theta) + \Delta E_{gain}}{\Delta \rho_z g + \frac{F_{ave}}{\sin \alpha}} \right) = 0 \quad (S4)$$

919 Therefore, the maximum run-up height H_{max} for the fluid is a function of their measured
 920 vertical velocity profile and density profile and hence cannot be completely specified until the
 921 velocity and density profiles are known (Kneller and McCaffrey, 1999).

922 In the limiting case where $S = 1$ (no energy lost during in the collision with the ramp), Equation
 923 S3 simplifies to:

$$924 \quad h_{max}(z) = z + \frac{\frac{1}{2}\rho_z u_z^2 \sin^2 \varphi + \Delta E_{gain}}{\Delta \rho_z g + \frac{F_{ave}}{\sin \alpha}}$$

925 For $S = 0$ (maximal collision loss model), it simplifies to:

$$926 \quad h_{max}(z) = z + \frac{\frac{1}{2}\rho_z u_z^2 \sin^2 \varphi \cos^2 \theta + \Delta E_{gain}}{\Delta \rho_z g + \frac{F_{ave}}{\sin \alpha}}$$

927 In frontal settings where $\varphi = 90^\circ$, the equation further reduces to:

$$928 \quad h_{max}(z) = z + \frac{\frac{1}{2}\rho_z u_z^2 \cos^2 \theta + \Delta E_{gain}}{\Delta\rho_z g + \frac{F_{ave}}{\sin \alpha}}$$

929 The model can also be generalised to incorporate initial velocities with significant cross-stream
930 and vertical components, if data is available for these. The initial velocity of the fluid parcel at
931 height z can be generalised to $\vec{U} = (u_z, v_z, w_z)$, where u , v and w are the downstream, cross-
932 stream and vertical velocity components, respectively, which for simplicity are assumed to
933 depend only on z (although in reality these velocity components will depend on x, y too). As
934 before, the predicted run-up height for a fluid parcel at initial height z is

$$935 \quad h_{max}(z) = z + \frac{\frac{1}{2}\rho_z((U_u(z))^2 + S(U_n(z))^2) + \Delta E_{gain}}{\Delta\rho_z g + \frac{F_{ave}}{\sin \alpha}} \quad (S5)$$

936 where the various quantities are defined as follows: $U_u(z)$, and $U_n(z)$ are the projections of \vec{U}
937 onto the up-dip direction and the direction normal to the ramp surface, respectively; ΔE_{gain} is
938 the energy gained from internal pressure and interactions of the fluid parcel with neighbouring
939 fluid parcels; S is a dimensionless collision factor ranging from 0 to 1, characterising the
940 fraction of kinetic energy associated with the normal component of the initial velocity that
941 contributes to H_{max} ; F_{ave} is the average dissipative force per unit volume, acting in the
942 direction opposed to the fluid parcel's velocity; the angle $\alpha = \tan^{-1}(\sin(\varphi +$
943 $\beta) \tan \theta)$ represents the 'effective slope' of the ramp in the vertical plane of the initial fluid
944 parcel's initial velocity, and is a function of the overall flow incidence angle φ against the ramp
945 and the slope gradient θ , as well as the angle β between the horizontal component of the fluid

946 parcel's velocity and the overall downstream direction, given by $\beta = \tan(v_z/u_z)$ (**Fig. 4D**).

947 The up-dip and normal projections of the velocity are

$$948 \quad U_u(z) = (u_z \sin \varphi - v_z \cos \varphi) \cos \theta + w_z \sin \theta$$

$$949 \quad U_n(z) = -(u_z \sin \varphi - v_z \cos \varphi) \sin \theta + w_z \cos \theta$$

950 Typically, the downstream velocity profile u_z is more predictable than those of the cross-
951 stream and vertical components, which will vary across the gravity current head, taking a range
952 of positive or negative values, depending on both the degree of lateral flow spreading and
953 unpredictable turbulent fluctuations. As a result, these components may be treated as small
954 fluctuations lying in some range proportional to the downstream velocity, $-\Delta v \leq v_z \leq \Delta v$ and
955 $-\Delta w \leq w_z \leq \Delta w$, with $\Delta v = f_y u_z$ and $\Delta w = f_z u_z$, where f_y and f_z quantify the lateral and
956 vertical variability as a dimensionless fraction of the downstream velocity, respectively (found
957 to be ca. 10% in Nomura et al., 2019; Marshall et al., 2023; Keavney et al. 2024). Then, as
958 before, one can find the maximum run-up height of the turbidity current by finding the
959 maximum value of the function $h_{max}(z)$.

960 **Supporting Information 5: Comparison of the numerical-model predictions with the** 961 **observed experimental data using 3D velocity components**

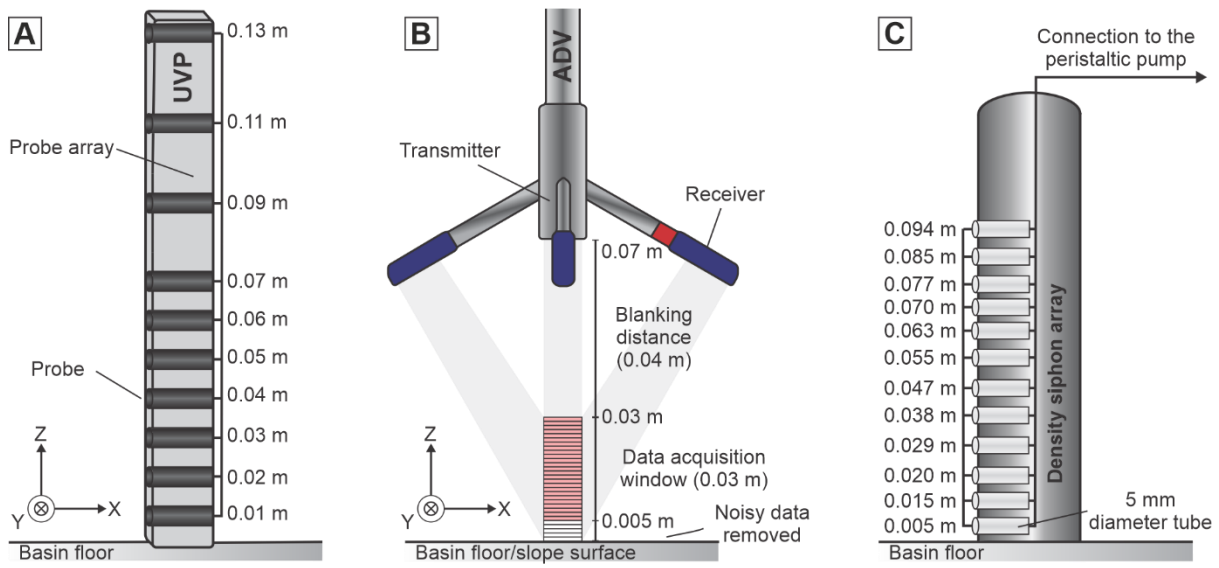
962 To test the validity of the numerical model above, its general run-up height predictions are
963 compared to the observed values for each of the 18 ramp experiments herein, with the aim of
964 approximating the overall dependence of (normalised) maximum run-up height on flow
965 incidence angle and slope gradient. Due to the broad approximations made in the model and
966 the turbulent nature of the flow ($Re \approx 3000$), only an approximate fit to the data is to be
967 expected. To be fully realistic would require a Computational Fluid Dynamics simulation, but
968 the purpose here is to provide a method of estimation that can be calculated quickly for practical
969 purposes. Values of the input quantities representative of those measured in the current physical

970 experiments in the first 5 s after the current head were substituted into the model ($z = 0.045$ m,
971 $\rho_z = 999.8$ kg/m³, $\Delta\rho_z = 0.22$ kg/m³, $h = 0.11$ m, $u_z = 0.0243$ m/s, $v_z = -0.12u_z$, $w_z = 0.09u_z$;
972 **Fig. S2**). However, accurate values for the energy gain ΔE_{gain} , averaged dissipative force
973 F_{ave} and collision factor S were not available from the physical experiments. Here, for
974 simplicity the collision factor S is assumed to be 0, i.e., none of the kinetic energy associated
975 with the component of the velocity normal to the ramp was converted into gravitational
976 potential energy, while optimised values for the remaining two parameters giving the best fit
977 with the observed experimental data points were found to be $\Delta E_{gain} = 0.479$ Jm⁻³ and $F_{ave} =$
978 0.459 Nm⁻³ (**Fig. S3A**). A contour map of the modelled normalised maximum run-up height,
979 H_{max}/h , as a function of the flow incidence angle onto the slope (φ) and the angle of slope
980 gradient (θ), using the input values above is given in **Figure S3B**.

981 Overall, our numerical model captured the first-order dependence of normalised maximum run-
982 up height as a function of flow incidence angle and slope gradient (**Fig. 6B**). A critical θ and
983 φ exists ($\theta = 35.5^\circ$ and $\varphi = 82.7^\circ$), where H_{max}/h reaches its maximum value of 2.58. This is
984 approximately consistent with the experimental observations of higher H_{max}/h for a slope of
985 30° compared to that of 20° and 40° in an oblique setting (**Fig. 3J**). Additionally, when the
986 slope gradient is set to a value equal to or close to the critical value, the normalised maximum
987 run-up height first increases first with a higher flow incidence angle and then begins to decrease
988 again towards 90° , consistent with the seemingly anomalous low value of H_{max}/h in frontal
989 setting of Experiment 30° (**Fig. 3I**). When the slope gradient is set to a constant deviated from
990 the critical value, the normalised maximum run-up height increases with a higher flow
991 incidence angle, consistent with the observed general positive relationship between values of
992 H_{max}/h versus the flow incidence angle in our physical experiments (**Fig. 3I**).

993

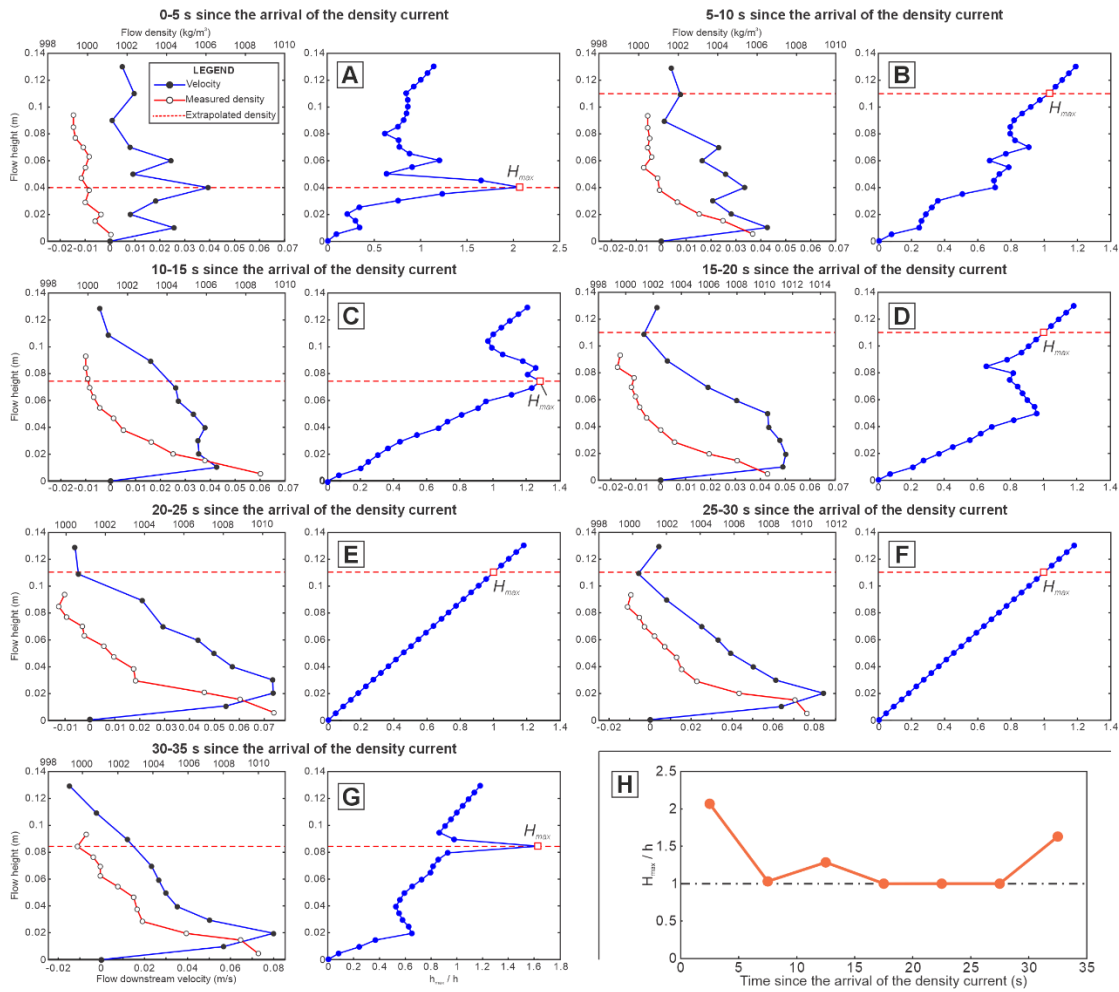
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998 **Fig. S1.** Set up of (A) the UVP, (B) ADV and (C) siphoning systems in this study to measure
 999 the velocity and density profiles, respectively. All profiles were measured vertical to the basin
 1000 floor, irrespective of whether the instrument was mounted above the basin floor or the slope
 1001 surface. Modified after Keavney et al. (2024).

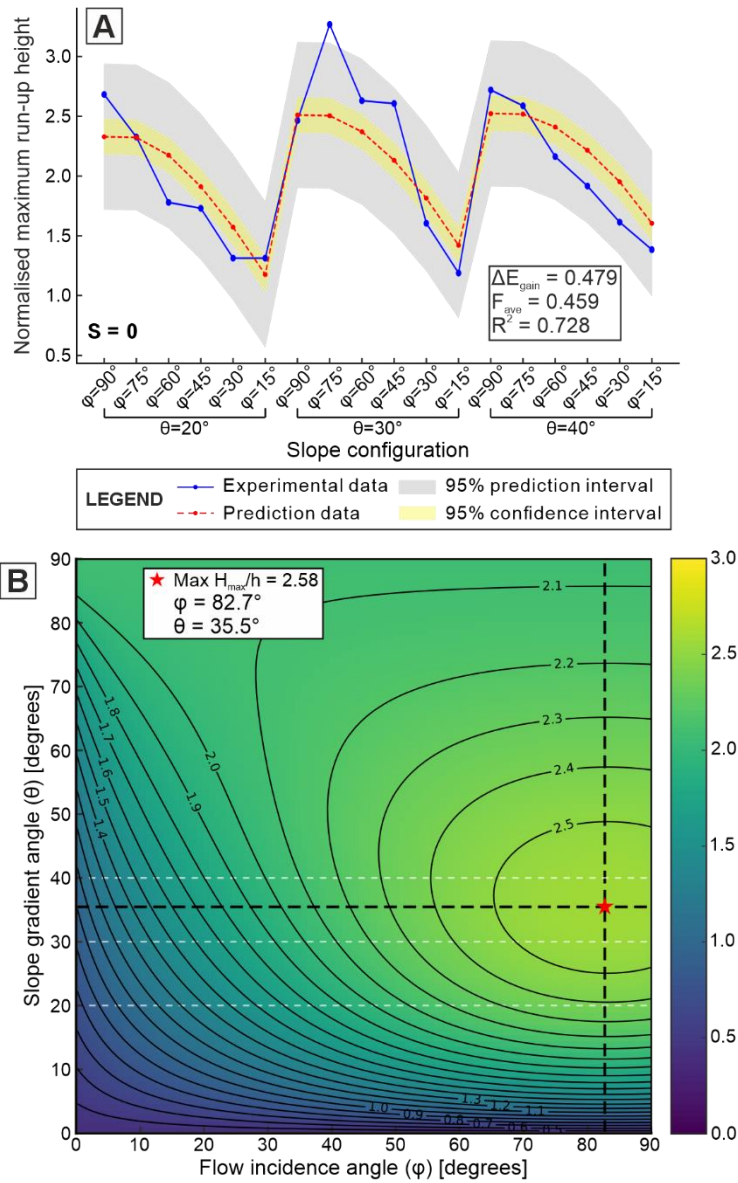
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1003

1004 **Fig. S2.** (A-G) Time-averaged downstream velocity profile of UVP and density profile of the
 1005 experimental density currents at 3 m downstream from the channel mouth along the channel-
 1006 basin centreline for the first 35 s (every 5 s time window) after the current head in the
 1007 unconfined reference experiment. The panel map on the right for each time interval indicates
 1008 the vertical profile of the estimated maximum run-up elevation h_{max} for any parcel of the fluid
 1009 at initial height z , with the maximum run-up height H_{max} for the overall flow indicated as a
 1010 red square. (H) Distribution of the H_{max}/h for the experimental density currents at 3 m
 1011 downstream from the channel mouth along the channel-basin centreline for the first 35 s time
 1012 window after the current head in the unconfined reference experiment.

1013



1014

1015 **Fig. S3.** (A) Comparison between observed and predicted values of normalised maximum run-

1016 up height upslope for our 18 ramp experiments. The results of optimised ΔE_{gain} and F_{ave} using

1017 Equation S5 best-fit for the observed experimental data points and the fit accuracy (R^2) are

1018 reported in the bottom right box. The input quantities in the model are set to constant,

1019 representative for the current physical experiments in the first 5 s after the current head ($z =$

1020 0.045 m, $\rho_z = 999.8$ kg/m³, $\Delta\rho_z = 0.22$ kg/m³, $h = 0.11$ m, $u_z = 0.0243$ m/s, $v_z = -0.12u_z$, $w_z =$

1021 $0.09u_z$). (B) Contour map of modelled normalised maximum run-up height, H_{max}/h , as a

1022 function of the flow incidence angle onto the slope (φ) and slope gradient (θ), with the input

1023 variables set to constant values typical of current physical experiments and the optimised

1024 ΔE_{gain} and F_{ave} . A critical θ and φ for the current setting is shown to exist, whereby H_{max}/h
1025 reaches its maximum value.

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1027 **SUPPLEMENTARY VIDEO CAPTIONS**

1028 **Movie S1. Annotated video illustrating the behaviour of density currents upon incidence**
1029 **with an oblique topographic slope (Experiment S40°IN75°).**

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